### ADDENDUM NO. 1

to the

# PROJECT DOCUMENTS AND SPECIFICATIONS

for

# DFS TERMINAL, HANGAR, AND APRON DEVELOPMENT

# Prepared for:

# CITY OF DEFUNIAK SPRINGS, FLORIDA

Prepared By:



320 Bayshore Drive, Suite A Niceville, Florida 32578-2425

**AVCON Project No. 2019.0028.01** 

Addendum Date: February 7, 2022

Note: The bidder shall acknowledge receipt of this addendum on the Bid Form, Page B1-3 in the space provided.

# ADDENDUM NO. 1 DFS TERMINAL, HANGAR, AND APRON DEVELOPMENT

**Date of Issue:** February 7, 2022

Bid Submittal Deadline: Thursday, March 3, 2022 @ 2:00 a.m. (local time) (UNCHANGED)

**Notice to all Plan Holders:** Please insert this addendum (2 pages including cover, excluding attachments)

into your copy of the Project Bid Documents.

The following changes to the Project Documents and Specifications are issued by the Engineer and shall have the same force and effect as though part of the original issue:

# A. Changes to the Bid Drawings:

1. Structural Drawings ADD the following structural design drawings in their entirety included in

Attachment A (20 pages) hereto.

\*\*Sheets S-1.00 through S-5.07\*\*

**END OF ADDENDUM NO. 1** 

### **GENERAL:**

- ALL STRUCTURAL WORK SHALL BE IN ACCORDANCE WITH THE FOLLOWING 13.
  - FLORIDA BUILDING CODE 2020
  - REGULATIONS IN ACCORDANCE WITH THE LOCAL JURISDICTION.
    SIGNED AND SEALED PROJECT CONTRACT DOCUMENTS (INCLUDING SIGNED AND SEALED DRAWINGS BY DELEGATED ENGINEERS AND THE GEOTECHNICAL REPORT) AND LATEST ADDENDA. CONTRACT DOCUMENTS DO NOT INCLUDE SHOP DRAWINGS AND OTHER UNSEALED SUBMITTAL DOCUMENTS.
- THE REFERENCE OF "GENERAL CONTRACTOR" WITHIN THE STRUCTURAL GENERAL NOTES INCLUDES BUT IS NOT LIMITED TO SUBCONTRACTORS ERECTORS. FABRICATORS. MATERIAL SUPPLIERS AND/OR INDIVIDUALS PERFORMING THE WORK.
- THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH PROJECT SPECIFICATIONS, ARCHITECTURAL, MECHANICAL, ELECTRICAL PLUMBING, AND SITE DRAWINGS. REFER TO THESE DRAWINGS FOR DIMENSIONS, EMBEDDED ITEMS, AND OTHER DETAILS NOT SHOWN ON THE STRUCTURAL DRAWINGS. THE GENERAL CONTRACTOR SHALL REVIEW THE DRAWINGS OF ALL DISCIPLINES AND REPORT ANY DISCREPANCIES TO THE ARCHITECT OF RECORD AND STRUCTURAL ENGINEER OF RECORD IN WRITING PRIOR TO SECURING MATERIALS, FABRICATING, OR COMMENCING WORK. THE MORE STRINGENT REQUIREMENTS SHALL GOVERN UNLESS OTHERWISE STATED IN WRITING BY THE ARCHITECT OF RECORD AND STRUCTURAL ENGINEER OF RECORD.
- DO NOT SCALE DRAWINGS.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND FIELD CONDITIONS PRIOR TO SECURING MATERIALS, FABRICATING, OR
- NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED, OR OTHERWISE ALTERED UNLESS APPROVED IN WRITING BY THE ENGINEER OF RECORD.
- THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER ITS CONSTRUCTION IS COMPLETE. THE CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE AND THEY DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS TO ENSURE STABILITY AND SAFEKEEPING OF THE STRUCTURE AND ITS COMPONENTS DURING CONSTRUCTION AS PER THE MOST RECENT PRINTING/ERRATA OF ASCE 37-14 'DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION'. THE ENGINEER DOES NOT HAVE CONTROL OF, AND SHALL NOT BE RESPONSIBLE FOR: CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, SAFETY PRECAUTIONS, SAFETY PROGRAMS IN CONNECTION WITH THE WORK, OMISSIONS BY THE GENERAL CONTRACTOR, OR THE FAILURE OF THE GENERAL CONTRACTOR TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT
- DETAILS LABELED "TYPICAL" ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. THE APPLICABILITY OF THE DETAIL TO ITS LOCATION ON THE PLANS CAN BE DETERMINED BY THE TLE OF DETAIL. SUCH DETAILS SHALL APPLY WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION, QUESTIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE DIRECTED TO THE STRUCTURAL ENGINEER
- PERIODIC, LIMITED, SITE OBSERVATION BY FIELD REPRESENTATIVES OF AVCON, INC IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE CONTRACTOR'S WORK IS PROCEEDING IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. SITE OBSERVATIONS SHALL NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS CHECKS OF THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER AS PERIODIC SPOT CHECKS OF READILY APPARENT DEFECTS OR DEFICIENCIES IN THE WORK LIMITED SITE VISITS. BY THE ENGINEER OF RECORD DO NOT RELIEVE OR TAKE THE PLACE OF SCHEDULED TESTING AND INSPECTIONS BY AUTHORIZED AGENT OR
- ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE. A PLANNED MAINTENANCE PROGRAM SHALL BE ESTABLISHED BY THE OWNER AND SHALL INCLUDE ITEMS SUCH AS, BUT NOT LIMITED TO:
- PROTECTIVE COATINGS FOR STEEL AND SEALANTS FOR CRACKED CONCRETE SURFACES
- SEALANTS WITHIN EXPANSION AND CONTROL JOINTS
- GENERAL CLEANING OF EXPOSED STRUCTURAL ELEMENTS TO HARSH ENVIRONMENTS OR CHEMICALS
- THE USE OF REPRODUCTION OF THESE CONTRACT DOCUMENTS AND/OR USE OF CAD FILES BY THE GENERAL CONTRACTOR IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFY HIS ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT, AND OBLIGATES HIMSELF TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT
- THE GENERAL CONTRACTOR SHALL PROTECT EXISTING FACILITIES, STRUCTURES, AND UTILITIES FROM DAMAGE. INFORMATION SHOWN RELATED TO EXISTING STRUCTURES WAS GATHERED FROM DRAWINGS DATED \_\_\_ PREVIOUSLY PREPARED FOR PREVIOUSLY PREPARED FOR BY DAI

  / / . THE STRUCTURAL DRAWINGS ASSUME THAT THE

  CONSTRUCTION OF EXISTING STRUCTURES WERE PERFORMED IN ACCORDANCE WITH THE PREVIOUSLY PREPARED DRAWINGS INCLUDING, BUT NOT LIMITED TO DIMENSIONS ELEVATIONS MEMBER SIZES MATERIALS, DETAILS, ETC. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO VERIFY THE EXISTING CONDITIONS AND NOTIFY THE ARCHITECT OF RECORD AND STRUCTURAL ENGINEER OF RECORD OF ANY DISCREPANCIES IN WRITING IMMEDIATELY

- THE GENERAL CONTRACTOR IS REQUIRED TO PERFORM HIS WORK IN A MANNER WHICH MINIMIZES CONFLICT WITH THE OPERATIONS OF THE FACILITY DURING THE COURSE OF THE PROJECT AND TO TAKE ALL PRECAUTIONS TO MINIMIZE VIBRATION, NOISE, DUST AND DEBRIS IN ALL AREAS ADJACENT TO AREAS OF WORK
- STRUCTURAL WORK SHALL BE INSPECTED BY QUALIFIED INSPECTORS. FIELD INSPECTION REPORTS SHALL BE FILED WITH THE STRUCTURAL ENGINEER OF RECORD WITHIN 5 DAYS OF TIME OF ACTUAL INSPECTION.
- 15 SUBSTITUTIONS IN ITEMS (PRODUCTS MATERIALS FOUIPMENT AND INSTRUCTIONS) WITHIN THE CONTRACT DOCUMENTS FOR ANY REASON SHALL BE APPROVED BY THE ARCHITECT OF RECORD AND STRUCTURAL ENGINEER OF RECORD PRIOR TO SECURING MATERIALS, FABRICATING, OR COMMENCING WORK. THE CONTRACTOR SHALL SUBMIT REQUEST FOR SUBSTITUTION IN WRITING TO THE STRUCTURAL ENGINEER OF RECORD IMMEDIATELY UPON DISCOVERY OF NEED AND 15 DAYS PRIOR TO INITIAL LEAD TIME DATE THE SUBSTITUTION REQUEST SHALL BE SIGNED/DATED AND INCLUDE. AT MINIMUM, THE
  - THE ITEM TO BE SUBSTITUTED WITH CLEAR REFERENCE TO THE INTENDED USE IN THE STRUCTURAL SPECIFICATIONS AND DRAWINGS
- REASONS FOR SUBSTITUTION INCLUDING CHANGES TO CONTRACT COST AND SCHEDULE
- MANUFACTURER TECHNICAL PRODUCT DATA SHEETS, TEST REPORTS FOR THE SUBSTITUTION ITEM
- A STATEMENT OF ITEM'S COMPLIANCE WITH THE FLORIDA BUILDING CODE
- A STATEMENT OF ITEM'S COMPATIBILITY WITH OTHER PORTIONS OF WORK
- 16. THE GENERAL CONTRACTOR SHALL MAINTAIN, IN GOOD ORDER WITH ALL CHANGES RECORDED AS THEY OCCUR DURING CONSTRUCTION, AS-BUILT DRAWINGS AT THE SITE FOR THE OWNER CONSISTING OF ONE COLLECTIVE COPY OF THE FOLLOWING:
  - ALL DRAWINGS
  - ALL SPECIFICATIONS
  - ALL ADDENDA
  - ALL APPROVED SHOP DRAWINGS

  - ALL APPROVED SETTING DRAWINGS
    ALL CHANGE ORDERS AND OTHER MODIFICATIONS

THESE SHALL BE AVAILABLE TO THE ARCHITECT OF RECORD, ENGINEER OF RECORD, OWNER, OWNER AUTHORIZED REPRESENTATIVE, AND PROJECT INSPECTOR. THE DRAWINGS SHALL BE NEATLY AND CLEARLY MARKED DURING CONSTRUCTION TO RECORD ALL VARIATIONS MADE DURING CONSTRUCTION. UPON COMPLETION OF THE WORK AND PRIOR TO THE FINAL INSPECTION. THE CONTRACTOR SHALL DELIVER ONE COMPLETE SET OF AS-BUILT DRAWINGS TO THE ARCHITECT OF RECORD AND ENGINEER OF RECORD, FOR PREPARATION OF THE RECORD DRAWINGS.

### **GEOTECHNICAL:**

- REFER TO THE LATEST EDITION OF THE GEOTECHNICAL REPORT FOR RECOMMENDATIONS AND INSTALLATION PROCEDURES. SITE PREPARATION AND FOUNDATION INSTALLATION SHALL COMPLY WITH:
- REPORT No. 1011-2020054
- PREPARED BY: NOVA
- DATED: MAY 29, 2020
- VERIFICATION THAT THE COMPACTION REQUIREMENTS HAVE BEEN MET SHALL BE MADE BY THE GEOTECHNICAL CONSULTANT PRIOR TO PLACING FOUNDATIONS AND SLABS.
  LOCATIONS FAILING TO MEET THE REQUIREMENTS SHALL BE RECOMPACTED AND RETESTED AT THE CONTRACTORS EXPENSE AND AS DIRECTED BY THE GEOTECHNICAL CONSULTANT.

#### **SHALLOW FOUNDATIONS:**

- SHALLOW FOUNDATION DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF AS PER THE REFERENCED GEOTECHNICAL REPORT
- NO CONCRETE SHALL BE PLACED IN WATER. CONTRACTOR SHALL SAFEGUARD AND PROTECT ALL EXCAVATIONS AND SHALL KEEP THEM FREE OF WATER. GROUND WATER MUST BE MAINTAINED AT A MINIMUM OF 2 FT. BELOW BOTTOM OF EXCAVATION AT ALL
- LOCATE ALL EXISTING UTILITIES IN THE CONSTRUCTION AREA PRIOR TO EXCAVATION AND AVOID DAMAGE TO THEM CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIR OF ANY AND ALL DAMAGED UTILITIES CAUSED IN CONSTRUCTION TO THE SATISFACTION OF THE LOCAL BUILDING OFFICIALS AND/OR OWNER.
- BACKEILLING AGAINST FOLINDATION WALLS SHALL NOT BE DONE LINTIL WALL HAS BEEN CURED TO A SUFFICIENT STRENGTH (7 DAYS MINIMUM), AND WALLS ARE PROPERLY SHORED OR BRACED LISE MECHANICAL TAMPERS OR OTHER APPROVED HAND TAMPERS WITHIN FIVE FEET OF THE PERIMETER FOR ALL CONCRETE STRUCTURES AND BURIED STRUCTURES TO WITHIN 18 INCH COVERAGE
- CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS WITHIN ±2", UNO.
- COORDINATE BURIED PLUMBING AND OTHER LITHLITY LINES WITH FOOTING LOCATIONS FOR INTERFERENCE AS PER THE TYPICAL CONDUIT PENETRATION AT FOOTING DETAIL. WITH PRIOR WRITTEN APPROVAL OF THE ENGINEER OF RECORD, WALL FOOTINGS MAY BE STEPPED AND ISOLATED COLUMN FOOTINGS MAY BE DROPPED AS IN ACCORDANCE WITH THE TYPICAL WALL STEP FOOTING DETAIL
- EXCAVATING UNDER OR NEAR IN-PLACE FOUNDATIONS WHICH DISTURBS THE COMPACTED SOIL BENEATH SHALL NOT BE PERMITTED. TRENCH EXCAVATIONS WITHIN THE 45 DEGREE LOAD INFLUENCE WIDTH ALONG FOUNDATIONS ARE PROHIBITED.
- CONSTRUCTION JOINTS IN CONTINUOUS FOOTINGS SHALL TERMINATE IN A VERTICAL CONCRETE FACE WITH A HORIZONTAL FORMED KEYWAY (2x4), CONTINUE REINFORCING THROUGH THE CONSTRUCTION JOINT
- SECURE ALL WALL AND COLUMNS DOWELS, ANCHOR RODS, AND OTHER EMBEDDED COMPONENTS INTO PROPER PLACE WITHIN FOOTINGS. DO NOT WET-SET THEM INTO NEWLY POURED CONCRETE.

### **CAST-IN-PLACE CONCRETE:**

- ALL CAST-IN-PLACE CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT PRINTING/ERRATA OF ACI 318-14 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' AND ACI 301-10 'SPECIFICATION FOR STRUCTURAL CONCRETE
- STRUCTURAL CONCRETE MIX DESIGNS SHALL BE AN APPROVED COMMERCIAL MIX OF PLASTIC AND WORKABLE CONSISTENCY AT TIME OF PLACEMENT CONFORMING TO THE MINIMUM REQUIREMENTS PER THE CONCRETE MIX DESIGN SCHEDULE.
- SUBMIT MIX DESIGN FOR ENGINEER'S APPROVAL FOR EACH CLASS WITH SPECIFIC LOCATION OF PLACEMENT INDICATED NO LATER THAN TWO WEEKS PRIOR TO SECURING CONCRETE MATERIALS. EACH MIX DESIGN SHALL INCLUDE TESTED, STATISTICAL BACK-UI DATA AS PER ACI 301. ALL MIXES SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C33
- CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE BATCH TICKET SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED. ALL CONCRETE MIXES USING MAXIMUM COURSE AGGREGATE SIZE OF 3/8" (PEAROCK) SHALL CONFORM TO THE REQUIREMENTS OF ASTM C89 AND SHALL BE CONTINUOUSLY PLACED TO NOT EXCEED 50 LINEAR FEET.
- PLACE AND CURE CONCRETE IN ACCORDANCE WITH ACI STANDARDS AND SPECIFICATIONS. DISCARD CONCRETE EXCEEDING 1-1/2 HOURS FROM THE TIME THE MIXING WATER IS ADDED AT THE BATCH PLANT UNTIL THE CONCRETE IS DEPOSITED IN ITS FINAL POSITION.
- A CERTIFIED TESTING AGENT SHALL PERFORM INDUSTRY STANDARD TESTS SUCH AS BUT NOT LIMITED TO SLUMP, CYLINDER AND PRISM COMPRESSIVE BREAKS, UNIT WEIGHT, ETC. SLUMP TEST SAMPLES SHALL BE TAKEN AT DISCHARGE POINTS OF CONCRETE PER ASTM C143 'STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE'. WHERE NOT INCLUDED IN THE SPECIFICATIONS, COLLECT AND TEST 4 CYLINDER SETS FOR EACH COMPRESSIVE STRENGTH TEST IN CONFORMANCE WITH ASTM C39 'STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS' EACH SET OF CYLINDRES CONSIST OF ONE FIELD-CURED SET OF CYLINDRES AND ONE LABORATORY-CURED SET OF CYLINDERS. TESTS SHALL BE CONDUCTED FOR EACH CLASS AND EACH DAY PLACED AT A FREQUENCY OF EVERY 50 CUBIC YARDS. BREAK ONE SET OF CYLINDERS AT 7 DAYS, 1 SET AT 28 DAYS, AND HOLD 2 SETS IN RESERVE. BREAK RESERVE CYLINDERS AS DIRECTED BY THE ENGINEER. TESTING AGENT SHALL SUBMIT REPORTS TO THE ENGINEER WITHIN FIVE WORKING DAYS OF TEST RESULTS.
- ANY DEVIATION OR ADDITION OF CONSTRUCTION JOINTS FROM THOSE SHOWN ON PLANS MUST BE SUBMITTED IN THE CONCRETE ERECTION OR REINFORCING STEEL SHOP
- FOR SHOWROOM INTERIOR SLABS ONLY: SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAIL)
- ALL TIE BEAMS (TB#) AND COLUMNS (TC#) INTEGRATED IN CMU WALLS ARE INDICATED AS NOMINAL WIDTH DIMENSIONS. ACTUAL DIMENSIONS SHALL BE THE WIDTH OF THE CMU WALL. USE METAL LATH, MORTAR, OR SPECIALTY PRODUCT TO CONFINE POURED CONCRETE AT ALL TIE BEAMS AND BOND BEAMS IN ACCORDANCE WITH ACL 530.1. USE OF SOLID METAL, FELT CAVITY CAPS, AND PAPER ARE PROHIBITED.
- CONTRACTOR SHALL COORDINATE ALL TRADES FOR INSTALLATION OF ALL BUILT-IN WORK, SLEEVES, INSERTS, ETC. AS REQUIRED FOR THE COMPLETION OF CONSTRUCTION
- ALL CORNERS AND EDGES OF PERMANENTLY EXPOSED CONCRETE SHALL BE 3/4" CHAMFER, UNO.
- ALL STRUCTURAL SLABS AND FLAT HORIZONTAL SURFACES TO REMAIN EXPOSED TO WEATHER THROUGHOUT ITS LIFETIME SHALL BE TREATED WITH A CLEAR NON-FLAMMABLE PENETRATING SILANE SEALER. PREPARE CONCRETE SURFACES AND APPLY SEALER IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS
- REINFORCING STEEL: REINFORCING STEEL SHALL BE ASTM A615 GRADE 60 DEFORMED BARS AND WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI MANUAL OF STANDARD PRACTICE AND THE PROJECT SPECIFICATIONS. SUBMIT REINFORCING SHOP DRAWINGS AND OBTAIN ENGINEER'S APPROVAL PRIOR TO SECURING MATERIAL AND COMMENCING FABRICATION. SEE THE CONCRETE SCHEDULE(S) FOR ALL REQUIRED INFORMATION TO BE SHOWN ON THE BEAM REINFORCING ERECTION SHOP DRAWING SUBMITTAL.
- PROVIDE CONCRETE COVER OVER REINFORCEMENT PER THE CONCRETE COVER
- PROVIDE STANDARD HOOKS AT DISCONTINUOUS ENDS OF ALL TOP BARS. TOP BARS AT EXPOSED AREAS SUCH AS BALCONIES, WALKWAYS AND AS SPECIFICALLY SHOWN ON PLAN SHALL BE EPOXY COATED ACCORDING TO ASTM A775
- PROVIDE 48xBAR DIAMETER FOR ALL LAP SPLICES IN FOUNDATIONS AND WHERE SPECIFIED ELSEWHERE IN THE PLANS. LAP WELDED WIRE FABRIC SHEETS WITH ONE
- PROVIDE CONTINUOUS REINFORCING OR STAGGER SPLICES WHERE POSSIBLE. DO NOT EXCEED 3" CENTER-TO-CENTER BAR SPACING OF SPLICED REINFORCING AND LOCATE SPLICE REINFORCING IN THE SAME PLANE WITH RESPECT TO THE CONCRETE ELEMENT'S CLEAR CONCRETE SURFACE.

	BAR SIZE	3000 PSI	4,000 PSI AND GREATER
Α.	#6 AND SMALLER	58xBAR DIA	48xBAR DIA
B.	#7 AND GREATER	72xBAR DIA	62xBAR DIA

#### CONCRETE MIX DESIGN SCHEDULE:

LOCATION	28-DAY STRENGTH	<u>SLUMP</u>	AGGREGATE SIZE
MASONRY FILLED CELLS LINTELS AND BOND BEAMS SLAB-ON-GRADE SIDEWALKS FOUNDATIONS BEAMS, COLUMNS, AND WALLS TIE BEAMS AND TIE COLUMNS	2500 2500 3000 3000 3000 4000 4000	8" - 11" 8" - 11" 4" ± 1" 4" ± 1" 4" ± 1" 5" ± 1" 6" ± 1"	3/8" 3/8" 3/4" 3/4" 1" 3/4" 3/8"
CONCRETE COVER SCHEDULE: LOCATION	4000	COVER	3/0
BEAMS, JOISTS, AND EXTERIOR S INTERIOR SLABS CAST-IN-PLACE COLUMNS, PEDES FOUNDATIONS CAST AGAINST EA	STALS, AND WALLS	1 1/2" 3/4" 1 1/2" 3"	

#### **MASONRY:**

- ALL MASONRY WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT PRINTING/ERRATA OF TMS 402-16 'BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES' AND TMS 602-16 'SPECIFICATION FOR MASONRY STRUCTURES'.
- CONCRETE MASONRY UNITS (CMU): CMU SHALL MEET THE REQUIREMENTS OF ASTM C90 FOR HOLLOW LOAD BEARING CMU WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2000 PSI / 3250 PSI / 4500 PSI / FOR A CORRESPONDING COMPRESSIVE STRENGTH OF MASONRY fm = 2000 PSI / 2500 PSI / 3000 PSI). COMPRESSIVE STRENGTH REQUIREMENTS SHALL BE MET PER THE UNIT STRENGTH METHOD WITHIN TMS 602 OR PRISM SAMPLING AND TESTING PER ASTM
- GROUT: GROUT SHALL CONFORM TO ASTM C476 WITH A SLUMP BETWEEN 8" AND 11", A MAXIMUM AGGREGATE SIZE OF 3/8". SUBMIT PROPOSED GROUT MIX DESIGNS FOR REVIEW PRIOR TO USE. MIX NUMBER OR OTHER POSITIVE IDENTIFICATION SHALL UNIQUELY IDENTIFY MIX. USE OF SUPERPLASTICIZER IS PROHIBITED AND SITE MIXING OF GROUT SHALL NOT BE PERMITTED SAMPLE AND TEST GROUT IN ACCORDANCE WITH ASTM C1019
- MORTAR: MORTAR SHALL MEET THE REQUIREMENTS OF ASTM C270 TYPE "M" FOR MASONRY LAID BELOW GRADE AND TYPE "S" FOR ALL OTHER LOCATIONS. FULLY BED CROSS WEBS OF BLOCKS ALL AROUND CELLS TO RECEIVE GROUT. BED MORTAR THICKNESS GREATER THAN 5/8" IS
- GROUTED CELLS: CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE. ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SLICH CELL WALLS. CMU SHALL BE MOISTENED BEFORE GROUTING CELLS. GROUT SHALL BE CONSOLIDATED AT TIME OF PLACING BY VIBRATING AND RECONSOLIDATED LATER BY VIBRATING BEFORE PLASTICITY IS LOST. USE METAL LATH OR SPECIAL UNITS OVER CELLS NOT TO BE FILLED WITH GROUT TO SUPPORT BOND BEAM. WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE POUR OF GROUT NOT LESS THAN 1 1/2" BELOW THE TOP OF THE UPPERMOST GROUTED MASONRY UNIT OR COURSE. GROUT POUR HEIGHT AND MINIMUM CELL DIMENSIONS TO RECEIVE GROUT SHALL MEET THAT PROVIDED BY THE "GROUT SPACE REQUIREMENTS" TABLE WITHIN TMS 602 WITH A MAXIMUM GROUT POUR HEIGHT OF 12 FEET. CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED FOR EACH POUR LIFT IN EXCESS OF 5 FEET IN HEIGHT. AFTER INSPECTION, THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING. ALL CMU IN CONTACT WITH SOIL SHALL BE FILLED SOLID WITH GROUT. THE FOLLOWING LOCATIONS SHALL BE FILLED SOLID WITH GROUT AND HAVE VERTICAL REINFORCING BARS:
- CORNERS
- INTERSECTIONS OF WALLS EACH END OF WALL
- EACH SIDE OF WALL OPENINGS EACH SIDE OF A CONTROL OR ISOLATION JOINT
- EACH END OF BEAM BEARING
- AS INDICATED ON THE PLANS
- VERTICAL REINFORCEMENT: VERTICAL REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60. VERTICAL DOWELS AND REINFORCING SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE WITH AT LEAST 6" EMBEDMENT INTO THE TOP BOND BEAM, UNO. PROVIDE HOOKED DOWELS IN FOOTINGS FOR VERTICAL REINFORCING ABOVE LAP SPLICES SHALL BE 48xBAR DIAMETERS, VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192xBAR DIAMETERS
- JOINT REINFORCEMENT: PROVIDE CONTINUOUS 9 GAGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR-O-WAL OR ENGINEER APPROVED SUBSTITUTION) CONFORMING TO ASTM A951 AT ALTERNATE BLOCK COURSES. PROVIDE PREFABRICATED "TEE" OR CORNER SECTIONS AT ALL INTERSECTING WALLS.
- BOND BEAM: PROVIDE A CONCRETE BOND BEAM AT THE FOLLOWING LOCATIONS WITH CONTINUOUS REINFORCEMENT AS INDICATED IN THE BEAM SCHEDULE
  - TOP OF WALL OR PARAPET
  - TOP OF OPENINGS OR DIRECTLY ABOVE LINTELS
  - BELOW OPENINGS AT BEARING LOCATIONS
  - AS INDICATED ON THE PLANS
- OPENINGS: COORDINATE ALL OPENINGS WITH THE ARCHITECTURAL DRAWINGS. PROVIDE REINFORCED CONCRETE LINTELS OVER OPENINGS AS INDICATED ON THE PLANS
- 10. JOINTS: REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION OF ALL MASONRY JOINTS AND WATERPROOFING DETAILS. REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS EXCEPT FOR BOND BEAM AND TIE BEAM REINFORCEMENT AS SPECIFIED ON PLAN. REINFORCING SHALL BE DISCONTINUOUS AT ISOLATION JOINTS.
- 11. CMU SHALL BE LAID IN RUNNING BOND PATTERN, UNO

| A T | O N A L Y SCOUT BLVD ITE 500 A, FL 33607 NSE # AA 2600248 ITERNA 1211 W. BOY: SUITE TAMPA, FRIDA LICENS INTER 4211 W.

IFFFREY YEAGER P.F. FL LICENSE NO.: 62853

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NOTES GENERAL

AIRPORT TERMINAL AND HANGAR DEVELOPMENT

DESIGNED BY: AJM AJM CHECKED BY: JY PROVED BY ROJECT NO: 2019.028.01 DATE: 11/1/2021

SHEET NUMBER

**S1.00** 

FOR PERMIT & BIDDING ONLY

#### **COLD-FORMED STEEL:**

- ALL COLD FORMED STEEL WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT PRINTING/ERRATA OF AISI S100-16 'NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS' AND AISI S202-15 'CODE OF STANDARD PRACTICE FOR COLD-FORMED STEEL STRUCTURAL FRAMING'. SECTION PROPERTY REQUIREMENTS SHALL CONFORM TO THE MINIMUM AS IDENTIFIED BY THE MOST RECENT PRINTING/ERRATA OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) PRODUCT TECHNICAL GUIDE.
- DELEGATED ENGINEER: SUBMIT SIGNED AND SEALED DETAILED DRAWINGS AND CALCULATIONS PREPARED UNDER THE SUPERVISION OF A FLORIDA PROFESSIONAL ENGINEER FOR ARCHITECT/ENGINEER REVIEW FOR ALL COLD-FORMED STEEL FRAMING, COMPONENTS, ATTACHMENTS, FASTENERS, CLOSURES, HARDWARE, ETC. FABRICATION OF FRAMING SHALL NOT BEGIN UNTIL THE SHOP DRAWINGS AND CALCULATIONS HAVE BEEN REVIEWED AND RETURNED APPROVED. EXTERIOR AND INTERIOR LOAD BEARING COLD-FORMED STEEL MEMBERS SHALL BE AS PER THE MINIMUM SIZES SHOWN ON PLAN AND DETAILS (WHERE NOTED). REFER TO CONTRACT SPECIFICATIONS FOR APPROVED SUPPLIERS. DRAWINGS AND CALCULATIONS SHALL INCLUDE, BUT ARE NOT LIMITED TO THE FOLLOWING:
  - ENGINEERING ANALYSIS SHOWING LOCATION, SPAN, PITCH, CAMBER. LOADING, BEARING CONDITIONS, CONNECTIONS, MEMBER SIZES, PROPERTIES, ANY YIELD STRENGTH, MEMBER STRESSES. DEFLECTIONS FOR EACH DIFFERENT ELEMENT TYPE CONFIGURATION, SPACING AND LAYOUT BASED ON DESIGN LOADS LISTED ON THE DRAWINGS.
  - ALL BRIDGING AND BRACING FOR DIAPHRAGM ACTION CONSTRUCTION LOADS, AND ALL OTHERS, TEMPORARY AND PERMANENT LOADS.
  - ALL STUD TO TRACK CONNECTIONS, CLIPS, FASTENERS, BRIDGING AND RELATED ACCESSORIES TO BE DESIGNED AND DETAILED FOR ALL LOADING CONDITIONS INCLUDING NET WIND PRESSURES AND REACTIONS
  - NOTE ANY PROPOSED TRUSS LAYOUT CHANGES THAT WOULD EFFECT THE LOCATION OF BEARING WALLS OR FOUNDATION DESIGN OR CONSTRUCTION.
  - COLD-FORMED STEEL FRAMING SYSTEM PROVIDING CONTINUOUS LOAD PATH TO STRUCTURE.
- MATERIAL SPECIFICATION: UNLESS OTHERWISE NOTED, COLD-FORMED STEEL FRAMING MATERIALS SHALL CONFORM TO ASTM C955 AND SHALL HAVE A MINIMUM YIELD STRENGTH OF 50,000 PSI (16 GAGE AND HEAVIER AND YIELD STRESS OF 33,000 PSI (18 GAGE AND LIGHTER), MEMBERS SHALL BE MANUFACTURED OF SHEET STEEL IN ACCORDANCE WITH ASTM A653 AND ASTM 525, G60 OR G90 GALVANIZED FINISH.
- FRAMING: FRAMING MEMBERS SHALL BE CUT SQUARELY OR AT AN ANGLE AS REQUIRED TO FIT SQUARELY AGAINST ABUTTING MEMBERS, MEMBERS SHALL BE HELD FIRMLY IN PLACE UNTIL PROPERLY JOINED, STUDS SHALL SIT SQUARELY IN THE TOP AND BOTTOM RUNNER TRACK WITH FIRM ABUTMENT AGAINST TRACK WEBS. STUDS SHALL BE ALIGNED OR PLUMBED AND SECURELY FASTENED TO THE FLANGES OF BOTH TOP AND BOTTOM RUNNER TRACK. DO NOT SPLICE MEMBERS UNO.
- CONNECTIONS: JOINING OF STRUCTURAL MEMBERS SHALL BE MADE WITH METAL SCREWS OR WELDING, WIRE TYING OF FRAMING MEMBERS IN STRUCTURAL APPLICATIONS SHALL NOT BE PERMITTED. ATTACHMENT OF COLLATERAL MATERIALS TO STEEL MEMBERS SHALL BE MADE WITH SELF-DRILLING METAL SCREWS OR HARDENED SCREW SHANK NAILS. METAL LATH MAY ALSO BE CONNECTED TO STEEL BY STAPLES OR OTHER FASTENERS, IF APPROVED BY THE FLORIDA BUILDING CODE
- METAL SCREWS: METAL SCREWS SHALL BE MINIMUM #12 SELF DRILLING AND SELF-TAPPING UNO. METAL SCREWS SHALL BE CADMIUM PLATED FOR ALL EXTERIOR USES.
- WELDING: WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3 USING E70 ELECTRODES. ALL WELDS SHALL BE TREATED WITH A ZINC-RICH PROTECTIVE PAINT FOR CORROSION RESISTANCE.
- DEEL ECTION CRITERIA
  - EXTERIOR WALLS: L/240 FOR BRITTLE FINISHES (OUT-OF-PLANE EXTERIOR WALLS: H/180 FOR BRITTLE FINISHES (DRIFT)
- VERTICAL DEFLECTIONS SHALL BE IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE.

# **COLD-FORMED STEEL DECK:**

DECK TYPE DESIGN THICKNESS MOMENT OF INERTIA I

- CORRUGATED STEEL DECK SHALL BE GALVANIZED (G60) TO REMAIN PERMANENTLY IN CONDITIONED SPACES, GALVANIZED (G90) TO REMAIN PERMANENTLY IN LINCONDITIONED SPACES OR PERMANENTLY EXPOSED TO WEATHER. PROVIDE DECK OF GAUGE AND DEPTH INDICATED ON THE PLANS, CONFORMING TO AISI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL HAVING MINIMUM YIELD STRENGTH OF 33 KSI AND MEETING THE MINIMUM
- A. COMPOSITE STEEL DECK

	a.	1.5VL20	0.0358 IN	0.186 IN^4/FT	0.231 IN <sup>3</sup> /FT	0.224 IN^3/FT
	b.	1.5VL18	0.0474 IN	0.272 IN^4/FT	0.324 IN <sup>3</sup> /FT	0.311 IN^3/FT
	c.	1.5VL16	0.0598 IN	0.373 IN^4/FT	0.411 IN <sup>3</sup> /FT	0.404 IN^3/FT
•		K TYPE B22	DESIGN THICKNESS  0.0295 IN	MOMENT OF INERTIA I	MODULUS Sn 0.186 IN^3/FT	MODULUS Sp 0.192 IN^3/FT
	b.	B20	0.0358 IN	0.201 IN^4/FT	0.234 IN^3/FT	0.247 IN^3/FT
	c.	B18	0.0474 IN	0.289 IN^4/FT	0.318 IN^3/FT	0.327 IN^3/FT

- PROVIDE A MINIMUM 1 1/2" BEARING ONTO ALL STEEL SUPPORTS. REFER TO THE PLAN NOTES FOR EFFECTIVE DIAMETER ARC PUDDLE WELD ATTACHMENT TO STEEL SUPPORTS IN ACCORDANCE WITH WITH ANSI/AWS D1.3 AND PATTERN. BUTTON PUNCHING OF SIDE LAPS IS NOT PERMITTED. DECK SHALL BE FREE OF STANDING WATER DURING
- PROVIDE CLOSURE STRIPS (FILLER DECKING) BETWEEN DECKING AND STEEL FRAMING MEMBERS PARALLEL TO DECK SPAN AND STEEL BUTT STRIP AT ALL LOCATION WHERE DECK CHANGES DIRECTIONS MATCHING THE GAUGE AND GALVANIZED COATING OF THE STEEL DECK.
- PROVIDE A EPOXY POLYAMIDE PRIMER AND TWO COATS OF 3 MILS OF EPOXY POLYAMIDE PAINT COMPATIBLE WITH GALVANIZED COATINGS ON ALL SURFACES FOLLOWING ALL WELDMENTS. DO NOT PAINT SURFACES TO RECEIVE SPRAY-ON FIRE RESISTANT MATERIAL
- DO NOT SUSPEND LOADS FROM ANY NON-COMPOSITE COLD FORMED STEEL ROOF DECK. HANGERS OF ALL SUSPENDED UTILITIES AND OTHER LOADS SHALL ATTACH TO STEEL BEAMS, GIRDERS, JOISTS, AND OTHER ISOLATED
- ERECTED COLD FORMED STEEL DECK SHALL BE INSPECTED BY QUALIFIED, INDEPENDENT INSPECTORS. FIELD INSPECTION REPORTS SHALL BE FILED WITH THE ENGINEER OF RECORD WITHIN 5 DAYS OF ACTUAL INSPECTION
- NO CONDUIT SHALL BE PLACED HORIZONTALLY WITHIN THE SLAB OF A COMPOSITE STEEL DECK. REFER TO THE TYPICAL COMPOSITE SLAB PENETRATION DETAIL FOR MORE INFORMATION.

# PRE-MANUFACTURED CFS TRUSSES:

- CONTRACTOR IS TO PROVIDE SHOP FABRICATED COLD-FORMED STEEL TRUSSES DESIGNED BY THE MANUFACTURER'S SPECIALTY ENGINEER LICENSED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED.
- THE MANUFACTURER SHALL HAVE A MINIMUM OF 5 YEARS OF EXPERIENCE FABRICATING CFS TRUSSES.
- THE ENGINEER SHALL HAVE A MINIMUM OF 5 YEARS OF EXPERIENCE DESIGNING CFS TRUSSES.
- 4. ALL TRUSS MEMBERS SHALL BE FABRICATED FROM GALVANIZED SHEET METAL
  - TOP & BOTTOM CHORD MEMBERS SHALL HAVE A MINIMUM THICKNESS OF 63 MILS (16 GA.) WEB MEMBERS SHALL HAVE A MINIMUM THICKNESS OF 47 MILS (18 GA.)
- TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED AND PROVIDED BY THE TRUSS MANUFACTURER.
- THE CES TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND A LETTER OF CERTIFICATION FOR REVIEW AND APPROVAL BY THE ARCHITECT/ENGINEER OF RECORD PRIOR TO CFS TRUSS FABRICATION. SHOP DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. THE SHOP DRAWINGS SHALL INCLUDE ERECTION DRAWINGS SHOWING THE PLACEMENT OF EACH TRUSS IN RELATION TO OTHER STRUCTURAL MEMBERS. A SHOP DRAWING SHALL BE SUBMITTED FOR EACH TRUSS PROFILE DESIGNATED ON THE ERECTION DRAWINGS AND SHALL INCLUDE THE PROJECT NAME, PROJECT NUMBER, DESIGN LOADS, AND REACTIONS AS WELL AS THE MATERIAL PROPERTIES, SIZES, AND GAUGES FOR EACH TRUSS COMPONENT
- SHOP DRAWINGS SHALL ALSO SHOW CONNECTION ASSEMBLIES WITH FASTENERS, AND ACCESSORIES.
- THE CFS TRUSS MANUFACTURER SHALL SUBMIT A CALCULATION PACKAGE BEARING THE SEAL AND SIGNATURE OF A REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. THE PACKAGE SHALL INCLUDE CALCULATIONS FOR ALL TRUSSES, CONNECTIONS, AND ACCESSORIES INDICATING DESIGN LOADS AND MATERIAL PROPERTIES FOR EACH TRUSS.
- SEE ROOF FRAMING PLANS AND ARCHITECTURAL DRAWINGS FOR EXTENTS OF CFS TRUSSES AS WELL AS SLOPES, LOCATIONS OF EAVES, RIDGES, HIPS, AND VALLEYS.
- TRUSSES SHALL BE MANUFACTURED FABRICATED AND SHIPPED TO THE SITE IN THE MAXIMUM LENGTHS AND
- ALL CFS TRUSSES SHALL BE TEMPORARILY BRACE AND PERMANENTLY BRACED AS REQUIRED BY THE TRUSS MANUFACTURER. BRACING AND BRIDGING SHALL BE DESIGNED AND PROVIDED BY THE MANUFACTURER
- 12 FIELD FARRICATION OF CES TRUSSES SHALL NOT BE PERMITTED.
- 13. DELIVER AND HANDLE TRUSSES PER MANUFACTURER'S RECOMMENDATIONS.
- 14. TRUSSES SHALL NOT BE STORED ON SITE.
- 15 TOP CHORD:
  - LIVE LOAD 20 PSF b. DEAD LOAD 10 PSF
- 16. BOTTOM CHORD
  - LIVE LOAD DEAD LOAD

### WOOD FRAMING:

MODULUS Sp

MODULUS Sr

- ALL CEILING AND WALL NAILERS SHALL BE PRESSURE TREATED AND HAVE THE MINIMUM PROPERTIES OF SOUTHERN PINE #2, ALLOWABLE STRESSES PER THE NATIONAL DESIGN SPECIFICATION SUPPLEMENT 2009 EDITION, 15% MAX MOISTURE
- ALL 2X STUD FRAMING SHALL HAVE THE MINIMUM PROPERTIES OF DOUGLAS FIR #2, ALLOWABLE STRESSES PER THE NATIONAL DESIGN SPECIFICATION SUPPLEMENT 2001 EDITION, 19% MAX MOISTURE CONTENT. PROVIDE SOLID BLOCKING BETWEEN STUDS AT 4'-6" ON CENTER VERTICAL (MAX.).
- ALL DIMENSION LUMBER SHALL BE GRADE STAMPED PER W.C.L.B. RULES
- ROOF DECK AND WALL SHEATHING SHALL BE 19/32" OSB, STRUCTURAL GRADE
- ROOF SHEATHING SHALL BE ORIENTED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS. ATTACH PLYWOOD TO FRAMING WITH [10D] NAILS AT 6" O.C. AT PANEL PERIMETER AND AT 8" O.C. AT INTERMEDIATE SUPPORTS. PROVIDE PANEL CLIPS AT MIDSPAN BETWEEN SUPPORTS AND SOLID BLOCKING AT ALL PANEL EDGES.
- WALL SHEATHING SHALL BE APPLIED DIRECTLY TO WALL FRAMING WITH 8D NAILS AT 6" O.C. AT PANEL PERIMETER AND 12" O.C. AT INTERMEDIATE SUPPORTS. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES. PROVIDE DOUBLE STUDS AND GALVANIZED HOLDOWNS AT EACH END OF SHEARWAI

#### **GLUED – LAMINATED WOOD:**

- CONTRACTOR IS TO PROVIDE FACTORY GLUED-LAMINATED STRUCTURAL UNITS, CONNECTIONS, AND ACCESSORIES PRODUCED BY AN AITC-LICENSED FABRICATOR QUALIFIED TO DESIGNATE PRODUCTS WITH AITC QUALITY INSPECTED MARK. FABRICATOR SHALL BE AISC CERTIFIED FOR STEEL
- . AS INDICATED IN AITC 110 USE ARCHITECTURAL APPEARANCE GRADE IN EXPOSED AREAS AND FRAMING APPEARANCE GRADE FOR ALL OTHER AREAS.
- FOR GLULAM BEAMS AND PURLINS PRIMARILY IN BENDING USE THE FOLLOWING MINIMUM STRENGTH PROPERTIES:
  - 24F-F4-1 8F
  - Fb = 2400 psi
  - Fv = 300 psi
- E = 1,800,000 ps
- CAMBER BEAMS AS DESIGNATED ON THE DRAWINGS IF NO CAMBER IS INDICATED PROVIDE CAMBER TO THE AMOUNT OF DEFLECTION DUE TO THE DEAD LOAD OF THE MEMBER ONLY
- FOR GLULAM COLUMNS PRIMARILY IN COMPRESSION USE THE FOLLOWING MINIMUM STRENGTH PROPERTIES
- 49-SP-N1M16
- Fc = 2100 psi Ft = 1350 psi
- Fby = 1950 psi Fbx = 1800 psi
- Fvy = 260 psi
- Fvx = 300 psi E = 1,700,000 ps
- CONNECTORS, ANCHORS, ACCESSORIES SHALL BE STEEL FABRICATED FROM ASTM A36 (MINIMUM Fy = 36 ksi) ROLLED SHAPES, PLATES, AND BARS. ASSEMBLIES SHALL BE MANUFACTURER'S STANDARD TYPES FOR THE GLULAM SIZES INDICATED.
- THE GLULAM MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND A LETTER OF CERTIFICATION FOR REVIEW AND APPROVAL BY THE ARCHITECT/ENGINEER OF RECORD PRIOR TO GLULAM FABRICATION. SHOP DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A REGISTERED PROFESSIONAL ENGINEER IN TH STATE WHERE THE PROJECT IS LOCATED. SHOP DRAWINGS SHALL INCLUDE THE PROJECT NAME PROJECT NUMBER, AND DESIGN LOADS. SHOP DRAWINGS SHALL INCLUDED THE MATERIAL PROPERTIES FOR EACH MEMBER, MEMBER SIZES, CONNECTION ASSEMBLIES WITH FASTENERS, AND ACCESSORIES.
- THE GLULAM MANUFACTURER SHALL SUBMIT A CALCULATION PACKAGE BEARING THE SEAL AND SIGNATURE OF A REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. THE PACKAGE SHALL INCLUDE CALCULATIONS FOR ALL FRAMING MEMBERS. CONNECTIONS AND ACCESSORIES INDICATING DESIGN LOADS AND MATERIAL PROPERTIES FOR

### SUBMITTALS:

- THE GENERAL CONTRACTOR SUBMITTALS FOR ENGINEER REVIEW ARE AS
  - STRUCTURAL STEEL (\*
  - NON-COMPOSITE METAL DECK COMPOSITE METAL DECK

  - COMPOSITE SHEAR CONNECTORS PRE-ENGINEERED METAL BUILDING (#)
  - PRE-ENGINEERED HANGAR DOOR (#)
  - PRE-ENGINEERED ALUMINUM CANOPY (#)
  - PRE-ENGINEERED ALUMINUM CANOPY FOUNDATIONS (#)
    PRE-ENGINEERED METAL OVERHANG SUNSHADE (#)
  - LIGHT GAUGE METAL FRAMING (#)
  - LIGHT GAGE METAL TRUSSES (#)
- PRE-ENGINEERED WOOD TRUSSES (#) CONCRETE MIX DESIGNS
- MASONRY UNITS AND MORTAR
- REINFORCING STEEL
- CHEMICAL ADHESIVE ANCHORS
- CONSTRUCTION JOINT LOCATIONS IN STRUCTURAL FLOORS
- HOLLOW-CORE PLANKS (\*)
  CONCRETE REPAIR, CURING, AND SEALER PRODUCTS
- FORMWORK, SHORING, RESHORING (#
- ITEMS MARKED (\*) SHALL REQUIRE SUBMITTAL OF SHOP DRAWINGS PREPARED LINDER THE DIRECT SUPERVISION AND REGISTERED IN THE STATE OF FLORIDA FOR ENGINEER OF RECORD'S REVIEW. GENERIC PRODUCTS WILL NOT BE ACCEPTED
- ITEMS MARKED (#) SHALL REQUIRE SUBMITTAL OF SHOP DRAWINGS AND CALCULATIONS PREPARED UNDER THE DIRECT SUPERVISION AND SIGNED AND SEALED BY A PROFESSIONAL DELEGATED ENGINEER REGISTERED IN THE STATE OF FLORIDA FOR ENGINEER OF RECORD'S REVIEW.
- ENGINEER OF RECORD REVIEW OF SUBMITTALS MARKED (\*) OR (#) WILL NOT BEGIN UNTIL THE SUBMITTAL IS SIGNED AND SEALED BY
- THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER
- SUBMITTALS SHALL CLEARLY IDENTIFY THE PROJECT NAME, SPECIFIC PRODUCT UTILIZED, APPLICABLE CODES, DESIGN CRITERIA, AND SHOW ALL DETAILS AND PLANS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. HAND-WRITTEN COMMENTS AND/OR MARKINGS ON THE SUBMITTALS BY THE CONTRACTOR SHALL BE MADE USING A GREEN COLOR PEN AND THE ENGINEER/ARCHITECT SHALL UTILIZE A RED COLOR PEN.
- SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR FOR FULL COORDINATION OF ALL CONSTRUCTION TRADES WITH THE LATEST DESIGN DISCIPLINES DOCUMENTS, REVISIONS/CLARIFICATIONS, AND RESPONSES TO RFIs. SHOP DRAWINGS SHALL BE MARKED BY THE GENERAL CONTRACTOR "APPROVED" PRIOR TO SUBMITTING TO THE OWNER. ARCHITECT OF RECORD, OR ENGINEER OF RECORD. GENERAL CONTRACTOR-GENERATED QUESTIONS OR REQUESTS FOR INFORMATION WITHIN THE SUBMITTALS SHALL BE CLEARLY MARKED. CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF THE RE-SUBMITTALS SHALL BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ARCHITECT/ENGINEER REVIEW WILL BE LIMITED TO ONLY THOSE ITEMS CAUSING THE RE-SUBMITTAL
- SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS AND PRODUCTS FUNCTIONAL EQUIVALENCE ONLY. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.
- A MINIMUM OF 10 WORKING DAYS SHALL BE ALLOWED FOR THE ENGINEER TO REVIEW A SUBMITTAL
- NON-CONFORMING SUBMITTALS WILL BE RETURNED WITHOUT REVIEW ARCHITECT/ENGINEER WILL NOT BE RESPONSIBLE FOR SCHEDULE DELAYS CAUSED BY RETURNED SUBMITTALS THAT FAILED TO MEET THE AFOREMENTIONED CRITERIA

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NOTES GENERAL

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AIRPORT TERMINAL AND HANGAR DEVELOPMENT

DESIGNED BY: AJM CHECKED BY: PROVED BY ROJECT NO: 2019.028.01 DATE: 11/1/2021

SHEET NUMBER

S1.01

FOR PERMIT & BIDDING ONLY

THE FOLLOWING SUPERIMPOSED LOADS HAVE BEEN UTILIZED IN ACCORDANCE WITH THE MOST RECENT PRINTING/ERRATA OF ASCE 7-16 'MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES'.

1 LIVELOADS

A. ROOF 20 PSI

DEAD LOADS:

METAL ROOF STRUCTURE W/ INSULATION 15 PSF 5 PSF CEILING

# **COMPONENTS AND CLADDING WIND PRESSURES (PSF)**

	LOCATION	ZONE			EFFECTIV	E WIND AREA (A	ι), FΤ <sup>2</sup>		
	LOCATION	ZONE	A = 2	A = 10	A = 20	A = 50	A = 100	A = 250	A = 500
		1	+29.8,-73.9	+24.3,-73.9	+21.9,-73.9	+18.7,-45.0	+16.3,-23.1	+16.3,-23.1	+16.3,-23.1
ן (		1-OH	-90.9	-90.9	-90.9	-71.6	-57.0	-57.0	-57.0
5		2e	+29.8,-73.9	+24.3,-73.9	+21.9,-73.9	+18.7,-45.0	+16.3,-23.1	+16.3,-23.1	+16.3,-23.1
-		2e-OH	-90.9	-90.9	-90.9	-71.6	-57.0	-57.0	-57.0
5		2n	+29.8,-107.8	+24.3,-107.8	+21.9,-93.2	+18.7,-73.9	+16.3,-59.3	+16.3,-40.0	+16.3,-40.0
į 🛮	ROOF	2n-OH	-124.8	-124.8	-113.8	-99.3	-88.4	-73.9	-73.9
	ROOF	2r	+29.8,-107.8	+24.3,-107.8	+21.9,-93.2	+18.7,-73.9	+16.3,-59.3	+16.3,-40.0	+16.3,-40.0
		2r-OH	-124.8	-124.8	-113.8	-99.3	-88.4	-73.9	-73.9
		3e	+29.8,-107.8	+24.3,-107.8	+21.9,-93.2	+18.7,-73.9	+16.3,-59.3	+16.3,-40.0	+16.3,-40.0
		3e-OH	-145.1	-145.1	-126.1	-101.0	-82.0	-57.0	-57.0
		3r	+29.8,-128.1	+24.3,-128.1	+21.9,-109.8	+18.7,-85.5	+16.3,-67.1	+16.3,-67.1	+16.3,-67.1
		3r-OH	-165.4	-165.4	-140.9	-108.6	-84.1	-84.1	-84.1
	WALLS	4	+40.0,-43.4	+40.0,-43.4	+38.2,-41.6	+35.8,-39.2	+34.0,-37.4	+31.6,-35.0	+29.8,-33.2
	WALLS	5	+40.0,-53.6	+40.0,-53.6	+38.2,-50.0	+35.8,-45.2	+34.0,-41.6	+31.6,-36.8	+29.8,-33.2

	LOCATION	7015	EFFECTIVE WIND AREA (A), FT <sup>2</sup>							
≿	LOCATION	ZONE	A = 7	A = 10	A = 50	A = 100	A = 200	A = 500		
LOBBY		1	+34.9,-62.6	+34.9,-62.6	+23.8,-46.0	+19.0,-38.8	+19.0,-38.8	+19.0,-38.8		
O.		2e	+34.9,-86.4	+34.9,-86.4	+23.8,-65.1	+19.0,-55.9	+19.0,-46.7	+19.0,-46.7		
	ROOF	2e-OH	-106.2	-104.1	-94.6	-90.5	-86.4	-86.4		
≰	KOOF	2r	+34.9,-86.4	+34.9,-86.4	+23.8,-65.1	+19.0,-55.9	+19.0,-46.7	+19.0,-46.7		
<b>\{</b>		3e	+34.9,-86.4	+34.9,-86.4	+23.8,-65.1	+19.0,-55.9	+19.0,-46.7	+19.0,-46.7		
TERMINAL		3e-OH	-129.9	-123.2	-92.8	-79.7	-66.6	-66.6		
	WALLS	4	+46.7,-50.7	+46.7,-50.7	+41.9,-45.8	+39.8,-43.7	+37.6,-41.6	+34.9,-38.8		
	WALLS	5	+46.7,-62.6	+46.7,-62.6	+41.9,-52.8	+39.8,-48.6	+37.6,-44.4	+34.9,-38.8		

LOCATION	ZONE			EFFECTIV	E WIND AREA (A	A), FT <sup>2</sup>			
LOCATION		A = 10	A = 20	A = 50	A = 100	A = 200	A = 500	A = 1000	
	1	+18.9,-74.0	+17.7,-69.1	+16.1,-62.6	+16.0,-57.8	+16.0,-52.9	+16.0,-46.4	+16.0,-46.4	
2005	1'	+18.9,-42.5	+17.7,-42.5	+16.1,-42.5	+16.0,-42.5	+16.0,-36.6	+16.0,-28.7	+16.0,-22.8	
ROOF	2	+42.5,-97.6	+40.6,-91.3	+38.1,-83.0	+36.2,-76.7	+34.4,-70.5	+31.9,-62.2	+31.9,-62.2	
	3	+42.5,-97.6	+40.6,-91.3	+38.1,-83.0	+36.2,-76.7	+34.4,-70.5	+31.9,-62.2	+31.9,-62.2	
WALLS	4	+42.5,-46.0	+40.6,-44.1	+38.1,-41.7	+36.2,-39.8	+34.4,-37.9	+31.9,-35.4	+31.9,-35.4	
WALLS	5	+42.5,-56.7	+40.6,-52.9	+38.1,-47.9	+36.2,-44.1	+34.4,-40.4	+31.9,-35.4	+31.9,-35.4	

#### NOTES:

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HAN

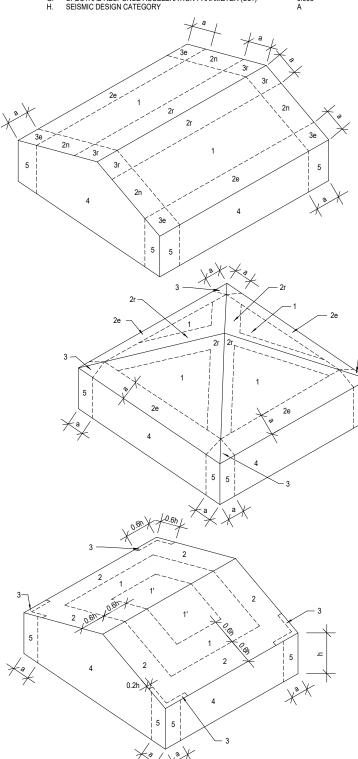
- EFFECTIVE WIND AREA IS SUCH AS DEFINED BY ASCE 7 FOR EFFECTIVE WIND AREAS BETWEEN THOSE GIVEN ABOVE, THE PRESSURE MAY BE INTERPOLATED, OTHERWISE USE PRESSURE WITH THE LOWER EFFECTIVE AREA.
- WIND PRESSURES ARE GROSS POSITIVE AND NEGATIVE ULTIMATE VALUES CALCULATED WITH THE ULTIMATE WIND SPEED (Vult) PER THE GENERAL NOTES DESIGN LOADS CRITERIA. FOR ALLOWABLE WIND PRESSURES USING NOMINAL WIND SPEED (Vasd), MULTIPLY THE ABOVE VALUES BY 0.6.
- POSITIVE (+) WIND PRESSURE INDICATES TOWARDS THE SURFACE, NEGATIVE (-) WIND PRESSURE INDICATES AWAY FROM THE SURFACE
- "XX-OH" INDICATES OVERHANG WIND PRESSURE.
- a = 6.2 FT (OFFICE); a = 4.4 FT (LOBBY); a =10.0 FT, h = 32.0 FT (HANGAR)

#### 3. WIND DESIGN DATA:

A.	ULTIMATE WIND SPEED (Vult)	135 MPH
B.	NOMINAL WIND SPEED (Vasd)	105 MPH
C.	RISK CATEGORY	II
D.	EXPOSURE CATEGORY	С
E.	INTERNAL PRESSURE COEFF. (GCpi)	+/- 0.18 (ENCLOSED)
F.	COMPONENTS AND CLADDING PRESSURE	SEE WIND PRESSURE PLAN

#### 4. SEISMIC DESIGN DATA:

A.	RISK CATEGORY	1
B.	SEISMIC IMPORTANCE FACTOR	1.0
C.	MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER (Ss)	0.075
D.	MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER (S1)	0.005
E.	SITE CLASS	D
F.	SPECTRAL RESPONSE ACCELERATION PARAMETER (Sds)	0.08
G.	SPECTRAL RESPONSE ACCELERATION PARAMETER (Sd1)	0.008



### PRE-ENGINEERED METAL BUILDING:

- THE PRE-ENGINEERED METAL RUILDING (PEMB) SHALL CONSIST OF ROOF DECK, RIGID FRAMES, METAL WALL PANELS ON FRAMING CANOPY FRAMING, GUTTERS AND DOWNSPOUTS, AND FLASHING. DEVIATION FROM BAY SPACING SHOWN ON THE PLANS SHALL NOT BE PERMITTED TO SUIT MANUFACTURER'S STANDARDS. THE PEMB MANUFACTURER SHALL BE A MEMBER OF THE METAL BUILDING MANUFACTURER'S ASSOCIATION (MBMA)
- THE SYSTEM SHALL BE DESIGNED AND DETAILED BY THE MANUFACTURER TO SUSTAIN THE DESIGN LOADS SPECIFIED IN 'DESIGN LOADS' GENERAL NOTES, INCLUDING WIND LOADS. REFER TO OTHER DESIGN DISCIPLINE DRAWINGS FOR OTHER LOADS NOT INDICATED HEREIN SUCH AS BUT NOT LIMITED TO HANGAR SWINGING DOORS, HANGAR BI-PARTING ROLLING DOORS, HANGAR FLOATING ROLLING DOORS, SUSPENDED FANS, WALL-MOUNTED FANS, STAIRS, CRANE RAILS, EQUIPMENT, CURTAINS, CURTAIN WALL FRAMING, OPERABLE PARTITIONS, OVERHEAD FOLDING DOORS, ETC. THE DESIGN SHALL BE IN ACCORDANCE WITH THE LATEST ISSUES OF THE AISC AND AISI SPECIFICATIONS AND MBMA 'METAL BUILDING SYSTEMS MANUAL' DESIGN PRACTICES.
- COLUMNS SHALL BE DESIGNED AS UNBRACED BY THE MASONRY WALLS. ATTACHMENT OF PEMB COLUMNS TO WALLS SHALL BE MADE AFTER ROOF DEAD LOADS ARE APPLIED.
- PEMB SHOP DRAWINGS AND A LETTER OF CERTIFICATION SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER. SHOP DRAWINGS SHALL INDICATE THE DESIGN LOADS AND JOB NAME AND NUMBER. THEY SHALL INCLUDE SIZES OF ALL THE FRAMING MEMBERS AND RELATED ACCESSORIES AND SPECIALIZED CONNECTIONS, THE ANCHOR BOLT PLAN AND REACTIONS. STANDARD CUT SHEETS OF THE ABOVE ARE NOT ACCEPTABLE. STANDARD CUT SHEETS MAY BE SUBMITTED FOR SECONDARY FRAMING CONNECTION DETAILS, FLASHING AND SHEETING DETAILS, ETC. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR REVIEW AND APPROVAL IN WRITING OF ANY INTERIM AND ALL FINAL PEMB SHOP DRAWINGS TO ASSURE THEY MATCH THE CONCEPT DESIGN INTENT PRIOR TO FORWARDING ONTO ENGINEER FOR REVIEW.
- THE MANUFACTURER SHALL PROVIDE SIGNED AND SEALED CALCULATIONS FOR ALL FRAMING MEMBERS, RELATED ACCESSORIES, AND SPECIALIZED CONNECTIONS PREPARED UNDER THE SUPERVISION OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER. THE SUBMITTAL SHALL INCLUDE THE LOAD COMBINATION REACTIONS FOR EACH COLUMN SUPPORT TO THE ENGINEER OF RECORD PRIOR TO THE FINAL FOUNDATION PRICING PACKAGE SUBMITTAL. IF THE MANUFACTURER DOES NOT COMPLY. THE FOUNDATION DESIGN WILL BE ASSUMED TO RESIST THE COMBINED GRAVITY AND WIND LOADS SHOWN ON PLAN. IF THE PEMB MANUFACTURER'S REACTION BECOME AVAILABLE AFTER THE OWNER'S FINAL NEGOTIATED COST AND THE ASSUMED FOUNDATION REQUIRE LARGER FOUNDATIONS, THE INCREASE IN FOUNDATION SIZE SHALL BE PROVIDED AT NO COST TO THE OWNER OR THE ENGINEER OF RECORD
- PEMB ANCHOR BOLT DIAMETER SHALL BE DETERMINED BY METAL BUILDING MANUFACTURER. LENGTH AND TYPE OF BOLT REQUIRED SHALL BE NOTED ON THESE DRAWINGS.

### **POST-INSTALLED ANCHORS:**

- POST-INSTALLED CONCRETE ANCHORS SHALL BE AS INDICATED ON THE DRAWINGS. THE ANCHOR TYPES AND CONDITIONS LISTED BELOW ARE THE DESIGN AND DETAILING BASIS OF ALL POST-INSTALLED ANCHORAGE IN THE CONTRACT DOCUMENTS
  - ANCHORAGE TO CONCRETE (CRACKED) ELEMENTS USING ADHESIVE ANCHOR
    - FOR FAST CURE APPLICATIONS:
      - HILTI HIT-HY 200 MAX-A (ICC ESR-3187) WITH CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STÀINLESS STEEL) OR CONTINUOUSLY DEFORMED REINFÒRCING STEEL.
    - FOR SLOW CURE APPLICATIONS:
    - HILTI HIT-RE 500-V3 (ICC ESR-3814) WITH HILTI HIS-N OR HIS-RN INTERNALLY THREADED INSERTS. CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL) OR CONTINUOUSLY
    - SIMPSON STRONG-TIE SET-XP (ICC-ES ESR-2508) CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL) OR CONTINUOUSLY DEFORMED REINFORCING STEEL
- ANCHORAGE TO SOLID (UNCRACKED) GROUTED MASONRY/MULTI-WYTHE BRICK USING ADHESIVE ANCHOR
  - HILTI HIT-HY 70 (ICC ESR-2682) WITH HILTI HAS-E CONTINUOUSLY THREADED RODS, CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL), OR CONTINUOUSLY DEFORMED STEEL
  - SIMPSON STRONG-TIE SET (ICC-ES ESR-1772) WITH (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL), OR CONTINUOUSLY DEFORMED STEEL REBAR.
- ANCHORAGE TO HOLLOW MASONRY USING ADHESIVE ANCHOR
  - SIMPSON STRONG-TIE SET (ICC-ES ESR-1772) WITH CONTINUOUSLY THREADED RODS (ASTM F1554 Gr36FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL)
  - HILTI HIT-HY 70 (ICC ESR-3342) WITH HILTI HAS-E CONTINUOUSLY THREADED RODS, CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL), OR CONTINUOUSLY DEFORMED STEEL
- D. ANCHORAGE USING EXPANSION BOLTS
  - a. HILTI KWIK BOLT 3 (ICC ESR-2302 IN CONCRETE), (ICC ESR-1385 IN GROUT FILLED CMU) CARBON STEEL, UNO.
- ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIC DRODUCT LISTED ABOVE FOR EACH OF THE SPECIFIC DRODUCT TO STATULED IN THE PLANS. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC-ES AC58, ICC-ES AC60, AND/OR ICC-ES AC308 SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE, AND INSTALLATION TEMPERATURE.
- INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING,
- OVERHEAD ADHESIVE ANCHORS MUST BE INSTALLED USING THE HILTI PROFIS SYSTEM
- THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS
- ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
- EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY FERROSCAN GPR X-RAY CHIPPING OR OTHER MEANS.

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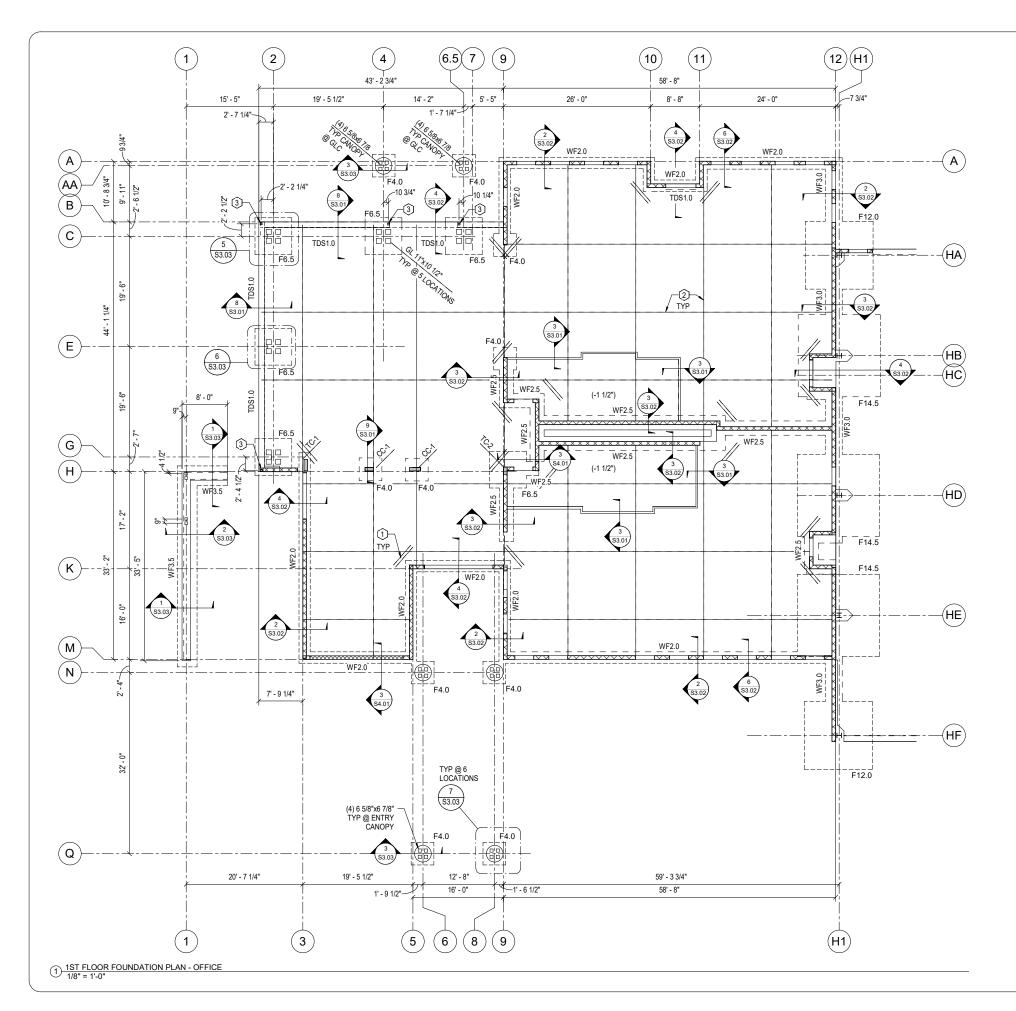
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AIRPORT TERMINAL AND HANGAR DEVELOPMENT

DESIGNED BY: AJM CHECKED BY: PROVED BY PROJECT NO: 2019.028.01 DATE: 11/1/2021

SHEET NUMBER

S1.02



#### FOUNDATION PLAN NOTES

- FLOOR SLAB CONSTRUCTION SHALL BE 4" THICK 3000 PSI CONCRETE SLAB-ON-GRADE REINFORCED WITH W2.0 x W2.0 - 6 x 6 WWF PLACED IN THE TOP ONE-THIRD OF THE SLAB
  - SLAB-ON-GRADE CONSTRUCTION, SEE DETAILS 1 THRU 3 /S3.01
  - T/SLAB EL = 0'-0" REFER TO CIVIL DWGS FOR REFERENCE ELEVATION REFERENCE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR
  - FLOOR SLOPES, DRAINS, AND DEPRESSION LOCATIONS.
- MAINTAIN SLAB THICKNESS AT ALL FLOOR SLOPES, DRAINS, AND DEPRESSIONS.
- REFERENCE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR CONCRETE EQUIPMENT PAD LOCATIONS, SEE DETAIL 4/S3.01
- FOOTING CONSTRUCTION SHALL BE AS INDICATED ON THE DRAWING
- a. SEE FOUNDATION SCHEDULE ON SHEET S5.06 FOR THE FOLLOWING:
  - 'WFX.X' INDICATES WALL FOOTING
  - 'FX.X' INDICATES SPREAD FOOTING 'TDSX.X' INDICATES TURN DOWN SLAB
- T/WALL FTG EL = -1-4" UNO
- T/SPREAD FTG EL = -1-4" UNO CONDUIT AND PIPE PENETRATIONS: STEP AND/OR LOWER FOUNDATIONS WHERE SHOWN AND AS NECESSARY TO AVOID INTERFERENCE WITH OTHER TRADES. SEE FOUNDATION DETAILS 7/S3.01 AND 1/S3.02
- FOUNDATION REINFORCEMENT AT CORNERS AND INTERSECTIONS: SEE 5/S3.01
- SEE FOUNDATION DETAILS FOR ADDITIONAL INFORMATION.
- 3. MASONRY CONSTRUCTION SHALL BE AS INDICATED ON THE DRAWING
  - ALL CMU WALLS SHALL BE CONSTRUCTED WITH 8 INCH NOMINAL UNITS, UNO
  - COMPREHENSIVE STRENGTH OF CONCRETE MASONRY (fm):
    ALL CMU WALLS SHALL BE CENTERED ON FOOTINGS, UNO

  - - EXTERIOR BRG WALLS: #5 @ 24" OC MAX, TYP, UNO INTERIOR BRG WALLS: #5 @ 24" OC MAX, TYP, UNO
  - ADDITIONAL BAR SIZES AND LOCATIONS ARE INDICATED ON THE DRAWING
  - 'TC-X' INDICATES TIE COLUMN, SEE SCHEDULE S5.06
  - 'CC-X' INDICATES CONCRETE COLUMNS, SEE SCHEDULE \$5.06
- NON-LOAD BRG WALLS ARE NOT SHOWN. SEE ARCH DWGS FOR LOCATIONS OF ALL NON-LOAD BRG WALLS
- SEE TYPICAL MASONRY DETAILS ON SHEETS \$4.01 AND \$4.02 FOR ADDITIONAL INFORMATION AND ADDITIONAL LOCATIONS OF REINFORCEMENT NOT CALLED OUT IN THESE NOTES OR ON THIS PLAN.
- LOAD BEARING WALLS & EXTERIOR WALLS: 2000 PSI
- GENERAL NOTES
  - a. SEE GENERAL NOTES SHEETS \$1.00 & 1.01 FOR ADDITIONAL INFORMATION

# KEYED NOTES:

- RE-ENTRY BARS: PLACE (2) #4 x 4'-0" LONG @ 3" OC AND 3" CLR FROM CORNER AT MID-DEPTH OF THE SLAB. CENTER BARS ON CORNER.
- 2. SLAB ON GRADE CONTROL JOINT (CJ) SEE DETAIL 1/S3.01.
- 3. HSS4x4x1/4", SEE S3.03 FOR BASE PLATE.



N T E R N A T I O N A 4211 W. BOY SCOUT BLVD

Michael

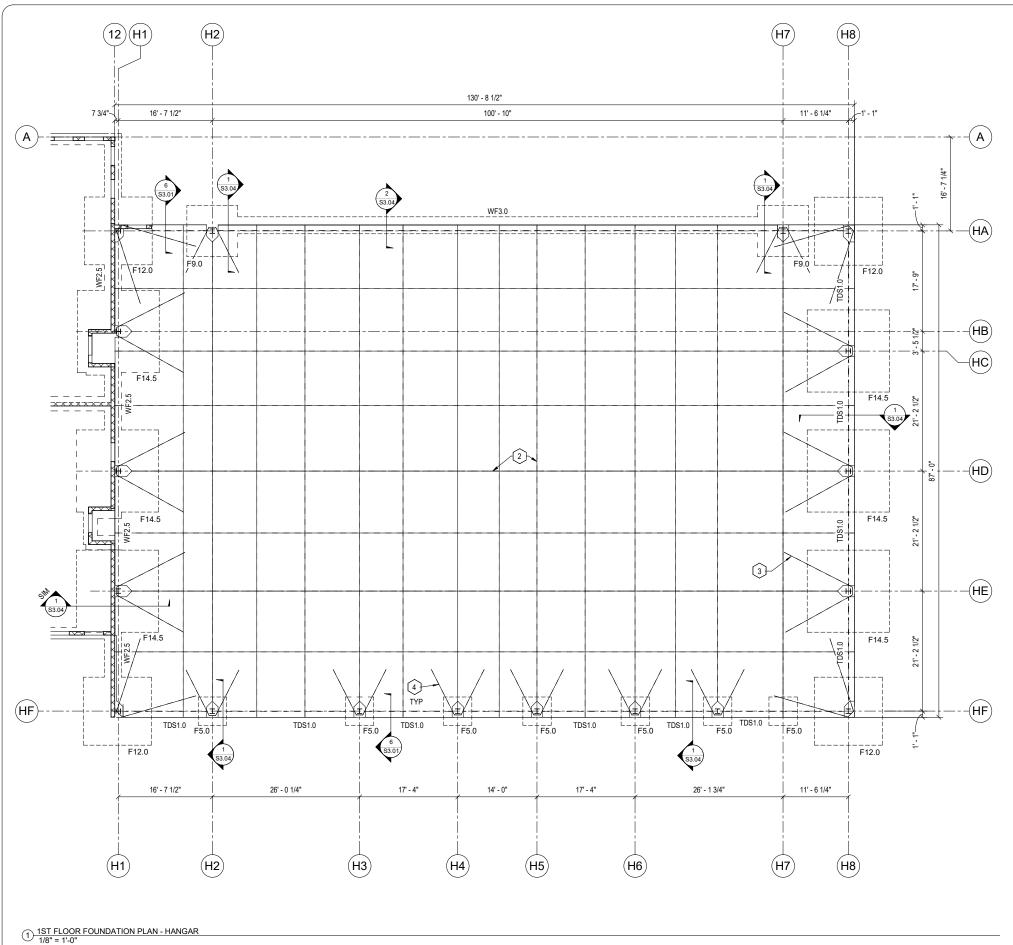
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AIRPORT TERMINAL AND HANGAR DEVELOPMENT

DESIGNED BY: CHECKED BY: PPROVED BY PROJECT NO: 2019.028.01 DATE: 11/1/2021

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#### PRE-ENGINEERED METAL BUILDING FOUNDATION PLAN NOTES

- FLOOR SLAB CONSTRUCTION SHALL BE 8" THICK CONCRETE SLAB-ON-GRADE REINFORCED WITH #5 @ 8" OC, EA WAY PLACED AT MID-DEPTH SLAB AND NOVEMESH
  - SLAB SHALL BE PLACED OVER VAPOR RETARDANT ON COMPACTED SUB-GRADE.
  - SLAB-ON-GRADE CONSTRUCTION, SEE DETAILS 1 THRU 3 /S3.01
  - T/SLAB EL = 0'-0" REFER TO CIVIL DWGS FOR REFERENCE ELEVATION
  - DRAWINGS FOR FLOOR SLOPES, DRAINS, AND DEPRESSION LOCATIONS. MAINTAIN SLAB THICKNESS AT ALL FLOOR SLOPES, DRAINS, AND
  - DEPRESSIONS
  - REFERENCE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR CONCRETE EQUIPMENT PAD LOCATIONS, SEE DETAIL 4/S3.01
- 2. FOOTING CONSTRUCTION SHALL BE AS INDICATED ON THE DRAWING
  - PRELIMINARY REACTIONS HAVE BEEN DEVELOPED BY AVCON, INC FOR INITIAL DESIGN OF THE FOUNDATIONS. GC SHALL EXPECT THE FOUNDATION SIZES AND REINFORCING TO CHANGE ONCE AVCON, INC REVIEWS THE FINAL REACTIONS PREPARED BY THE PRE-ENGINEERED METAL BUILDING MANUFACTURER'S PROFESSIONAL ENGINEER
  - b. SEE FOUNDATION SCHEDULE ON SHEET S5.06 FOR THE FOLLOWING:
    - 'WFX.X' INDICATES WALL FOOTING
    - 'FX.X' INDICATES SPREAD FOOTING 'TDSX.X' INDICATES TURN DOWN SLAB
  - T/WALL FTG EL = -1-4" UNO

  - T/SPREAD FTG EL = -1'-4" UNO CONDUIT AND PIPE PENETRATIONS: STEP AND/OR LOWER FOUNDATIONS WHERE SHOWN AND AS NECESSARY TO AVOID INTERFERENCE WITH OTHER TRADES. SEE FOUNDATION DETAILS 7/S3.01 AND 1/S3.02
  - FOUNDATION REINFORCEMENT AT CORNERS AND INTERSECTIONS: SEE 5/S3.01 SEE FOUNDATION DETAILS FOR ADDITIONAL INFORMATION.
- GENERAL NOTES 3.
  - a. SEE GENERAL NOTES SHEETS \$1.00 & 1.01 FOR ADDITIONAL INFORMATION

### KEYED NOTES:

- RE-ENTRY BARS: PLACE (2) #4 x 4'-0" LONG @ 3" OC AND 3" CLR FROM CORNER AT MID-DEPTH OF THE SLAB
- 2. SLAB ON GRADE CONTROL JOINTS (CJ) SEE DETAIL 1/S3.01
- #5 x 20'-0" HAIRPIN EA LEG, TYP. ALONG GRID LINES H1 & H8 MUST BE PLACED TIGHT TO EXTERIOR COLUMN FLANGE.
- #5 x 12'-0" HAIRPIN EA LEG, TYP. ALONG GRIDLINES HA & HF  $\underline{\text{MUST BE PLACED TIGHT}}$ TO EXTERIOR COLUMN FLANGE.



Michael Baker
I N T E R N A T I O N A L
4211 W. BOY SCOUT BLVD
SUITE 500
TAMPA, FL 33607
FLORIDA LICENSE # AA 26002484

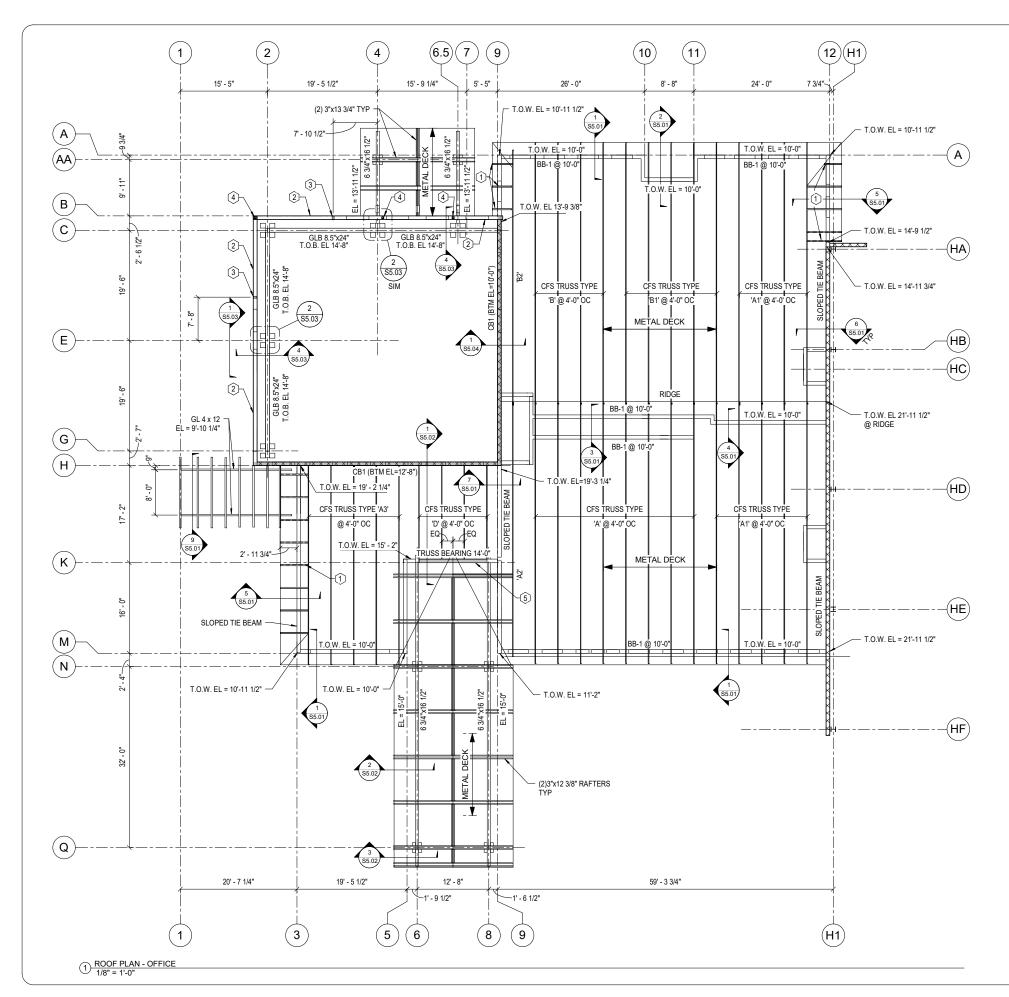
JEFFREY YEAGER, P.E. FL LICENSE NO.: 62853 o S

FOUNDATION PLAN - HANGAR

AIRPORT TERMINAL AND HANGAR DEVELOPMENT

DESIGNED BY: CHECKED BY: PPROVED BY PROJECT NO: 2019.028.01 DATE: 11/1/2021

SHEET NUMBER



#### ROOF PLAN NOTES:

- ROOF CONSTRUCTION SHALL CONSIST OF 1-1/2" x 20 GA. TYPE 'B' GALV G90 METAL DECK ON PRE-ENGINEERED CFS TRUSSES SPACED AT 4'-0" OC, MAX.
- CONNECT METAL DECK TO TRUSSES WITH #12 TEK SCREWS AT 36/7 PATTERN OF THE
- SUPPORTS AND 12" OC AT EXTREME ENDS.

  METAL DECK SIDE LAPS SHALL BE (1) #10 TEK SCREWS BETWEEN EACH SUPPORT.

  DECK FASTENERS SHALL BE CONCEALED AND NOT VISIBLE FROM THE BOTTOM FINISH OF EXPOSED DECK.
- CANOPY ROOF CONSTRUCTION SHALL CONSIST OF 2" x 20 GA. VERSA-DEK LS ON GLULAM
- CONNECT DECK TO GLB'S WITH #12 TEK SCREWS 3" MINIMUM PENETRATION AT 24/4 PATTERN AT SUPPORTS AND 6" OC AT EXTREME ENDS.
  SIDE LAP FASTENERS SHALL BE (2) #10 TEK SCREWS BETWEEN EACH SUPPORT
- DECK FASTENERS SHALL BE CONCEALED AND NOT VISIBLE FROM THE BOTTOM FINISH

#### 3.. CFS TRUSSES

- CFS TRUSSES SHALL BEAR IN MASONRY WALL PROVIDE BEARING STIFFENER AT EACH BEARING LOCATION
  ATTACH CFS TRUSSES TO EXTERIOR MASONRY WALLS AS INDICATED ON THE
- PROVIDE CONTINUOUS CFS BLOCKING / CHORD MEMBERS BETWEEN TRUSSES ON TOP OF BEARING WALLS. BLOCKING DEPTH SHALL MATCH THE DEPTH OF THE TRUSS AT BEARING. THE TOP FLANGE SHALL HAVE A MINIMUM THICKNESS OF 54 MILS AND SHALL BE SLOPED TO MATCH THE PITCH OF THE ROOF TO PROVIDE CONNECTION FOR THE METAL DECK. THE BLOCKING SHALL BE CONNECTED TO THE TOP OF THE WALL TO RESIST AN IN-PLANE LATERAL LRFD LOAD OF 400 PLF.
- PROVIDE CONTINUOUS CLOSURE PLATES WITH A MINIMUM THICKNESS OF 54 MILS AT RIDGE, HIP, AND EAVE LOCATIONS. CONNECT THE PLATES TO SUPPORTING MEMBERS WITH #10 TEK SCREWS OR WELDS SPACED NO MORE THAN 12" OC

#### 4. GLULAM FRAMING

- 'GLB-X' DESIGNATED GLULAM BEAM GLULAM COLUMNS SHALL BE LACED PER DETAIL 5/S5.04

#### MASONRY/CONCRETE

- a. SEE BOND BEAM / CONCRETE BEAM / TIE BEAM SCHEDULE FOR THE FOLLOWING:
  - 'BB-X' INDICATES BOND BEAM, SEE SCHEDULE 1/S5.06
  - 'TB-X' INDICATES TIE BEAM, SEE SCHEDULE 1/S5.06
  - 'CB-X' INDICATES CONCRETE BEAM, SEE SCHEDULE 1/S5.06
- TOP OF WALL EL SHALL BE AS INDICATED ON THE DRAWINGS
- TOP OF INTERIOR WALL SHALL BE TERMINATED WITH A 8" x 16" DEEP BOND BEAM W/ (2) #5 CONTINUOUS TOP & BOTTOM OR WITH TIE BEAM AS ON THE EXTERIOR WALL

#### 6. GENERAL NOTES

a. SEE GENERAL NOTES FOR ADDITIONAL INFORMATION

# KEYED NOTES:

- COLD-FORMED STEEL OUTLOOKERS BY TRUSS MANUFACTURER. 2 1/2" DEEP x 54 MILS MINIMUM @ 2'-0" OC MAX. SEE DETAIL 5/S5.01
- CONT HSS8x4x5/16" @ T/STL = 13'-0". SEE DETAIL 2 & 4/S5.03 FOR CONNECTION TO GLC SEE DETAIL 8/S5.01 FOR CONNECTION TO CMU WALLS. CONNECT TO CORNER GLC IN
- HSS8x4x5/16" COLUMN BETWEEN CFS WALL & STOREFRONT. WELD TO INTERMEDIATE & TOP HSS BEAM W/BEVEL WELD & PARTIAL PENETRATION GROOVE WELD.
- HSS 4x4x1/4" COLUMNS @ ENTRY JAMBS. WELD TO UNDERSIDE OF HSS8x4x5/16" W/ 1/4" FILLET WELD ALL AROUND.
- 5. HSS12x6x5/16" HEADER, BOTTOM ELEVATION = 7'-8".



1 N T E R N A T I O N A L 4211 W. BOY SCOUT BLVD SUITE 500 TAMPA, FI 33607 FLORIDA LICENSE# AA 26002484

Michael Baker

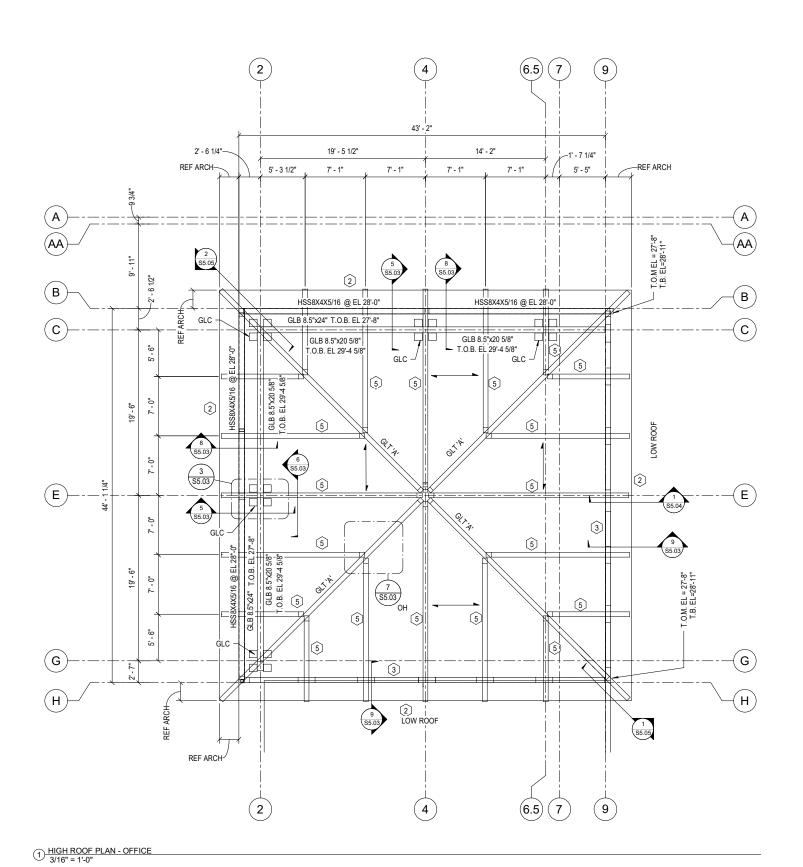
JEFFREY YEAGER, P.E. ŏ.

ROOF PLAN OFFICE

AIRPORT TERMINAL AND HANGAR DEVELOPMENT

DESIGNED BY: AJM CHECKED BY: PPROVED BY PROJECT NO: 2019.028.01 DATE: 11/1/2021

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#### HIGH ROOF PLAN NOTES

- ROOF CONSTRUCTION SHALL CONSIST OF 2" x 20 GA. VERSA-DEK ACOUSTICAL LS METAL DECK ON PRE-ENGINEERED GLULAM TRUSSES, BEAMS OR PURLINS.
- CONNECT DECK TO GLT'S AND GLB'S WITH #12 TEK SCREWS 3" MINIMUM PENETRATION AT 24/4 PATTERN AT SUPPORTS AND 6" OC AT EXTREME ENDS.

  CONNECT DECK TO STEEL OR CFS SUPPORTS WITH #12 TEK SCREWS AT 24/4
- PATTERN AT SUPPORTS AND 6" OC AT EXTRENE ENDS.
- SIDE LAP FASTENERS SHALL BE (2) #10 TEK SCREWS BETWEEN EACH SUPPORT DECK FASTENERS SHALL BE CONCEALED AND NOT VISIBLE FROM THE BOTTOM FINISH OF EXPOSED DECK.

#### 2. MASONRY/CONCRETE

- a. SEE BOND BEAM / CONCRETE BEAM / TIE BEAM SCHEDULE FOR THE FOLLOWING:
  - 'BB-X' INDICATES BOND BEAM, SEE SCHEDULE 1/S5.06 'TB-X' INDICATES TIE BEAM, SEE SCHEDULE 1/S5.06

  - 'CB-X' INDICATES CONCRETE BEAM, SEE SCHEDULE 1/S5.06
- TOP OF WALL EL SHALL BE AS INDICATED ON THE DRAWINGS
  TOP OF INTERIOR WALL SHALL BE TERMINATED WITH A 8" x 16" DEEP BOND BEAM W/ (2) #5 CONTINUOUS TOP & BOTTOM OR WITH TIE BEAM AS ON THE EXTERIOR WALL.

#### GLULAM NOTES

- GLT "X" DESIGNATES GLULAM TRUSS
- GLB DESIGNATES GLULAM BEAMS
- GLC DESIGNATES GLULAM COL. REF DETAIL 3/S5.03 HANG GLB 3.5'S FROM GLT "A" & GLB 6 3/4" W/CENTER WEB OF PURLINS & LAG
- SCREWS THRU TOP OF GLB OR GLT.

  DOUBLE TAPER TOPS OF GLT'S FOR METAL DECK BEARING AND CONNECTIONS.
- GLULAM COLUMNS SHALL BE LACED TOGETHER PER DETAIL 5/S5.04
  TAPER TOPS OF PERIMETER GLB'S FOR METAL DECK BEARING AND CONNECTIONS.

#### GENERAL NOTES

a. SEE GENERAL NOTES FOR ADDITIONAL INFORMATION

# KEYED NOTES:

- 1. FOR TRUSS CONNX SEE 1/S5.05 FOR INFO.
  - 1A. CONNECT METAL DECK TO ALL GLULAM PURLINS, BEAMS, AND TRUSSES W/ #12 TEK SCREWS (3" MIN PENETRATION) @ 36/7 PATTERN AND 12" OC @ EXTREME ENDS.
- 2. CONT BENT PLATE 3x3x5/16" ATTACH TO TOP OF GLB & GLT W/ 1/2" DIA x 6" LAG SCREW @ EA LOCATION AND AT EA END.
- TB-4 @ EL = 28'-11" BETWEEN GLB ALONG GRID LINES 'H' & '9'. LEAVE 2'-0" WIDE POCKET @
- 4. HSS8x4x5/16" COLUMN ATTACHED TO UNDERSIDE OF HSS BEAM.
- 5. GLB 6 3/4" x 17 7/8" (SL)



Michael Baker
I N T E R N A T I O N A L
4211 W. BOY SCOUT BLVD
SUITE 500
TAMPA, FL 33607
FLORIDA LICENSE # AA 26002484

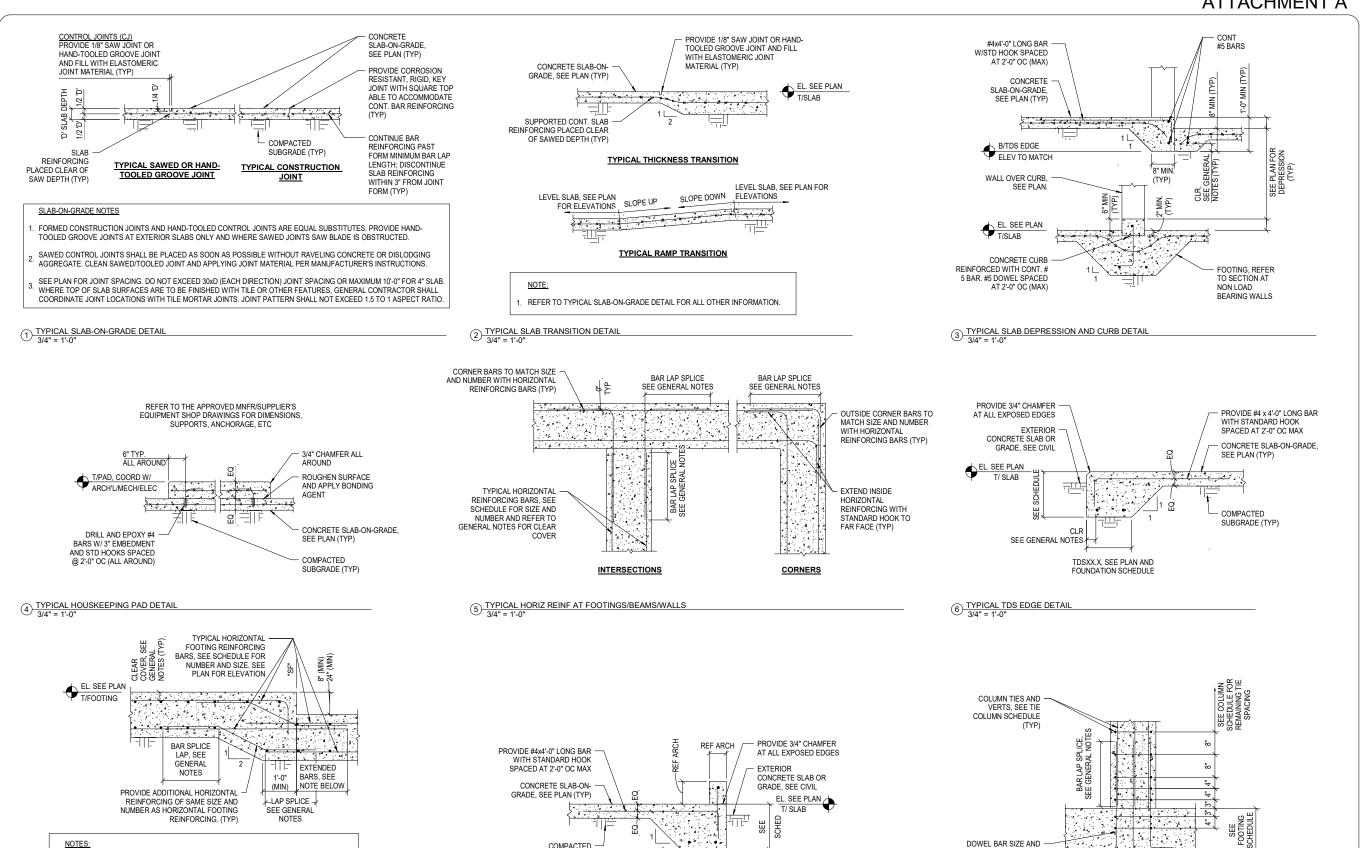
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HIGH ROOF PLAN OFFICE

AIRPORT TERMINAL AND HANGAR DEVELOPMENT

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SUBGRADE (TYP)

8 TYPICAL TDS EDGE W/CURB DETAIL 3/4" = 1'-0"

TDSXX.X. SEE PLAN AND

FOUNDATION SCHEDULE

SEE GENERAL NOT ES

"SF" INDICATES STEP FOOTING (AS REQUIRED). GENERAL CONTRACTOR SHALL COORDINATE STEP FOOTING LOCATIONS WITH WALL PIPE SLEEVES.

EXTEND TYPICAL HORIZONTAL BAR 48xBAR DIA. BEYOND STEP FOOTING VERTICAL CONCRETE FACE WHERE POSSIBLE, ELSE PROVIDE STANDARD HOOK.

7 TYPICAL WALL STEP FOOTING DETAIL 3/4" = 1'-0"



NUMBER TO MATCH

TIE COLUMN

REINFORCING

FOUNDATION.

SEE PLAN

9 TYPICAL CONCRETE COLUMN (CC#)
3/4" = 1'-0"



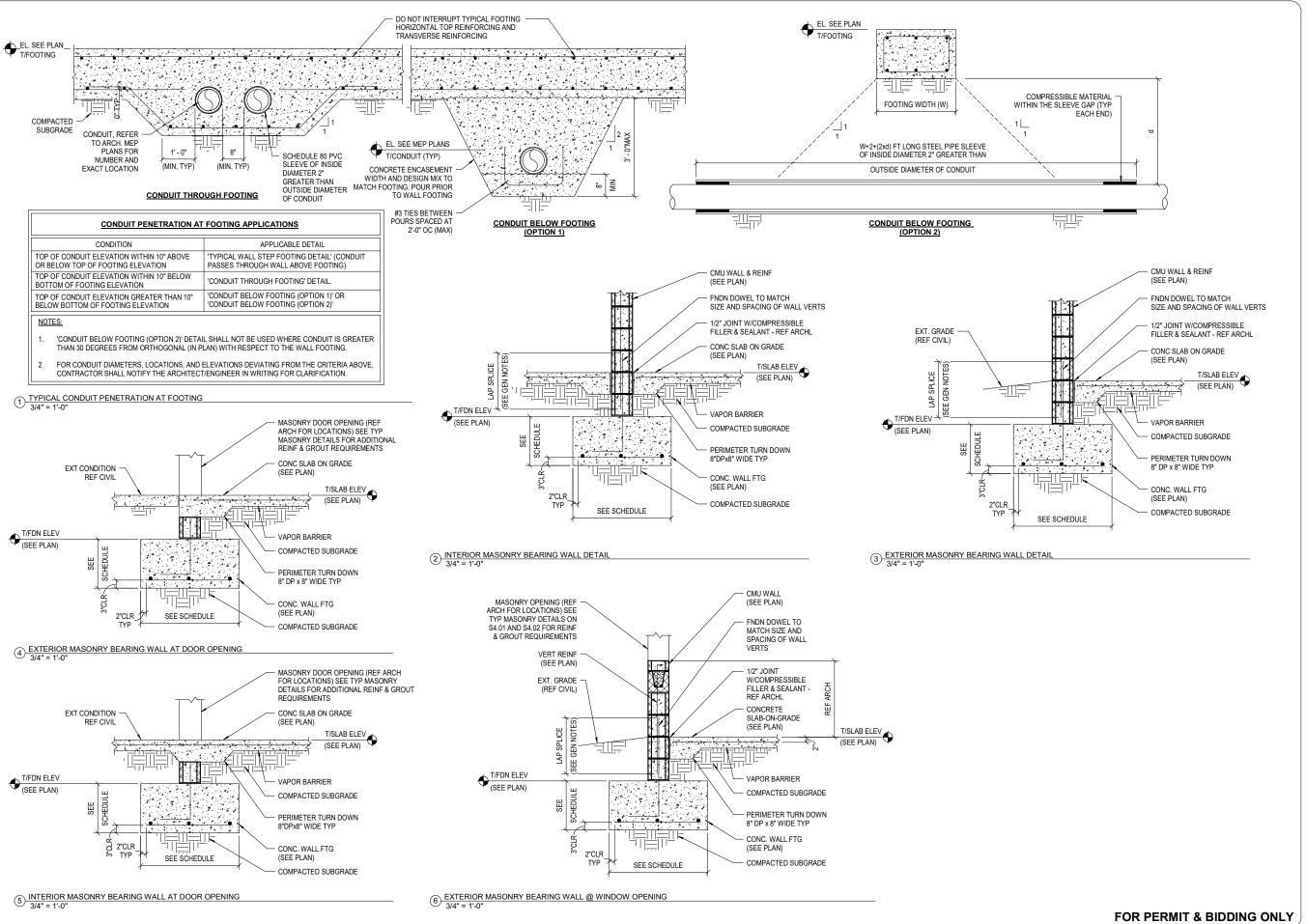
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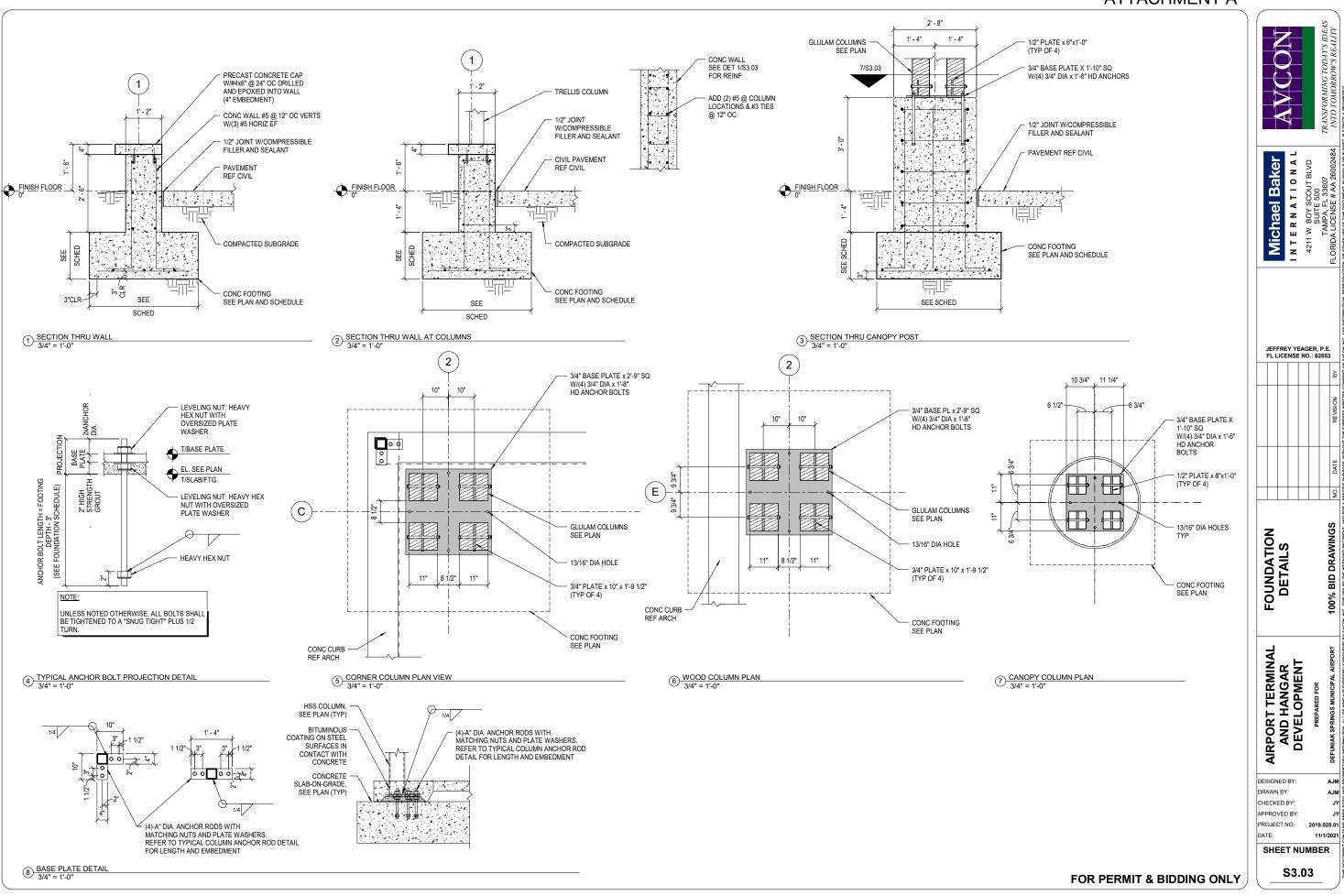
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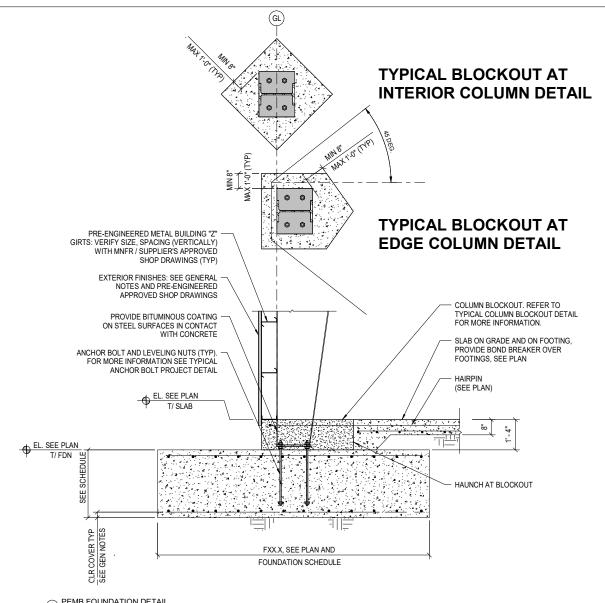
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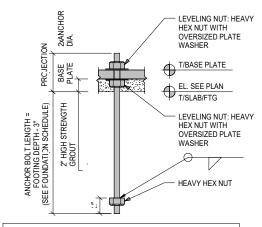


EL SEE PLAN
T/ FDN
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T/ FDN
FOUNDATION SCHEDULE

SLAB ON GRADE AND ON FOOTING, PROVIDE BOND BREAKER OVER FOOTINGS, SEE PLAN

HAIRPIN SEE PLAN
FXX.X, SEE PLAN AND
FOUNDATION SCHEDULE

1) PEMB FOUNDATION DETAIL 1/2" = 1'-0" 2 PEMB FOUNDATION DETAIL AT HANGER DOOR 1/2" = 1'-0"



### NOTE:

- UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE TIGHTENED TO A "SNUG TIGHT" PLUS 1/2 TURN.
- 2. REFER TO THE APPROVED PEMB SHOP DRAWINGS FOR THE NUMBER AND DIAMETER OF ANCHOR BOLTS.

3 PEMB ANCHOR BOLT DETAIL
3/4" = 1'-0"



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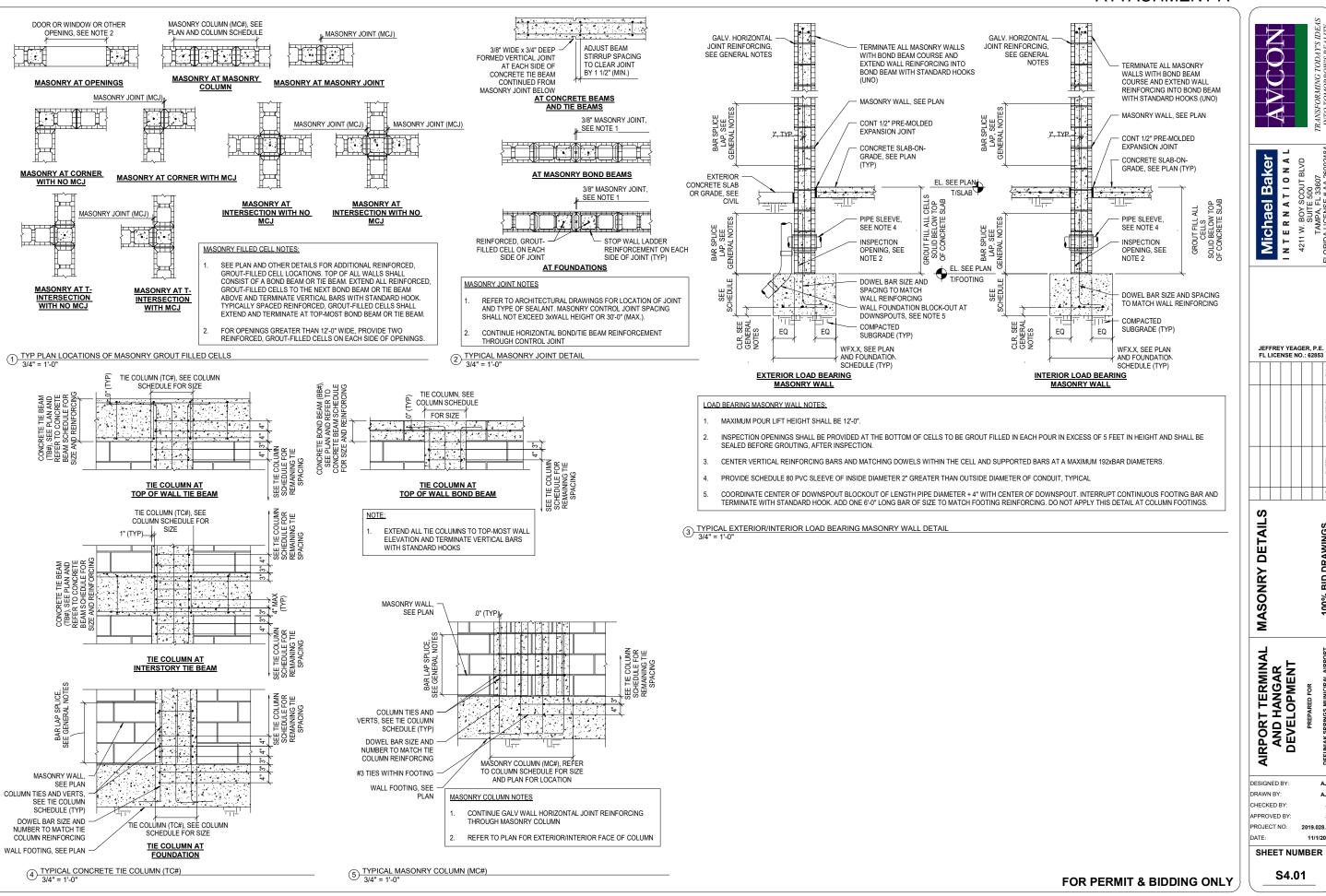
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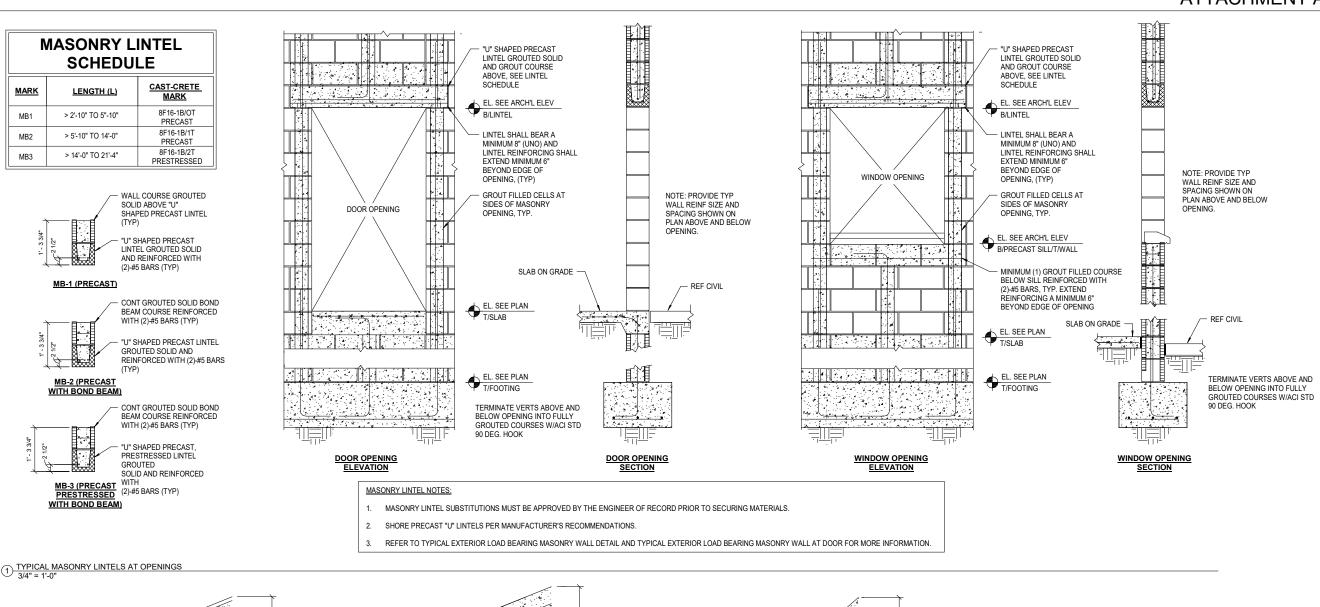
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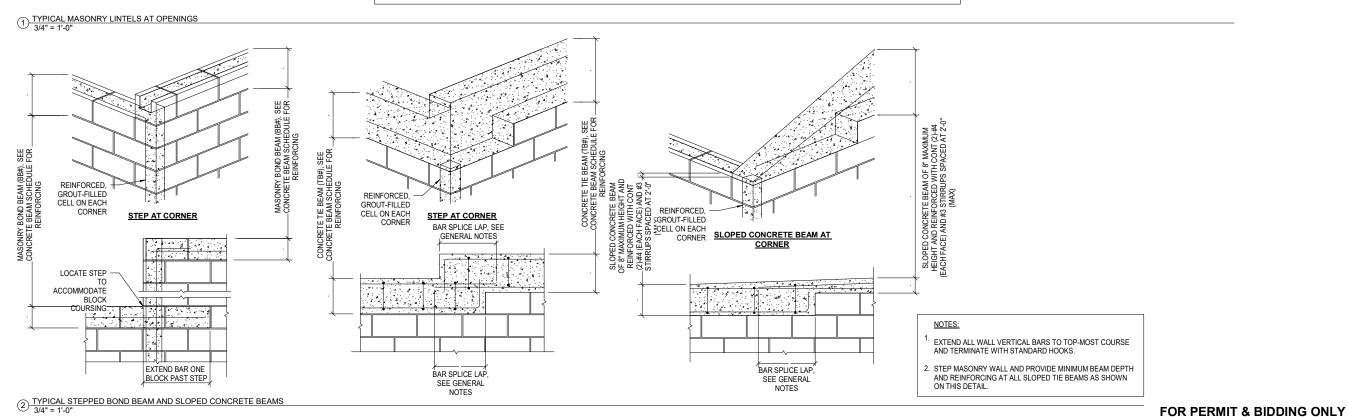
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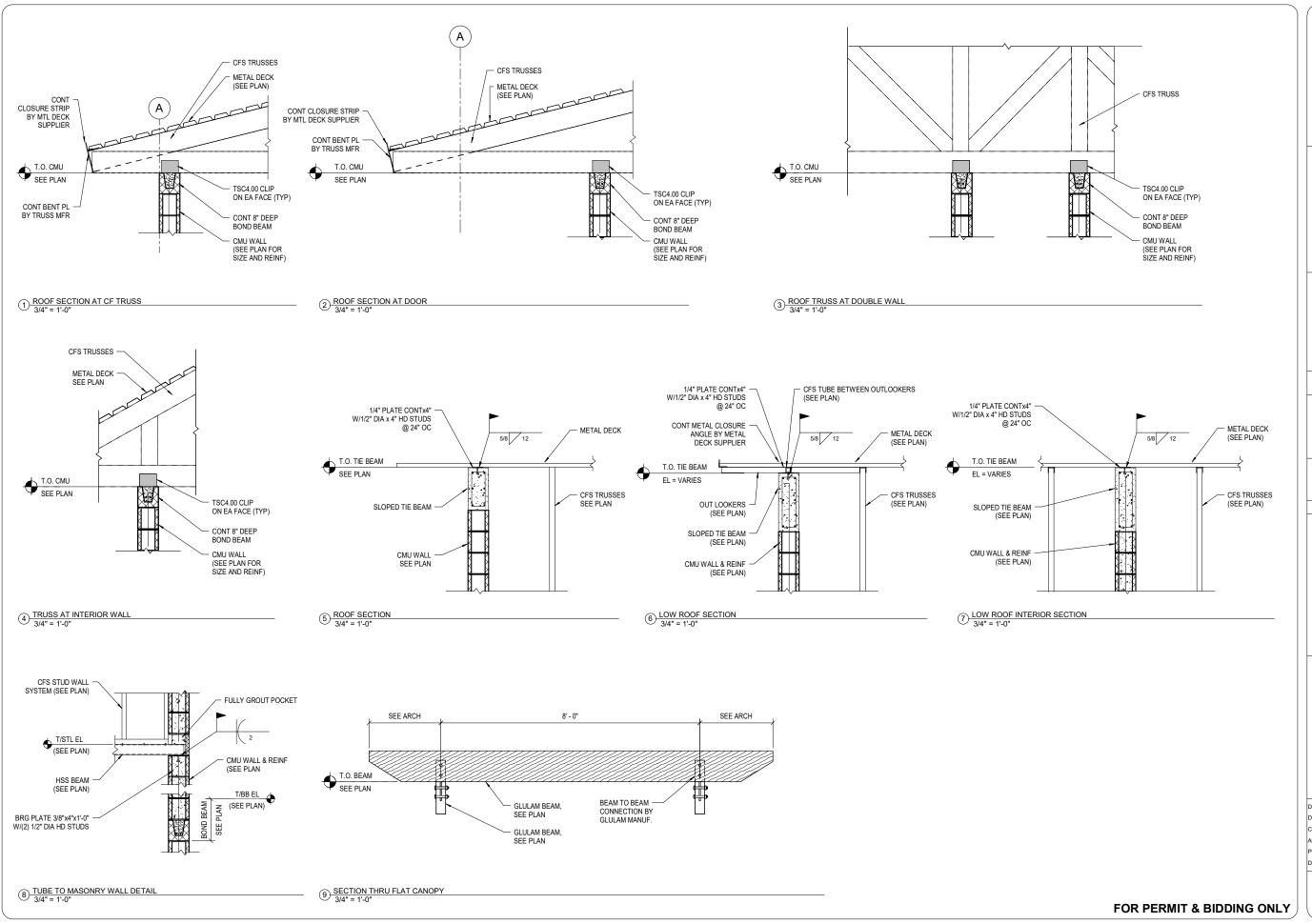
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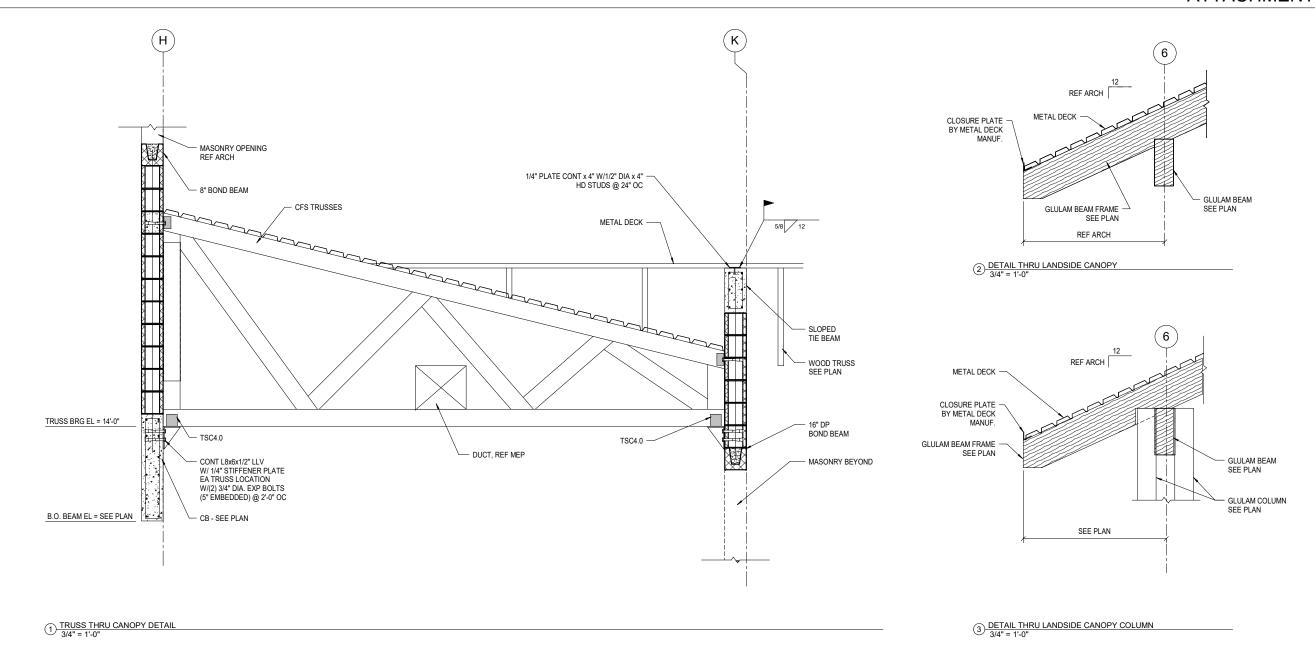


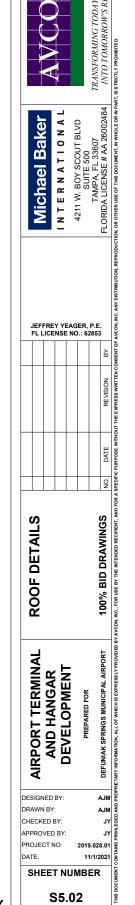


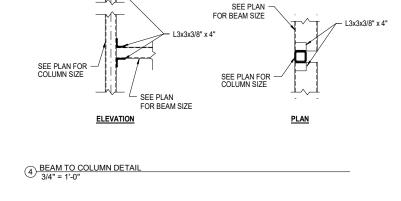




Michael Baker
INTERNATIONAL
4211 W. BOY SCOUTBLYD
SUITE 500
TAMPA, FL 33607
FLORIDALICENSE # AA 26002484 JEFFREY YEAGER, P.E. FL LICENSE NO.: 62853 Ŏ. ROOF DETAILS AIRPORT TERMINAL AND HANGAR DEVELOPMENT DESIGNED BY: AJM CHECKED BY: APPROVED BY: PROJECT NO: 2019.028.01 DATE: 11/1/2021 SHEET NUMBER S5.01



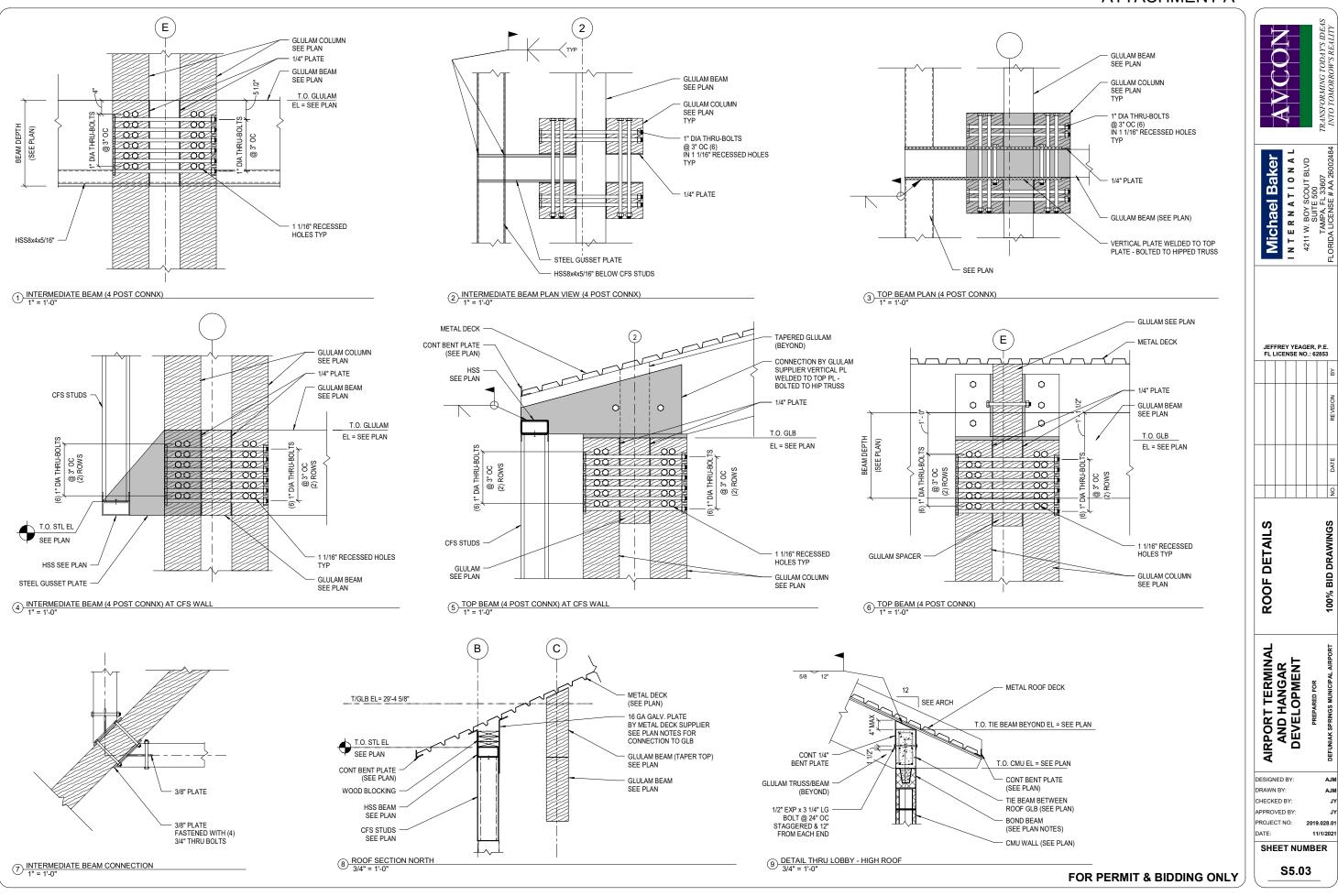


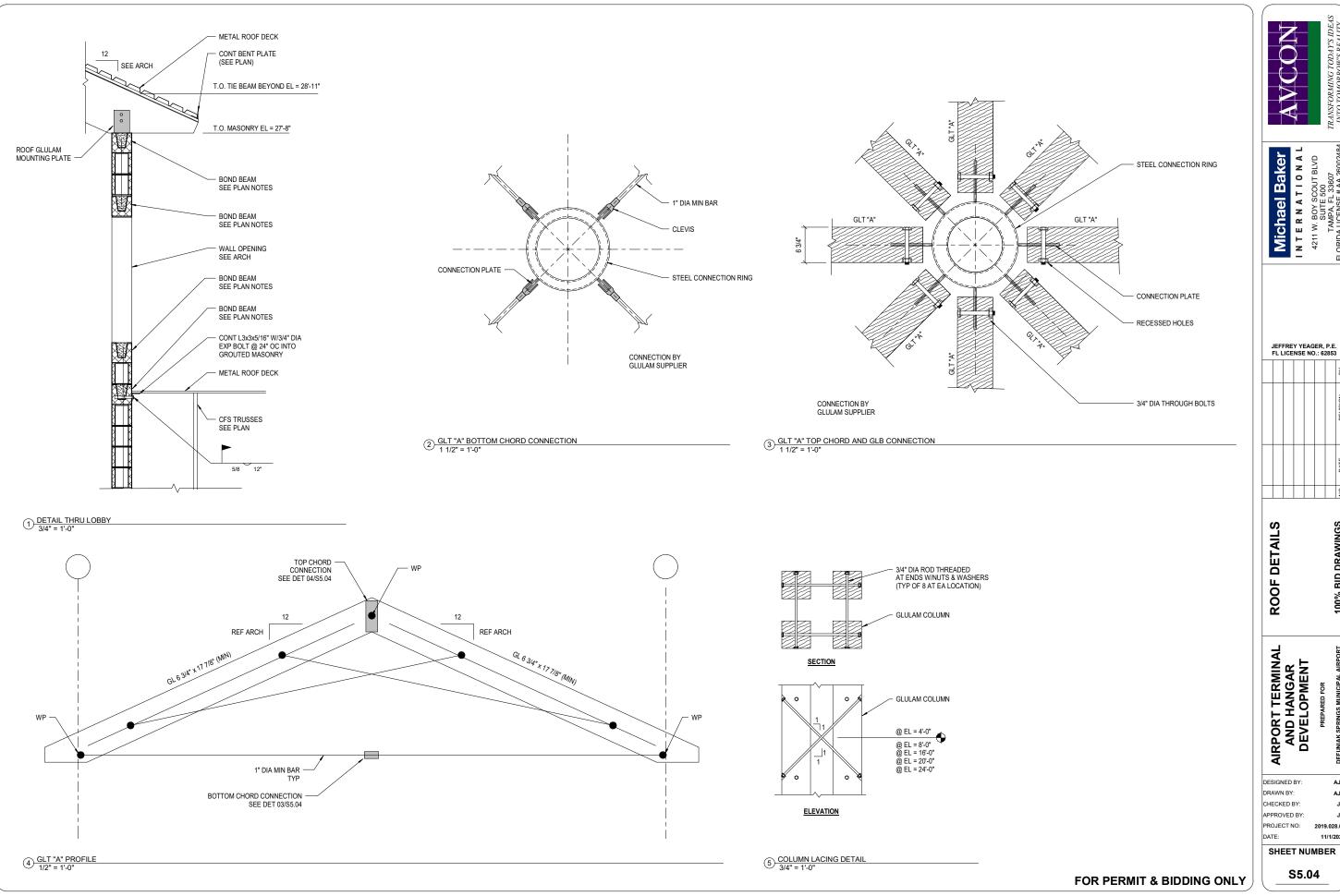


1/2" CAP PLATE

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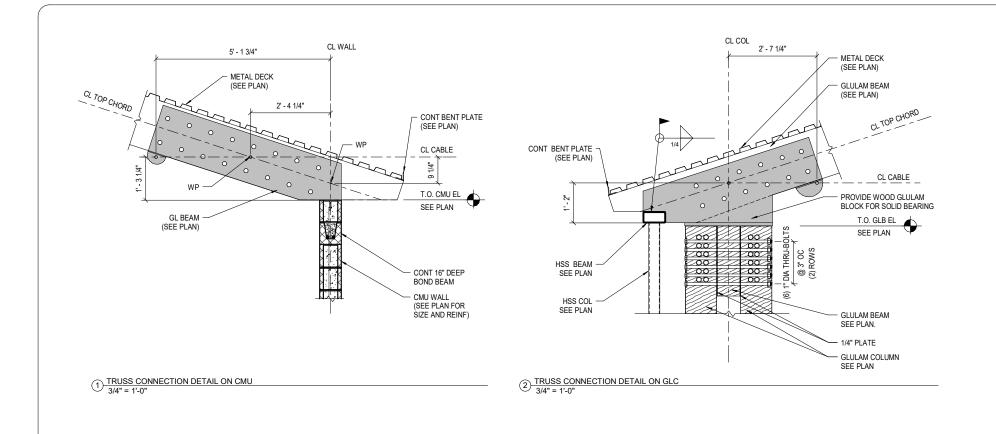
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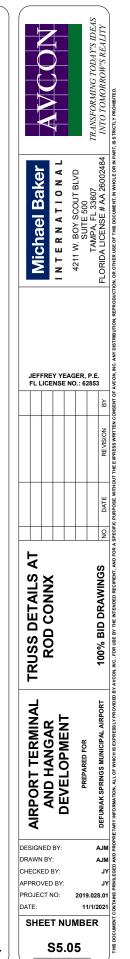
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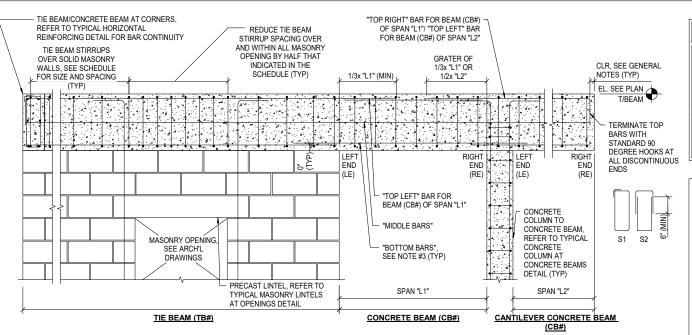
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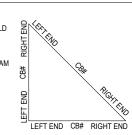




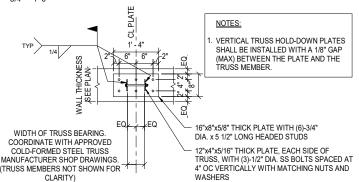
	BOND BEAM/CONCRETE BEAM/TIE BEAM SCHEDULE												
	SI	ZE		REINFOR	CEMENT				STIRRUPS	REMARKS			
BEAM MARK	WIDTH	DEPTH	TOP LEFT	TOP RIGHT	воттом	MIDDLE	SIZE	TYPE	SPACING (FROM FACE OF EACH SUPPORT/BALANCE)				
BB1	8"	16"	N/A	N/A	(2)-#5	N/A	N/A	N/A	N/A				
CB1	8"	24"	(2)-#7	(2)-#7	(2)-#7	#7 EF	#3	S1	8" OC (MAX)				
TB1	8"	16"	(2)-#5	(2)-#5	(2)-#5	N/A	#3	S1	12" OC (MAX)				
TB2	8"	24"	(2)-#5	(2)-#5	(2)-#5	#5 EF	#3	S1	12" OC (MAX)				
TB3	12"	16"	(2)-#8	(2)-#8	(2)-#8	N/A	#3	S1	12" OC (MAX)				
TB4	8"	15"	(2)-#8	(2)-#8	(2)-#8	N/A	#3	S1	12" OC (MAX)				

#### BEAM SCHEDULE NOTES

- TO DETERMINE LEFT AND RIGHT ENDS OF BEAM MEMBERS, ORIENT PLAN WITH SAME SITE BEARING. LEFT AND RIGHT ENDS OF BEAM IN THE FIELD ARE DEFINED BY THE LEFT AND RIGHT END OF THE BEAM MARK ON PLAN WHEN THE BEAM MARK IS READ IN THE NORMAL LEFT TO RIGHT CONVENTION. (SEE EXAMPLE)
- REINFORCING SHOP DRAWINGS SHALL SHOW BEAM DIAGRAM WITH NOTE #1 AND SHALL INDICATE LEFT END (LE) AND RIGHT END (RE) IN THE BEAM ERECTION PLAN AND REINFORCING SCHEDULE.
- EXTEND ALL "BOTTOMS BARS" PAST THE OTHER SIDE OF COLUMN/VERTICAL SUPPORTS. LAP SPLICES OF "TOP LEFT" AND "TOP RIGHT" WITHIN 1/4xBEAM SPAN L# FROM A COLUMN/VERTICAL SUPPORT IS PROHIBITED. LAP SPLICES OF "BOTTOM BARS" WITHIN THE MIDDLE HALF OF A BEAM
- BEAM WIDTH AND DEPTHS SHOWN ARE NOMINAL DIMENSIONS. PROVIDE ACTUAL DIMENSIONS TO FIT MASONRY WIDTH AND COURSING. BEAM DEPTHS SHALL BE THE MINIMUM REQUIRED TO MEET THE DESIGN INTENT. BEAM DEPTHS SHALL BE INCREASED BEYOND THE MINIMUM DEPTH SHOWN TO ACCOMMODATE MASONRY COURSING AND EMBEDDED PLATE CONNECTIONS, AS FAR AS IT DOES NOT CONFLICT OR INTERRUPT OTHER STRUCTURAL OR ARCHITECTURAL ELEMENTS OF THE WALL



1) SCHEDULE - BOND BEAM/CONCRETE BEAM/TIE BEAM
3/4" = 1'-0"





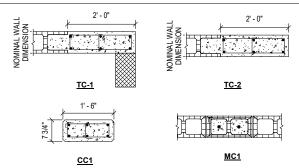
2 TRUSS HOLD-DOWN (H4) 3/4" = 1'-0"

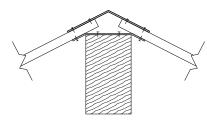
4 COLUMN SCHEDULE 3/4" = 1'-0"

<u>COLUMN SCHEDULE</u>									
COLUMN MARK	VERTS	TIES							
CC1	(6) #6	#4 @ 10"							
CC2	(10) #7	#4 @ 10"							
TC1	(9) #5	#3 @ 8"							
TC2	(10) #5	#3 @ 8"							

### **COLUMN SCHEDULE NOTES**

- REFER TO PLAN FOR COLUMN ORIENTATION.
- 2. FOR COLUMNS OF UNCOMMON SHAPE AND PLAN ORIENTATION, REFER TO THE COLUMN DETAILS BELOW.
- 3. REFER TO CONCRETE COLUMN, MASONRY TIE COLUMN, MASONRY COLUMN DETAILS FOR MORE INFORMATION.





#### DECK BEARING MATERIAL ON HIP OR RIDGE



SPANNING A RIDGE OR HIP

(3)	DECK BEARING DETAIL
9	1 1/2" = 1'-0"

	TRUSS HOLD-DOWN CONNECTOR SCHEDULE										
CONNECTOR MARK	MODEL NO.	FASTENERS	ANCHORAGE	ALLOWABLE UPLIFT LOAD (LB)							
H1	HHETA16	(10) #10 SS METAL SCREWS	N/A	2235							
пі	6"x4"X3" LONG GALV 14 GA CLIP	(6) #14 SS METAL SCREWS	(2) 5/16" DIA. CONCRETE SCREWS	N/A							
	(1)-S/VGT2.5	(16) #14 SS METAL SCREWS	(1)-5/8" DIA A307 (GALV) W/ 8" EMBED	3050							
H2	14 GA MIN STIFFENER PLATE PER TRUSS MFR	1/8" WELD	N/A	N/A							
	6"x4"X3" LONG GALV 14 GA CLIP	(6) #14 SS METAL SCREWS	(2) 5/16" DIA. CONCRETE SCREWS	N/A							
H3	(2)-S/VGT2.5	(32) #14 SS METAL SCREWS	(2)-5/8" DIA A307 (GALV) W/ 8" EMBED	6100							
H4	SEE DETAIL 2/S5.06			9000							
H5	GALV 14 GA CLIP	(8) #14 SS METAL SCREWS	3/8" DIA A307 x 5" EMBED, SS ADHESIVE THREADED RODS	1800							
H6	10"x4"X3" LONG GALV 14 GA CLIP	(16) #14 SS METAL SCREWS	SEE DETAIL	7200							
H7	6"x4"X3" LONG GALV 14 GA CLIP	(8) #14 SS METAL SCREWS	SEE DETAIL	2000							

- ALL "SIMPSON COMPANY" HANGERS, ETC. (OR APPROVED EQUAL) SHALL BE INSTALLED AS RECOMMENDED BY THE MANUFACTURER'S REQUIREMENTS.
- 2. EMBEDDED ANCHOR BOLTS SHALL BE STRAIGHT, THREADED RODS WITH A PLATED END AS SHOWN IN THE "TYPICAL ANCHOR BOLT DETAIL".

FOUNDATION SCHEDULE			
FOOTING	SIZE		REINFORCEMENT
MARK	T	WxL	REINFORGEMENT
TDS1.33	16"	1'-4" x CONT	CONT (2)-#5 BOTTOM
WF2.0	16"	2'-0" x CONT	CONT (3)-#5 BOTTOM, #5 @ 18" OC TRANSVERSE, BOTTOM
WF2.5	16"	2'-6" x CONT	CONT (3)-#5 BOTTOM, #5 @ 18" OC TRANSVERSE, BOTTOM
WF3.0	16"	3'-0" x CONT	CONT (4)-#5 BOTTOM, #5 @ 18" OC TRANSVERSE, BOTTOM
WF4.0	16"	4'-0" x CONT	CONT (5)-#5 TOP & BOTTOM, #5 @ 18" TRANSVERSE, BOTTOM
F5.0	24"	5'-0" x 5'-0"	(6)-#6 BOTTOM, EACH WAY
F6.0	24"	6'-0" x 6'-0"	(7)-#5 BOTTOM, EACH WAY
F6.5	24"	6'-6" x 6'-6"	(8)-#5 BOTTOM, EACH WAY
F9.0	24"	9'-0" x 9'-0"	(11)-#6 BOTTOM, EACH WAY
F12.0	24"	12'-0" x 12'-0"	(21)-#5 BOTTOM, EACH WAY
F13.5	24"	13'-6" x 13'-6"	(16)-#6 BOTTOM, EACH WAY
F14.5	24"	14'-6" x 14'-6"	(13)-#7 BOTTOM, EACH WAY
TSF1.33	16"	1'-4" x CONT	CONT (2)-#5 BOTTOM, #5 @ 18" OC TRANSVERSE, BOTTOM

5 FOUNDATION SCHEDULE
3/4" = 1'-0"

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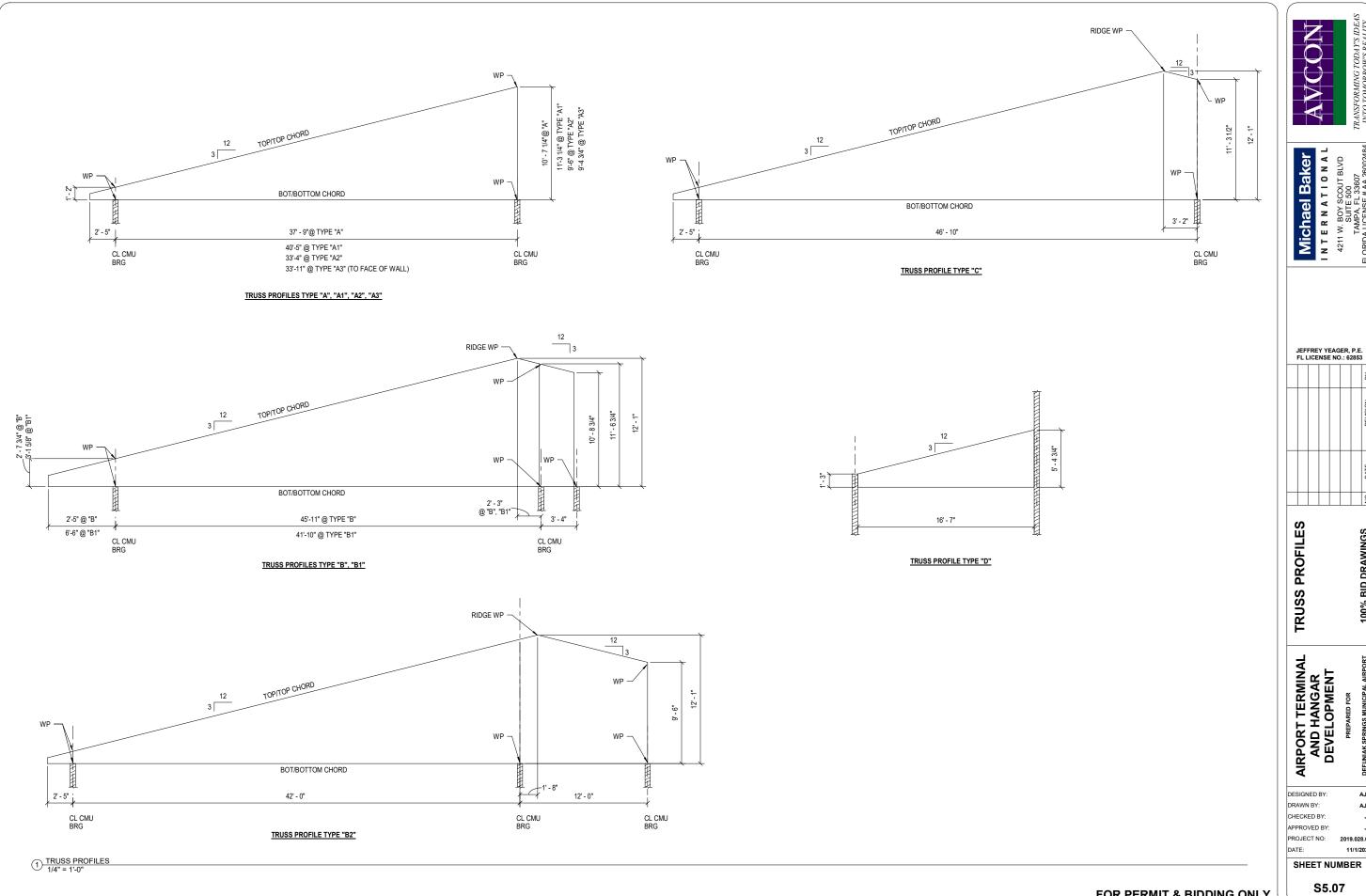
SCHEDULES

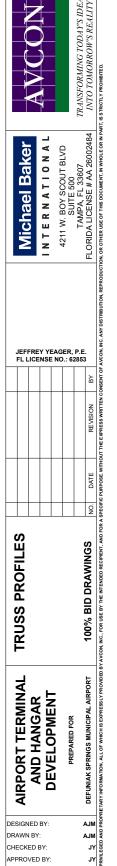
AIRPORT TERMINAL AND HANGAR DEVELOPMENT

DESIGNED BY: CHECKED BY: PROVED BY PROJECT NO: 2019.028.01 DATE: 11/1/2021

SHEET NUMBER

S5.06





2019.028.01

11/1/2021

S5.07