

**ADDENDUM NO. 1**  
**to the**  
**PROJECT DOCUMENTS AND SPECIFICATIONS**  
**for**  
**DFS TERMINAL, HANGAR, AND APRON DEVELOPMENT**

**Prepared for:**  
**CITY OF DEFUNIAK SPRINGS, FLORIDA**

**Prepared By:**



**320 Bayshore Drive, Suite A**  
**Niceville, Florida 32578-2425**

**AVCON Project No. 2019.0028.01**

**Addendum Date: February 7, 2022**

Note: The bidder shall acknowledge receipt of this addendum on the Bid Form, Page B1-3 in the space provided.

**ADDENDUM NO. 1**  
**DFS TERMINAL, HANGAR, AND APRON DEVELOPMENT**

**Date of Issue:** February 7, 2022

**Bid Submittal Deadline:** Thursday, March 3, 2022 @ 2:00 a.m. (local time) (**UNCHANGED**)

**Notice to all Plan Holders:** Please insert this addendum (2 pages including cover, excluding attachments) into your copy of the Project Bid Documents.

The following changes to the Project Documents and Specifications are issued by the Engineer and shall have the same force and effect as though part of the original issue:

**A. Changes to the Bid Drawings:**

1. Structural Drawings      **ADD** the following structural design drawings in their entirety included in **Attachment A** (20 pages) hereto.

*\*\*Sheets S-1.00 through S-5.07\*\**

**END OF ADDENDUM NO. 1**

GENERAL:

1. ALL STRUCTURAL WORK SHALL BE IN ACCORDANCE WITH THE FOLLOWING MINIMUM STANDARDS:

A. FLORIDA BUILDING CODE 2020

B. REGULATIONS IN ACCORDANCE WITH THE LOCAL JURISDICTION.

C. SIGNED AND SEALED PROJECT CONTRACT DOCUMENTS (INCLUDING SIGNED AND SEALED DRAWINGS BY DELEGATED ENGINEERS AND THE GEOTECHNICAL REPORT) AND LATEST ADDENDA. CONTRACT DOCUMENTS DO NOT INCLUDE SHOP DRAWINGS AND OTHER UNSEALED SUBMITTAL DOCUMENTS.

2. THE REFERENCE OF "GENERAL CONTRACTOR" WITHIN THE STRUCTURAL GENERAL NOTES INCLUDES BUT IS NOT LIMITED TO SUBCONTRACTORS, ERECTORS, FABRICATORS, MATERIAL SUPPLIERS AND/OR INDIVIDUALS PERFORMING THE WORK.

3. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH PROJECT SPECIFICATIONS, ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. REFER TO THESE DRAWINGS FOR DIMENSIONS, EMBEDDED ITEMS, AND OTHER DETAILS NOT SHOWN ON THE STRUCTURAL DRAWINGS. THE GENERAL CONTRACTOR SHALL REVIEW THE DRAWINGS OF ALL DISCIPLINES AND REPORT ANY DISCREPANCIES TO THE ARCHITECT OF RECORD AND STRUCTURAL ENGINEER OF RECORD IN WRITING PRIOR TO SECURING MATERIALS, FABRICATING, OR COMMENCING WORK. THE MORE STRINGENT REQUIREMENTS SHALL GOVERN UNLESS OTHERWISE STATED IN WRITING BY THE ARCHITECT OF RECORD AND STRUCTURAL ENGINEER OF RECORD.

4. DO NOT SCALE DRAWINGS.

5. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND FIELD CONDITIONS PRIOR TO SECURING MATERIALS, FABRICATING, OR COMMENCING WORK.

6. NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED, OR OTHERWISE ALTERED UNLESS APPROVED IN WRITING BY THE ENGINEER OF RECORD.

7. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER ITS CONSTRUCTION IS COMPLETE. THE CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE AND THEY DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS TO ENSURE STABILITY AND SAFEKEEPING OF THE STRUCTURE AND ITS COMPONENTS DURING CONSTRUCTION AS PER THE MOST RECENT PRINTING/ERRATA OF ASCE 37-14 DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION'. THE ENGINEER DOES NOT HAVE CONTROL OF, AND SHALL NOT BE RESPONSIBLE FOR: CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, SAFETY PRECAUTIONS, SAFETY PROGRAMS IN CONNECTION WITH THE WORK, OMISSIONS BY THE GENERAL CONTRACTOR, OR THE FAILURE OF THE GENERAL CONTRACTOR TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

8. DETAILS LABELED "TYPICAL" ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. THE APPLICABILITY OF THE DETAIL TO ITS LOCATION ON THE PLANS CAN BE DETERMINED BY THE TITLE OF DETAIL. SUCH DETAILS SHALL APPLY WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION. QUESTIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE DIRECTED TO THE STRUCTURAL ENGINEER OF RECORD.

9. PERIODIC, LIMITED, SITE OBSERVATION BY FIELD REPRESENTATIVES OF AYCON, INC IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE CONTRACTOR'S WORK IS PROCEEDING IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. SITE OBSERVATIONS SHALL NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS CHECKS OF THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER AS PERIODIC SPOT CHECKS OF READILY APPARENT DEFECTS OR DEFICIENCIES IN THE WORK. LIMITED SITE VISITS BY THE ENGINEER OF RECORD DO NOT RELIEVE OR TAKE THE PLACE OF SCHEDULED TESTING AND INSPECTIONS BY AUTHORIZED AGENT OR INSPECTOR.

10. ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE. A PLANNED MAINTENANCE PROGRAM SHALL BE ESTABLISHED BY THE OWNER AND SHALL INCLUDE ITEMS SUCH AS, BUT NOT LIMITED TO:

A. PROTECTIVE COATINGS FOR STEEL AND SEALANTS FOR CRACKED CONCRETE SURFACES

B. SEALANTS WITHIN EXPANSION AND CONTROL JOINTS

C. GENERAL CLEANING OF EXPOSED STRUCTURAL ELEMENTS TO HARSH ENVIRONMENTS OR CHEMICALS

11. THE USE OF REPRODUCTION OF THESE CONTRACT DOCUMENTS AND/OR USE OF CAD FILES BY THE GENERAL CONTRACTOR IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFY HIS ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT, AND OBLIGATES HIMSELF TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR.

12. THE GENERAL CONTRACTOR SHALL PROTECT EXISTING FACILITIES, STRUCTURES, AND UTILITIES FROM DAMAGE. INFORMATION SHOWN RELATED TO EXISTING STRUCTURES WAS GATHERED FROM DRAWINGS PREVIOUSLY PREPARED FOR \_\_\_\_\_ BY \_\_\_\_\_ DATED \_\_\_\_ / \_\_\_\_ / \_\_\_\_\_. THE STRUCTURAL DRAWINGS ASSUME THAT THE CONSTRUCTION OF EXISTING STRUCTURES WERE PERFORMED IN ACCORDANCE WITH THE PREVIOUSLY PREPARED DRAWINGS INCLUDING, BUT NOT LIMITED TO, DIMENSIONS, ELEVATIONS, MEMBER SIZES, MATERIALS, DETAILS, ETC. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO VERIFY THE EXISTING CONDITIONS AND NOTIFY THE ARCHITECT OF RECORD AND STRUCTURAL ENGINEER OF RECORD OF ANY DISCREPANCIES IN WRITING IMMEDIATELY.

13. THE GENERAL CONTRACTOR IS REQUIRED TO PERFORM HIS WORK IN A MANNER WHICH MINIMIZES CONFLICT WITH THE OPERATIONS OF THE FACILITY DURING THE COURSE OF THE PROJECT AND TO TAKE ALL PRECAUTIONS TO MINIMIZE VIBRATION, NOISE, DUST AND DEBRIS IN ALL AREAS ADJACENT TO AREAS OF WORK.

14. STRUCTURAL WORK SHALL BE INSPECTED BY QUALIFIED INSPECTORS. FIELD INSPECTION REPORTS SHALL BE FILED WITH THE STRUCTURAL ENGINEER OF RECORD WITHIN 5 DAYS OF TIME OF ACTUAL INSPECTION.

15. SUBSTITUTIONS IN ITEMS (PRODUCTS, MATERIALS, EQUIPMENT, AND INSTRUCTIONS) WITHIN THE CONTRACT DOCUMENTS FOR ANY REASON SHALL BE APPROVED BY THE ARCHITECT OF RECORD AND STRUCTURAL ENGINEER OF RECORD PRIOR TO SECURING MATERIALS, FABRICATING, OR COMMENCING WORK. THE CONTRACTOR SHALL SUBMIT REQUEST FOR SUBSTITUTION IN WRITING TO THE STRUCTURAL ENGINEER OF RECORD IMMEDIATELY UPON DISCOVERY OF NEED AND 15 DAYS PRIOR TO INITIAL LEAD TIME DATE. THE SUBSTITUTION REQUEST SHALL BE SIGNED/DATED AND INCLUDE, AT MINIMUM, THE FOLLOWING:

A. THE ITEM TO BE SUBSTITUTED WITH CLEAR REFERENCE TO THE INTENDED USE IN THE STRUCTURAL SPECIFICATIONS AND DRAWINGS

B. REASONS FOR SUBSTITUTION INCLUDING CHANGES TO CONTRACT COST AND SCHEDULE

C. MANUFACTURER TECHNICAL PRODUCT DATA SHEETS, TEST REPORTS FOR THE SUBSTITUTION ITEM

D. A STATEMENT OF ITEM'S COMPLIANCE WITH THE FLORIDA BUILDING CODE

E. A STATEMENT OF ITEM'S COMPATIBILITY WITH OTHER PORTIONS OF WORK

16. THE GENERAL CONTRACTOR SHALL MAINTAIN, IN GOOD ORDER WITH ALL CHANGES RECORDED AS THEY OCCUR DURING CONSTRUCTION, AS-BUILT DRAWINGS AT THE SITE FOR THE OWNER CONSISTING OF ONE COLLECTIVE COPY OF THE FOLLOWING:

A. ALL DRAWINGS

B. ALL SPECIFICATIONS

C. ALL ADDENDA

D. ALL APPROVED SHOP DRAWINGS

E. ALL APPROVED SETTING DRAWINGS

F. ALL CHANGE ORDERS AND OTHER MODIFICATIONS

THESE SHALL BE AVAILABLE TO THE ARCHITECT OF RECORD, ENGINEER OF RECORD, OWNER, OWNER AUTHORIZED REPRESENTATIVE, AND PROJECT INSPECTOR. THE DRAWINGS SHALL BE NEATLY AND CLEARLY MARKED DURING CONSTRUCTION TO RECORD ALL VARIATIONS MADE DURING CONSTRUCTION. UPON COMPLETION OF THE WORK AND PRIOR TO THE FINAL INSPECTION, THE CONTRACTOR SHALL DELIVER ONE COMPLETE SET OF AS-BUILT DRAWINGS TO THE ARCHITECT OF RECORD AND ENGINEER OF RECORD, FOR PREPARATION OF THE RECORD DRAWINGS.

GEOTECHNICAL:

1. REFER TO THE LATEST EDITION OF THE GEOTECHNICAL REPORT FOR RECOMMENDATIONS AND INSTALLATION PROCEDURES. SITE PREPARATION AND FOUNDATION INSTALLATION SHALL COMPLY WITH:

A. REPORT No. 1011-2020054

B. PREPARED BY: NOVA

C. DATED: MAY 29, 2020

2. VERIFICATION THAT THE COMPACTION REQUIREMENTS HAVE BEEN MET SHALL BE MADE BY THE GEOTECHNICAL CONSULTANT PRIOR TO PLACING FOUNDATIONS AND SLABS. LOCATIONS FAILING TO MEET THE REQUIREMENTS SHALL BE RECOMPACTED AND RETESTED AT THE CONTRACTORS EXPENSE AND AS DIRECTED BY THE GEOTECHNICAL CONSULTANT.

SHALLOW FOUNDATIONS:

1. SHALLOW FOUNDATION DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF AS PER THE REFERENCED GEOTECHNICAL REPORT.

2. NO CONCRETE SHALL BE PLACED IN WATER. CONTRACTOR SHALL SAFEGUARD AND PROTECT ALL EXCAVATIONS AND SHALL KEEP THEM FREE OF WATER. GROUND WATER MUST BE MAINTAINED AT A MINIMUM OF 2 FT. BELOW BOTTOM OF EXCAVATION AT ALL TIMES DURING CONSTRUCTION.

3. LOCATE ALL EXISTING UTILITIES IN THE CONSTRUCTION AREA PRIOR TO EXCAVATION AND AVOID DAMAGE TO THEM. CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIR OF ANY AND ALL DAMAGED UTILITIES CAUSED IN CONSTRUCTION TO THE SATISFACTION OF THE LOCAL BUILDING OFFICIALS AND/OR OWNER.

4. BACKFILLING AGAINST FOUNDATION WALLS SHALL NOT BE DONE UNTIL WALL HAS BEEN CURED TO A SUFFICIENT STRENGTH (7 DAYS MINIMUM), AND WALLS ARE PROPERLY SHORED OR BRACED. USE MECHANICAL TAMPERS OR OTHER APPROVED HAND TAMPERS WITHIN FIVE FEET OF THE PERIMETER FOR ALL CONCRETE STRUCTURES AND BURIED STRUCTURES TO WITHIN 18 INCH COVERAGE.

5. CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS WITHIN ±2", UNO.

6. COORDINATE BURIED PLUMBING AND OTHER UTILITY LINES WITH FOOTING LOCATIONS FOR INTERFERENCE AS PER THE TYPICAL CONDUIT PENETRATION AT FOOTING DETAIL. WITH PRIOR WRITTEN APPROVAL OF THE ENGINEER OF RECORD, WALL FOOTINGS MAY BE STEPPED AND ISOLATED COLUMN FOOTINGS MAY BE DROPPED AS IN ACCORDANCE WITH THE TYPICAL WALL STEP FOOTING DETAIL.

7. EXCAVATING UNDER OR NEAR IN-PLACE FOUNDATIONS WHICH DISTURBS THE COMPACTED SOIL BENEATH SHALL NOT BE PERMITTED. TRENCH EXCAVATIONS WITHIN THE 45 DEGREE LOAD INFLUENCE WIDTH ALONG FOUNDATIONS ARE PROHIBITED.

8. CONSTRUCTION JOINTS IN CONTINUOUS FOOTINGS SHALL TERMINATE IN A VERTICAL CONCRETE FACE WITH A HORIZONTAL FORMED KEYWAY (2x4). CONTINUE REINFORCING THROUGH THE CONSTRUCTION JOINT.

9. SECURE ALL WALL AND COLUMNS DOWELS, ANCHOR RODS, AND OTHER EMBEDDED COMPONENTS INTO PROPER PLACE WITHIN FOOTINGS. DO NOT WET-SET THEM INTO NEWLY POURED CONCRETE.

CAST-IN-PLACE CONCRETE:

1. ALL CAST-IN-PLACE CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT PRINTING/ERRATA OF ACI 318-14 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' AND ACI 301-10 'SPECIFICATION FOR STRUCTURAL CONCRETE'.

2. STRUCTURAL CONCRETE MIX DESIGNS SHALL BE AN APPROVED COMMERCIAL MIX OF PLASTIC AND WORKABLE CONSISTENCY AT TIME OF PLACEMENT CONFORMING TO THE MINIMUM REQUIREMENTS PER THE CONCRETE MIX DESIGN SCHEDULE.

3. SUBMIT MIX DESIGN FOR ENGINEER'S APPROVAL FOR EACH CLASS WITH SPECIFIC LOCATION OF PLACEMENT INDICATED NO LATER THAN TWO WEEKS PRIOR TO SECURING CONCRETE MATERIALS. EACH MIX DESIGN SHALL INCLUDE TESTED, STATISTICAL BACK-UP DATA AS PER ACI 301. ALL MIXES SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C33 FOR COURSE AGGREGATE, UNO.

4. CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE BATCH TICKET SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED. ALL CONCRETE MIXES USING MAXIMUM COURSE AGGREGATE SIZE OF 3/8" (PEAROCK) SHALL CONFORM TO THE REQUIREMENTS OF ASTM C89 AND SHALL BE CONTINUOUSLY PLACED TO NOT EXCEED 50 LINEAR FEET.

5. PLACE AND CURE CONCRETE IN ACCORDANCE WITH ACI STANDARDS AND SPECIFICATIONS. DISCARD CONCRETE EXCEEDING 1-1/2 HOURS FROM THE TIME THE MIXING WATER IS ADDED AT THE BATCH PLANT UNTIL THE CONCRETE IS DEPOSITED IN ITS FINAL POSITION.

6. A CERTIFIED TESTING AGENT SHALL PERFORM INDUSTRY STANDARD TESTS SUCH AS BUT NOT LIMITED TO SLUMP, CYLINDER AND PRISM COMPRESSIVE BREAKS, UNIT WEIGHT, ETC. SLUMP TEST SAMPLES SHALL BE TAKEN AT DISCHARGE POINTS OF CONCRETE PER ASTM C143 'STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE'. WHERE NOT INCLUDED IN THE SPECIFICATIONS, COLLECT AND TEST 4 CYLINDER SETS FOR EACH COMPRESSIVE STRENGTH TEST IN CONFORMANCE WITH ASTM C39 'STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS'. EACH SET OF CYLINDERS CONSIST OF ONE FIELD-CURED SET OF CYLINDERS AND ONE LABORATORY-CURED SET OF CYLINDERS. TESTS SHALL BE CONDUCTED FOR EACH CLASS AND EACH DAY PLACED AT A FREQUENCY OF EVERY 50 CUBIC YARDS. BREAK ONE SET OF CYLINDERS AT 7 DAYS, 1 SET AT 28 DAYS, AND HOLD 2 SETS IN RESERVE. BREAK RESERVE CYLINDERS AS DIRECTED BY THE ENGINEER. TESTING AGENT SHALL SUBMIT REPORTS TO THE ENGINEER WITHIN FIVE WORKING DAYS OF TEST RESULTS.

7. ANY DEVIATION OR ADDITION OF CONSTRUCTION JOINTS FROM THOSE SHOWN ON PLANS MUST BE SUBMITTED IN THE CONCRETE ERECTION OR REINFORCING STEEL SHOP DRAWINGS FOR ENGINEER'S APPROVAL.

8. FOR SHOWROOM INTERIOR SLABS ONLY: SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY.

9. ALL TIE BEAMS (TB#) AND COLUMNS (TC#) INTEGRATED IN CMU WALLS ARE INDICATED AS NOMINAL WIDTH DIMENSIONS. ACTUAL DIMENSIONS SHALL BE THE WIDTH OF THE CMU WALL. USE METAL LATH, MORTAR, OR SPECIALTY PRODUCT TO CONFINE POURED CONCRETE AT ALL TIE BEAMS AND BOND BEAMS IN ACCORDANCE WITH ACI 530.1. USE OF SOLID METAL, FELT CAVITY CAPS, AND PAPER ARE PROHIBITED.

10. CONTRACTOR SHALL COORDINATE ALL TRADES FOR INSTALLATION OF ALL BUILT-IN WORK, SLEEVES, INSERTS, ETC. AS REQUIRED FOR THE COMPLETION OF CONSTRUCTION.

11. ALL CORNERS AND EDGES OF PERMANENTLY EXPOSED CONCRETE SHALL BE 3/4" CHAMFER, UNO.

12. ALL STRUCTURAL SLABS AND FLAT HORIZONTAL SURFACES TO REMAIN EXPOSED TO WEATHER THROUGHOUT ITS LIFETIME SHALL BE TREATED WITH A CLEAR NON-FLAMMABLE PENETRATING SILANE SEALER. PREPARE CONCRETE SURFACES AND APPLY SEALER IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS.

13. REINFORCING STEEL: REINFORCING STEEL SHALL BE ASTM A615 GRADE 60 DEFORMED BARS AND WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI MANUAL OF STANDARD PRACTICE AND THE PROJECT SPECIFICATIONS. SUBMIT REINFORCING SHOP DRAWINGS AND OBTAIN ENGINEER'S APPROVAL PRIOR TO SECURING MATERIAL AND COMMENCING FABRICATION. SEE THE CONCRETE SCHEDULE(S) FOR ALL REQUIRED INFORMATION TO BE SHOWN ON THE BEAM REINFORCING ERECTION SHOP DRAWING SUBMITTAL.

14. PROVIDE CONCRETE COVER OVER REINFORCEMENT PER THE CONCRETE COVER SCHEDULE, UNO.

15. PROVIDE STANDARD HOOKS AT DISCONTINUOUS ENDS OF ALL TOP BARS. TOP BARS AT EXPOSED AREAS SUCH AS BALCONIES, WALKWAYS AND AS SPECIFICALLY SHOWN ON PLAN SHALL BE EPOXY COATED ACCORDING TO ASTM A775.

16. PROVIDE 48xBAR DIAMETER FOR ALL LAP SPLICES IN FOUNDATIONS AND WHERE SPECIFIED ELSEWHERE IN THE PLANS. LAP WELDED WIRE FABRIC SHEETS WITH ONE SPACE PLUS TWO INCHES.

17. PROVIDE CONTINUOUS REINFORCING OR STAGGER SPLICES WHERE POSSIBLE. DO NOT EXCEED 3" CENTER-TO-CENTER BAR SPACING OF SPLICED REINFORCING AND LOCATE SPLICE REINFORCING IN THE SAME PLANE WITH RESPECT TO THE CONCRETE ELEMENT'S CLEAR CONCRETE SURFACE.

	BAR SIZE	3000 PSI	4,000 PSI AND GREATER
A.	#6 AND SMALLER	58xBAR DIA	48xBAR DIA
B.	#7 AND GREATER	72xBAR DIA	62xBAR DIA

CONCRETE MIX DESIGN SCHEDULE:

LOCATION	28-DAY STRENGTH	SLUMP	AGGREGATE SIZE
MASONRY FILLED CELLS	2500	8" - 11"	3/8"
LINTELS AND BOND BEAMS	2500	8" - 11"	3/8"
SLAB-ON-GRADE	3000	4" ± 1"	3/4"
SIDEWALKS	3000	4" ± 1"	3/4"
FOUNDATIONS	3000	4" ± 1"	1"
BEAMS, COLUMNS, AND WALLS	4000	5" ± 1"	3/4"
TIE BEAMS AND TIE COLUMNS	4000	6" ± 1"	3/8"

CONCRETE COVER SCHEDULE:

LOCATION	COVER
BEAMS, JOISTS, AND EXTERIOR SLABS	1 1/2"
INTERIOR SLABS	3/4"
CAST-IN-PLACE COLUMNS, PEDESTALS, AND WALLS	1 1/2"
FOUNDATIONS CAST AGAINST EARTH	3"

MASONRY:

1. ALL MASONRY WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT PRINTING/ERRATA OF TMS 402-16 'BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES' AND TMS 602-16 'SPECIFICATION FOR MASONRY STRUCTURES'.

2. CONCRETE MASONRY UNITS (CMU): CMU SHALL MEET THE REQUIREMENTS OF ASTM C90 FOR HOLLOW LOAD BEARING CMU WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2000 PSI / 3250 PSI / 4500 PSI. (FOR A CORRESPONDING COMPRESSIVE STRENGTH OF MASONRY, fm = 2000 PSI / 2500 PSI / 3000 PSI). COMPRESSIVE STRENGTH REQUIREMENTS SHALL BE MET PER THE UNIT STRENGTH METHOD WITHIN TMS 602 OR PRISM SAMPLING AND TESTING PER ASTM C1314.

3. GROUT: GROUT SHALL CONFORM TO ASTM C476 WITH A SLUMP BETWEEN 8" AND 11", A MAXIMUM AGGREGATE SIZE OF 3/8". SUBMIT PROPOSED GROUT MIX DESIGNS FOR REVIEW PRIOR TO USE. MIX NUMBER OR OTHER POSITIVE IDENTIFICATION SHALL UNIQUELY IDENTIFY MIX. USE OF SUPERPLASTICIZER IS PROHIBITED AND SITE MIXING OF GROUT SHALL NOT BE PERMITTED. SAMPLE AND TEST GROUT IN ACCORDANCE WITH ASTM C1019.

4. MORTAR: MORTAR SHALL MEET THE REQUIREMENTS OF ASTM C270 TYPE "M" FOR MASONRY LAID BELOW GRADE AND TYPE "S" FOR ALL OTHER LOCATIONS. FULLY BED CROSS WEBS OF BLOCKS ALL AROUND CELLS TO RECEIVE GROUT. BED MORTAR THICKNESS GREATER THAN 5/8" IS PROHIBITED.

5. GROUTED CELLS: CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE. ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SUCH CELL WALLS. CMU SHALL BE MOISTENED BEFORE GROUTING CELLS. GROUT SHALL BE CONSOLIDATED AT TIME OF PLACING BY VIBRATING AND RECONSOLIDATED LATER BY VIBRATING BEFORE PLASTICITY IS LOST. USE METAL LATH OR SPECIAL UNITS OVER CELLS NOT TO BE FILLED WITH GROUT TO SUPPORT BOND BEAM. WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE POUR OF GROUT NOT LESS THAN 1 1/2" BELOW THE TOP OF THE UPPERMOST GROUTED MASONRY UNIT OR COURSE. GROUT POUR HEIGHT AND MINIMUM CELL DIMENSIONS TO RECEIVE GROUT SHALL MEET THAT PROVIDED BY THE "GROUT SPACE REQUIREMENTS" TABLE WITHIN TMS 602 WITH A MAXIMUM GROUT POUR HEIGHT OF 12 FEET. CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED FOR EACH POUR LIFT IN EXCESS OF 5 FEET IN HEIGHT. AFTER INSPECTION, THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING. ALL CMU IN CONTACT WITH SOIL SHALL BE FILLED SOLID WITH GROUT. THE FOLLOWING LOCATIONS SHALL BE FILLED SOLID WITH GROUT AND HAVE VERTICAL REINFORCING BARS:

A. CORNERS

B. INTERSECTIONS OF WALLS

C. EACH END OF WALL

D. EACH SIDE OF WALL OPENINGS

E. EACH SIDE OF A CONTROL OR ISOLATION JOINT

F. EACH END OF BEAM BEARING

G. AS INDICATED ON THE PLANS

6. VERTICAL REINFORCEMENT: VERTICAL REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60. VERTICAL DOWELS AND REINFORCING SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE WITH AT LEAST 6" EMBEDMENT INTO THE TOP BOND BEAM, UNO. PROVIDE HOOKED DOWELS IN FOOTINGS FOR VERTICAL REINFORCING ABOVE. LAP SPLICES SHALL BE 48xBAR DIAMETERS. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192xBAR DIAMETERS.

7. JOINT REINFORCEMENT: PROVIDE CONTINUOUS 9 GAGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR-O-WAL OR ENGINEER APPROVED SUBSTITUTION) CONFORMING TO ASTM A951 AT ALTERNATE BLOCK COURSES. PROVIDE PREFABRICATED "TEE" OR CORNER SECTIONS AT ALL INTERSECTING WALLS.

8. BOND BEAM: PROVIDE A CONCRETE BOND BEAM AT THE FOLLOWING LOCATIONS WITH CONTINUOUS REINFORCEMENT AS INDICATED IN THE BEAM SCHEDULE:

A. TOP OF WALL OR PARAPET

B. TOP OF OPENINGS OR DIRECTLY ABOVE LINTELS

C. BELOW OPENINGS

D. AT BEARING LOCATIONS

E. AS INDICATED ON THE PLANS

9. OPENINGS: COORDINATE ALL OPENINGS WITH THE ARCHITECTURAL DRAWINGS. PROVIDE REINFORCED CONCRETE LINTELS OVER OPENINGS AS INDICATED ON THE PLANS.

10. JOINTS: REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION OF ALL MASONRY JOINTS AND WATERPROOFING DETAILS. REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS EXCEPT FOR BOND BEAM AND THE BEAM REINFORCEMENT AS SPECIFIED ON PLAN. REINFORCING SHALL BE DISCONTINUOUS AT ISOLATION JOINTS.

11. CMU SHALL BE LAID IN RUNNING BOND PATTERN, UNO.

TRANSFORMING TODAY'S IDEAS INTO TOMORROW'S REALITY

Michael Baker

INTERNATIONAL

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JEFFREY YEAGER, P.E.  
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GENERAL NOTES

AIRPORT TERMINAL AND HANGAR DEVELOPMENT

PREPARED FOR

DEFUNIAK SPRINGS MUNICIPAL AIRPORT

100% BID DRAWINGS

DESIGNED BY:

AJM

DRAWN BY:

AJM

CHECKED BY:

JY

APPROVED BY:

JY

PROJECT NO:

2019.028.01

DATE:

11/1/2021

SHEET NUMBER

\$1.00

FOR PERMIT & BIDDING ONLY

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DESIGN LOADS:

THE FOLLOWING SUPERIMPOSED LOADS HAVE BEEN UTILIZED IN ACCORDANCE WITH THE MOST RECENT PRINTING/ERRATA OF ASCE 7-16 "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES".

1. LIVE LOADS:
- A. ROOF20 PSI
2. DEAD LOADS:
- A. METAL ROOF STRUCTURE W/ INSULATION15 PSF
- B. ROOF TO RESIST WIND UPLIFT10 PSF
- C. CEILING5 PSF

COMPONENTS AND CLADDING WIND PRESSURES (PSF)

LOCATION	ZONE	EFFECTIVE WIND AREA (A), FT <sup>2</sup>						
		A = 2	A = 10	A = 20	A = 50	A = 100	A = 250	A = 500
ROOF	1	+29.8,-73.9	+24.3,-73.9	+21.9,-73.9	+18.7,-45.0	+16.3,-23.1	+16.3,-23.1	+16.3,-23.1
	1-OH	-90.9	-90.9	-90.9	-71.6	-57.0	-57.0	-57.0
	2e	+29.8,-73.9	+24.3,-73.9	+21.9,-73.9	+18.7,-45.0	+16.3,-23.1	+16.3,-23.1	+16.3,-23.1
	2e-OH	-90.9	-90.9	-90.9	-71.6	-57.0	-57.0	-57.0
	2n	+29.8,-107.8	+24.3,-107.8	+21.9,-93.2	+18.7,-73.9	+16.3,-59.3	+16.3,-40.0	+16.3,-40.0
	2n-OH	-124.8	-124.8	-113.8	-99.3	-88.4	-73.9	-73.9
	2r	+29.8,-107.8	+24.3,-107.8	+21.9,-93.2	+18.7,-73.9	+16.3,-59.3	+16.3,-40.0	+16.3,-40.0
	2r-OH	-124.8	-124.8	-113.8	-99.3	-88.4	-73.9	-73.9
	3e	+29.8,-107.8	+24.3,-107.8	+21.9,-93.2	+18.7,-73.9	+16.3,-59.3	+16.3,-40.0	+16.3,-40.0
	3e-OH	-145.1	-145.1	-126.1	-101.0	-82.0	-57.0	-57.0
WALLS	3r	+29.8,-128.1	+24.3,-128.1	+21.9,-109.8	+18.7,-85.5	+16.3,-67.1	+16.3,-67.1	+16.3,-67.1
	3r-OH	-165.4	-165.4	-140.9	-108.6	-84.1	-84.1	-84.1
	4	+40.0,-43.4	+40.0,-43.4	+38.2,-41.6	+35.8,-39.2	+34.0,-37.4	+31.6,-35.0	+29.8,-33.2
	5	+40.0,-53.6	+40.0,-53.6	+38.2,-50.0	+35.8,-45.2	+34.0,-41.6	+31.6,-36.8	+29.8,-33.2

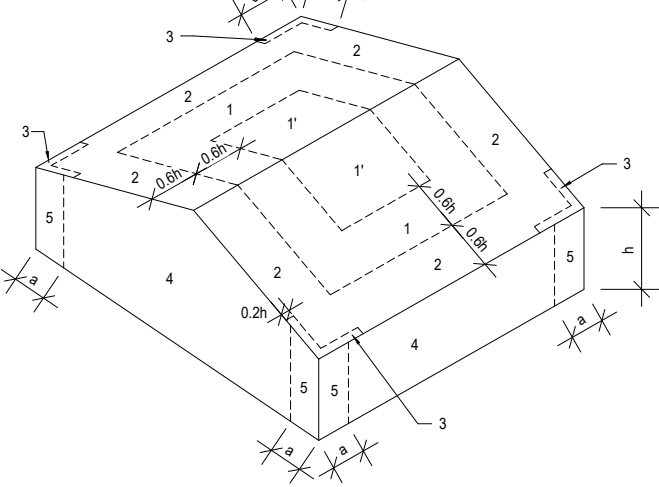
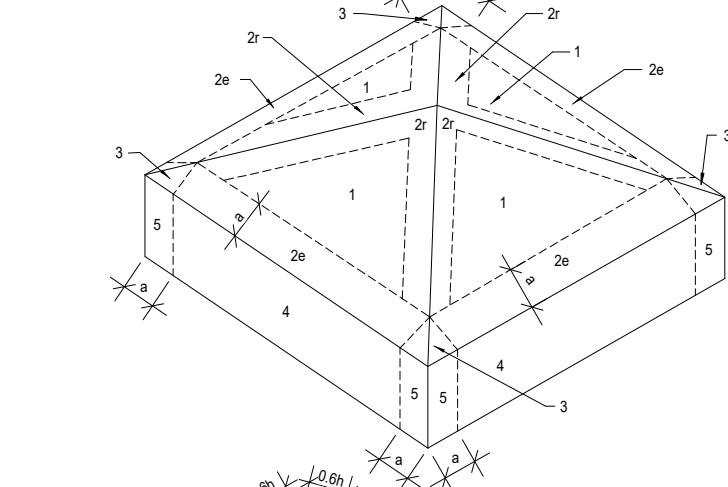
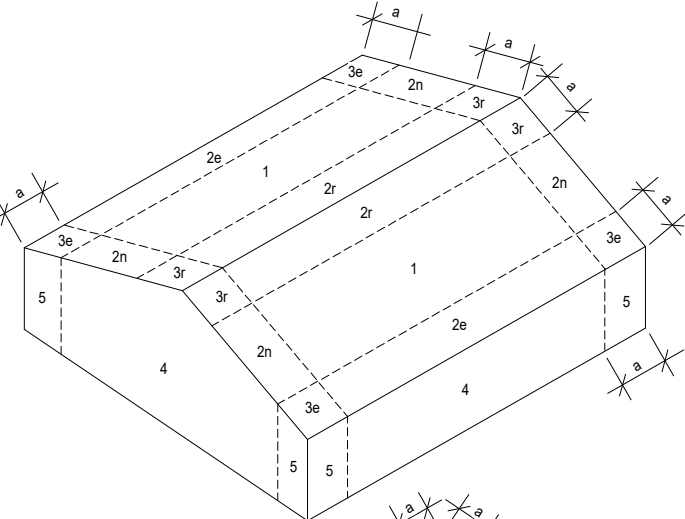
LOCATION	ZONE	EFFECTIVE WIND AREA (A), FT <sup>2</sup>					
		A = 7	A = 10	A = 50	A = 100	A = 200	A = 500
ROOF	1	+34.9,-62.6	+34.9,-62.6	+23.8,-46.0	+19.0,-38.8	+19.0,-38.8	+19.0,-38.8
	2e	+34.9,-86.4	+34.9,-86.4	+23.8,-65.1	+19.0,-55.9	+19.0,-46.7	+19.0,-46.7
	2e-OH	-106.2	-104.1	-94.6	-90.5	-86.4	-86.4
	2r	+34.9,-86.4	+34.9,-86.4	+23.8,-65.1	+19.0,-55.9	+19.0,-46.7	+19.0,-46.7
	3e	+34.9,-86.4	+34.9,-86.4	+23.8,-65.1	+19.0,-55.9	+19.0,-46.7	+19.0,-46.7
WALLS	3e-OH	-129.9	-123.2	-92.8	-79.7	-66.6	-66.6
	4	+46.7,-50.7	+46.7,-50.7	+41.9,-45.8	+39.8,-43.7	+37.6,-41.6	+34.9,-38.8
	5	+46.7,-62.6	+46.7,-62.6	+41.9,-52.8	+39.8,-48.6	+37.6,-44.4	+34.9,-38.8

LOCATION	ZONE	EFFECTIVE WIND AREA (A), FT <sup>2</sup>						
		A = 10	A = 20	A = 50	A = 100	A = 200	A = 500	A = 1000
ROOF	1	+18.9,-74.0	+17.7,-69.1	+16.1,-62.6	+16.0,-57.8	+16.0,-52.9	+16.0,-46.4	+16.0,-46.4
	1'	+18.9,-42.5	+17.7,-42.5	+16.1,-42.5	+16.0,-42.5	+16.0,-36.6	+16.0,-28.7	+16.0,-22.8
	2	+42.5,-97.6	+40.6,-91.3	+38.1,-83.0	+36.2,-76.7	+34.4,-70.5	+31.9,-62.2	+31.9,-62.2
	3	+42.5,-97.6	+40.6,-91.3	+38.1,-83.0	+36.2,-76.7	+34.4,-70.5	+31.9,-62.2	+31.9,-62.2
WALLS	4	+42.5,-46.0	+40.6,-44.1	+38.1,-41.7	+36.2,-39.8	+34.4,-37.9	+31.9,-35.4	+31.9,-35.4
	5	+42.5,-56.7	+40.6,-52.9	+38.1,-47.9	+36.2,-44.1	+34.4,-40.4	+31.9,-35.4	+31.9,-35.4

NOTES:

1. EFFECTIVE WIND AREA IS SUCH AS DEFINED BY ASCE 7 FOR EFFECTIVE WIND AREAS BETWEEN THOSE GIVEN ABOVE, THE PRESSURE MAY BE INTERPOLATED, OTHERWISE USE PRESSURE WITH THE LOWER EFFECTIVE AREA.
2. WIND PRESSURES ARE GROSS POSITIVE AND NEGATIVE ULTIMATE VALUES CALCULATED WITH THE ULTIMATE WIND SPEED (Vult) PER THE GENERAL NOTES DESIGN LOADS CRITERIA. FOR ALLOWABLE WIND PRESSURES USING NOMINAL WIND SPEED (Vasd), MULTIPLY THE ABOVE VALUES BY 0.6.
3. POSITIVE (+) WIND PRESSURE INDICATES TOWARDS THE SURFACE, NEGATIVE (-) WIND PRESSURE INDICATES AWAY FROM THE SURFACE.
4. "XX-OH" INDICATES OVERHANG WIND PRESSURE.
5. a = 6.2 FT (OFFICE); a = 4.4 FT (LOBBY); a =10.0 FT, h = 32.0 FT (HANGAR)

3. WIND DESIGN DATA:
- A. ULTIMATE WIND SPEED (Vult)135 MPH
- B. NOMINAL WIND SPEED (Vasd)105 MPH
- C. RISK CATEGORYII
- D. EXPOSURE CATEGORYC
- E. INTERNAL PRESSURE COEFF. (GCpi)+/- 0.18 (ENCLOSED)
- F. COMPONENTS AND CLADDING PRESSURESEE WIND PRESSURE PLAN
4. SEISMIC DESIGN DATA:
- A. RISK CATEGORYI
- B. SEISMIC IMPORTANCE FACTOR1.0
- C. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER (Ss)0.075
- D. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER (S1)0.005
- E. SITE CLASSD
- F. SPECTRAL RESPONSE ACCELERATION PARAMETER (Sds)0.08
- G. SPECTRAL RESPONSE ACCELERATION PARAMETER (Sd1)0.008
- H. SEISMIC DESIGN CATEGORYA



PRE-ENGINEERED METAL BUILDING:

1. THE PRE-ENGINEERED METAL BUILDING (PEMB) SHALL CONSIST OF ROOF DECK, RIGID FRAMES, METAL WALL PANELS ON FRAMING, CANOPY FRAMING, GUTTERS AND DOWNSPOUTS, AND FLASHING. DEVIATION FROM BAY SPACING SHOWN ON THE PLANS SHALL NOT BE PERMITTED TO SUIT MANUFACTURER'S STANDARDS. THE PEMB MANUFACTURER SHALL BE A MEMBER OF THE METAL BUILDING MANUFACTURER'S ASSOCIATION (MBMA).
2. THE SYSTEM SHALL BE DESIGNED AND DETAILED BY THE MANUFACTURER TO SUSTAIN THE DESIGN LOADS SPECIFIED IN 'DESIGN LOADS' GENERAL NOTES, INCLUDING WIND LOADS. REFER TO OTHER DESIGN DISCIPLINE DRAWINGS FOR OTHER LOADS NOT INDICATED HEREIN SUCH AS BUT NOT LIMITED TO HANGAR SWINGING DOORS, HANGAR BI-PARTING ROLLING DOORS, HANGAR FLOATING ROLLING DOORS, SUSPENDED FANS, WALL-MOUNTED FANS, STAIRS, CRANE RAILS, EQUIPMENT, CURTAINS, CURTAIN WALL FRAMING, OPERABLE PARTITIONS, OVERHEAD FOLDING DOORS, ETC. THE DESIGN SHALL BE IN ACCORDANCE WITH THE LATEST ISSUES OF THE AISC AND AISI SPECIFICATIONS AND MBMA 'METAL BUILDING SYSTEMS MANUAL' DESIGN PRACTICES.
3. COLUMNS SHALL BE DESIGNED AS UNBRACED BY THE MASONRY WALLS. ATTACHMENT OF PEMB COLUMNS TO WALLS SHALL BE MADE AFTER ROOF DEAD LOADS ARE APPLIED.
4. PEMB SHOP DRAWINGS AND A LETTER OF CERTIFICATION SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER. SHOP DRAWINGS SHALL INDICATE THE DESIGN LOADS AND JOB NAME AND NUMBER. THEY SHALL INCLUDE SIZES OF ALL THE FRAMING MEMBERS AND RELATED ACCESSORIES AND SPECIALIZED CONNECTIONS, THE ANCHOR BOLT PLAN AND REACTIONS. STANDARD CUT SHEETS OF THE ABOVE ARE NOT ACCEPTABLE. STANDARD CUT SHEETS MAY BE SUBMITTED FOR SECONDARY FRAMING CONNECTION DETAILS, FLASHING AND SHEETING DETAILS, ETC. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR REVIEW AND APPROVAL IN WRITING OF ANY INTERIM AND ALL FINAL PEMB SHOP DRAWINGS TO ASSURE THEY MATCH THE CONCEPT DESIGN INTENT PRIOR TO FORWARDING ONTO ENGINEER FOR REVIEW.
5. THE MANUFACTURER SHALL PROVIDE SIGNED AND SEALED CALCULATIONS FOR ALL FRAMING MEMBERS, RELATED ACCESSORIES, AND SPECIALIZED CONNECTIONS PREPARED UNDER THE SUPERVISION OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER. THE SUBMITTAL SHALL INCLUDE THE LOAD COMBINATION REACTIONS FOR EACH COLUMN SUPPORT TO THE ENGINEER OF RECORD PRIOR TO THE FINAL FOUNDATION PRICING PACKAGE SUBMITTAL. IF THE MANUFACTURER DOES NOT COMPLY, THE FOUNDATION DESIGN WILL BE ASSUMED TO RESIST THE COMBINED GRAVITY AND WIND LOADS SHOWN ON PLAN. IF THE PEMB MANUFACTURER'S REACTION BECOME AVAILABLE AFTER THE OWNER'S FINAL NEGOTIATED COST AND THE ASSUMED FOUNDATION REQUIRE LARGER FOUNDATIONS, THE INCREASE IN FOUNDATION SIZE SHALL BE PROVIDED AT NO COST TO THE OWNER OR THE ENGINEER OF RECORD.
6. PEMB ANCHOR BOLT DIAMETER SHALL BE DETERMINED BY METAL BUILDING MANUFACTURER. LENGTH AND TYPE OF BOLT REQUIRED SHALL BE NOTED ON THESE DRAWINGS.

POST-INSTALLED ANCHORS:

1. POST-INSTALLED CONCRETE ANCHORS SHALL BE AS INDICATED ON THE DRAWINGS. THE ANCHOR TYPES AND CONDITIONS LISTED BELOW ARE THE DESIGN AND DETAILING BASIS OF ALL POST-INSTALLED ANCHORAGE IN THE CONTRACT DOCUMENTS:
- A. ANCHORAGE TO CONCRETE (CRACKED) ELEMENTS USING ADHESIVE ANCHOR
- FOR FAST CURE APPLICATIONS:
- a. HILTI HIT-HY 200 MAX-A (ICC ESR-3187) WITH CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL) OR CONTINUOUSLY DEFORMED REINFORCING STEEL.
- FOR SLOW CURE APPLICATIONS:
- a. HILTI HIT-RE 500-V3 (ICC ESR-3814) WITH HILTI HIS-N OR HIS-RN INTERNALLY THREADED INSERTS, CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL) OR CONTINUOUSLY DEFORMED REINFORCING STEEL.
- b. SIMPSON STRONG-TIE SET-XP (ICC-ES ESR-2508) CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL) OR CONTINUOUSLY DEFORMED REINFORCING STEEL.
- B. ANCHORAGE TO SOLID (UNCRACKED) GROUTED MASONRY/MULTI-WYTHE BRICK USING ADHESIVE ANCHOR
- a. HILTI HIT-HY 70 (ICC ESR-2682) WITH HILTI HAS-E CONTINUOUSLY THREADED RODS, CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL), OR CONTINUOUSLY DEFORMED STEEL REBAR.
- b. SIMPSON STRONG-TIE SET (ICC-ES ESR-1772) WITH (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL), OR CONTINUOUSLY DEFORMED STEEL REBAR.
- C. ANCHORAGE TO HOLLOW MASONRY USING ADHESIVE ANCHOR
- a. SIMPSON STRONG-TIE SET (ICC-ES ESR-1772) WITH CONTINUOUSLY THREADED RODS (ASTM F1554 Gr36FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL).
- b. HILTI HIT-HY 70 (ICC ESR-3342) WITH HILTI HAS-E CONTINUOUSLY THREADED RODS, CONTINUOUSLY THREADED RODS (ASTM A193 GrB7 FOR CARBON STEEL AND ASTM F593 FOR STAINLESS STEEL), OR CONTINUOUSLY DEFORMED STEEL REBAR.
- D. ANCHORAGE USING EXPANSION BOLTS
- a. HILTI KWIK BOLT 3 (ICC ESR-2302 IN CONCRETE), (ICC ESR-1385 IN GROUT FILLED CMU) CARBON STEEL, UNO.
2. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT LISTED ABOVE FOR EACH OF THE SPECIFIC CONNECTIONS DETAILED IN THE PLANS. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC-ES AC508, ICC-ES AC608, AND/OR ICC-ES AC308 SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE, AND INSTALLATION TEMPERATURE.
3. INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.
4. OVERHEAD ADHESIVE ANCHORS MUST BE INSTALLED USING THE HILTI PROFIS SYSTEM.
5. THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
6. ANCHOR CAPACITY IS PENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
7. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY FERROSCAN, GPR, X-RAY, CHIPPING OR OTHER MEANS.

FOR PERMIT & BIDDING ONLY

TERMINAL OFFICES

TERMINAL LOBBY

HANGAR



TRANSFORMING TODAY'S IDEAS INTO TOMORROW'S REALITY

Michael Baker INTERNATIONAL

4211 W. BOY SCOUT BLVD SUITE 500 TAMPA, FL 33607

FLORIDA LICENSE #AA 26002484

JEFFREY YEAGER, P.E. FL LICENSE NO.: 62853

GENERAL NOTES

AIRPORT TERMINAL AND HANGAR DEVELOPMENT

PREPARED FOR

DEFUNIAK SPRINGS MUNICIPAL AIRPORT

DESIGNED BY:

AJM

DRAWN BY:

AJM

CHECKED BY:

JY

APPROVED BY:

JY

PROJECT NO:

2019.028.01

DATE:

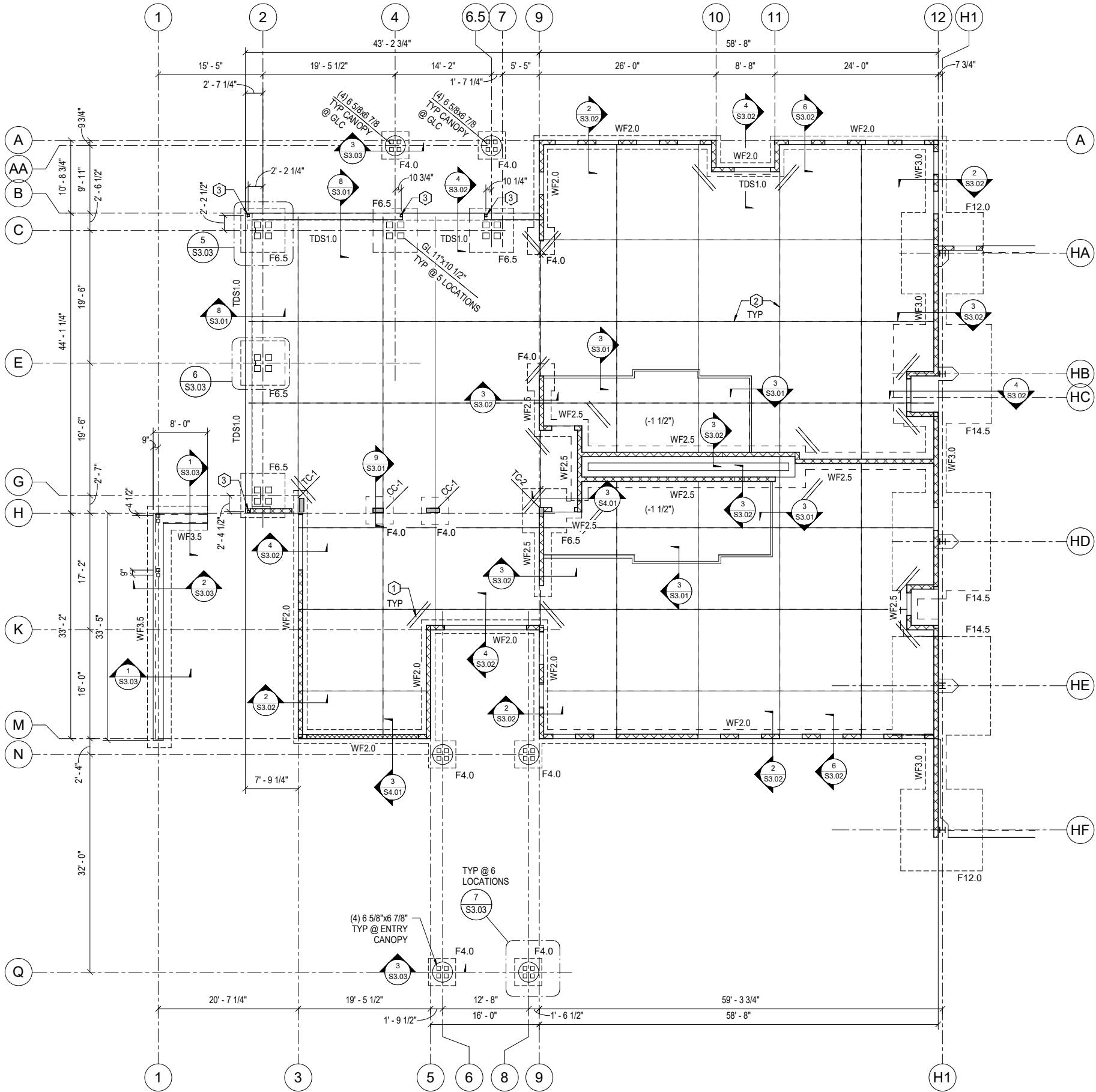
11/1/2021

SHEET NUMBER

S1.02

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1 1ST FLOOR FOUNDATION PLAN - OFFICE  
1/8" = 1'-0"

FOUNDATION PLAN NOTES

- FLOOR SLAB CONSTRUCTION SHALL BE 4" THICK 3000 PSI CONCRETE SLAB-ON-GRADE REINFORCED WITH W2.0 x W2.0 - 6 x 6 WWF PLACED IN THE TOP ONE-THIRD OF THE SLAB DEPTH OVER VAPOR RETARDANT ON COMPACTED SUB-GRADE.
  - SLAB-ON-GRADE CONSTRUCTION, SEE DETAILS 1 THRU 3 /S3.01
  - T/SLAB EL = 0'-0" - REFER TO CIVIL DWGS FOR REFERENCE ELEVATION
  - REFERENCE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR FLOOR SLOPES, DRAINS, AND DEPRESSION LOCATIONS.
  - MAINTAIN SLAB THICKNESS AT ALL FLOOR SLOPES, DRAINS, AND DEPRESSIONS.
  - REFERENCE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR CONCRETE EQUIPMENT PAD LOCATIONS, SEE DETAIL 4/S3.01
- FOOTING CONSTRUCTION SHALL BE AS INDICATED ON THE DRAWING
  - SEE FOUNDATION SCHEDULE ON SHEET S5.06 FOR THE FOLLOWING:
    - 'WFX-X' - INDICATES WALL FOOTING
    - 'FX-X' - INDICATES SPREAD FOOTING
    - 'TDSX-X' - INDICATES TURN DOWN SLAB
  - T/WALL FTG EL = -1'-4" UNO
  - T/SPREAD FTG EL = -1'-4" UNO
  - CONDUIT AND PIPE PENETRATIONS: STEP AND/OR LOWER FOUNDATIONS WHERE SHOWN AND AS NECESSARY TO AVOID INTERFERENCE WITH OTHER TRADES. SEE FOUNDATION DETAILS 7/S3.01 AND 1/S3.02
  - FOUNDATION REINFORCEMENT AT CORNERS AND INTERSECTIONS: SEE 5/S3.01
  - SEE FOUNDATION DETAILS FOR ADDITIONAL INFORMATION.
- MASONRY CONSTRUCTION SHALL BE AS INDICATED ON THE DRAWING
  - ALL CMU WALLS SHALL BE CONSTRUCTED WITH 8 INCH NOMINAL UNITS, UNO
  - COMPREHENSIVE STRENGTH OF CONCRETE MASONRY (f'm):
  - ALL CMU WALLS SHALL BE CENTERED ON FOOTINGS, UNO
  - CMU WALLS REINFORCEMENT
    - EXTERIOR BRG WALLS: #5 @ 24" OC MAX, TYP, UNO
    - INTERIOR BRG WALLS: #5 @ 24" OC MAX, TYP, UNO
  - ADDITIONAL BAR SIZES AND LOCATIONS ARE INDICATED ON THE DRAWING
  - 'TC-X' - INDICATES TIE COLUMN, SEE SCHEDULE S5.06
  - 'CC-X' - INDICATES CONCRETE COLUMNS, SEE SCHEDULE S5.06
  - NON-LOAD BRG WALLS ARE NOT SHOWN. SEE ARCH DWGS FOR LOCATIONS OF ALL NON-LOAD BRG WALLS.
  - SEE TYPICAL MASONRY DETAILS ON SHEETS S4.01 AND S4.02 FOR ADDITIONAL INFORMATION AND ADDITIONAL LOCATIONS OF REINFORCEMENT NOT CALLED OUT IN THESE NOTES OR ON THIS PLAN.
  - LOAD BEARING WALLS & EXTERIOR WALLS: 2000 PSI
- GENERAL NOTES
  - SEE GENERAL NOTES SHEETS S1.00 & 1.01 FOR ADDITIONAL INFORMATION

KEYED NOTES:

- RE-ENTRY BARS: PLACE (2) #4 x 4'-0" LONG @ 3" OC AND 3" CLR FROM CORNER AT MID-DEPTH OF THE SLAB. CENTER BARS ON CORNER.
- SLAB ON GRADE CONTROL JOINT (CJ) SEE DETAIL 1/S3.01.
- HSS4x4x1/4", SEE S3.03 FOR BASE PLATE.



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JEFFREY YEAGER, P.E.  
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FOUNDATION PLAN  
- OFFICE

AIRPORT TERMINAL  
AND HANGAR  
DEVELOPMENT

PREPARED FOR

DEFUNIAK SPRINGS MUNICIPAL AIRPORT

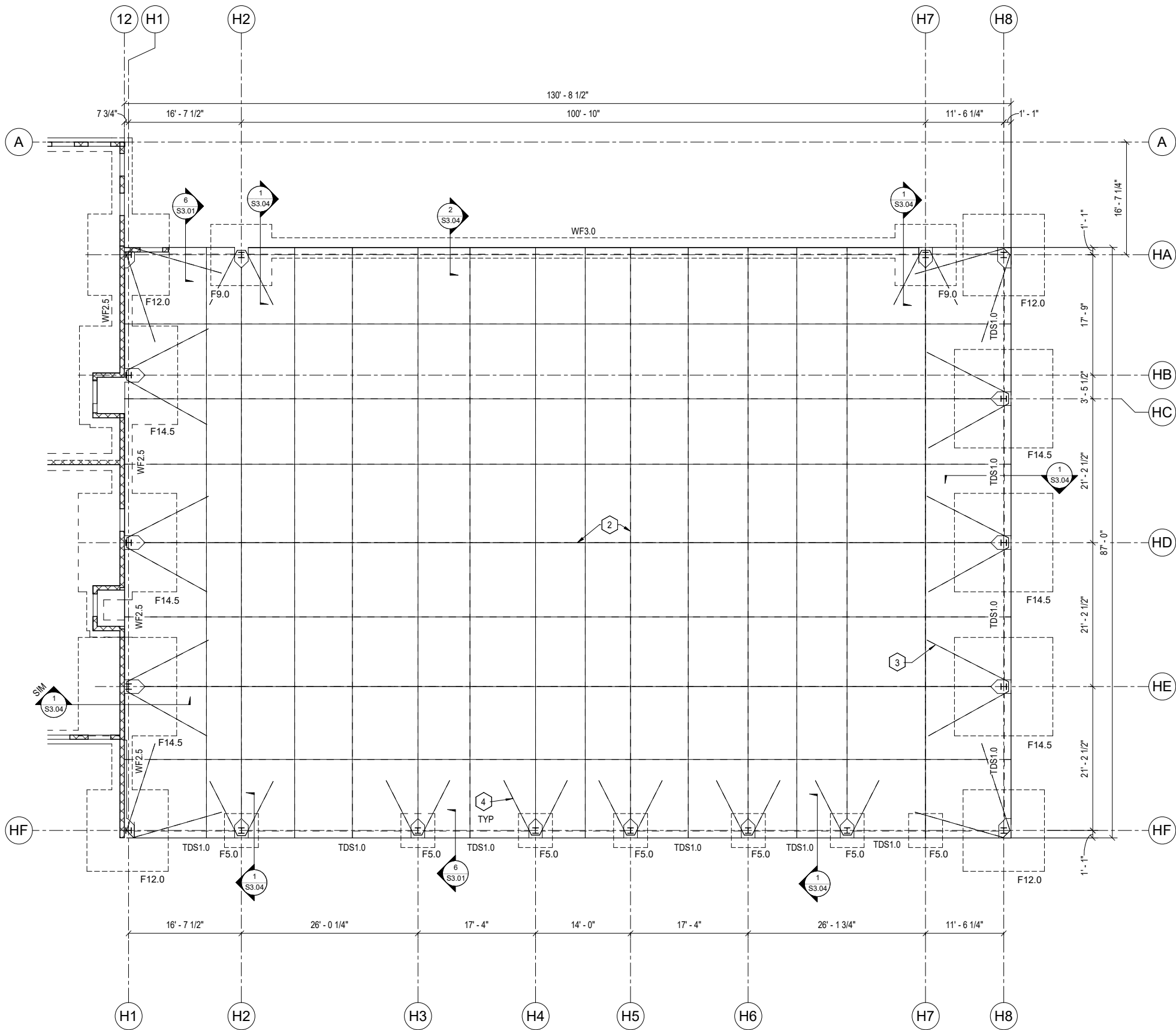
DESIGNED BY: AJM  
DRAWN BY: AJM  
CHECKED BY: JY  
APPROVED BY: JY  
PROJECT NO: 2019.028.01  
DATE: 11/1/2021

SHEET NUMBER

S2.00

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PRE-ENGINEERED METAL BUILDING FOUNDATION PLAN NOTES

- FLOOR SLAB CONSTRUCTION SHALL BE 8" THICK CONCRETE SLAB-ON-GRADE REINFORCED WITH #5 @ 8" OC, EA WAY PLACED AT MID-DEPTH SLAB AND NOVEMESH AT THE RATE OF 24 #/CY OF CONCRETE.
  - SLAB SHALL BE PLACED OVER VAPOR RETARDANT ON COMPACTED SUB-GRADE.
  - SLAB-ON-GRADE CONSTRUCTION, SEE DETAILS 1 THRU 3/S3.01
  - T/SLAB EL = 0'-0" - REFER TO CIVIL DWGS FOR REFERENCE ELEVATION
  - REFERENCE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR FLOOR SLOPES, DRAINS, AND DEPRESSION LOCATIONS.
  - MAINTAIN SLAB THICKNESS AT ALL FLOOR SLOPES, DRAINS, AND DEPRESSIONS.
  - REFERENCE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR CONCRETE EQUIPMENT PAD LOCATIONS, SEE DETAIL 4/S3.01
- FOOTING CONSTRUCTION SHALL BE AS INDICATED ON THE DRAWING
  - PRELIMINARY REACTIONS HAVE BEEN DEVELOPED BY AVCON, INC FOR INITIAL DESIGN OF THE FOUNDATIONS. GC SHALL EXPECT THE FOUNDATION SIZES AND REINFORCING TO CHANGE ONCE AVCON, INC REVIEWS THE FINAL REACTIONS PREPARED BY THE PRE-ENGINEERED METAL BUILDING MANUFACTURER'S PROFESSIONAL ENGINEER.
  - SEE FOUNDATION SCHEDULE ON SHEET S5.06 FOR THE FOLLOWING:
    - 'WFX.X' - INDICATES WALL FOOTING
    - 'FX.X' - INDICATES SPREAD FOOTING
    - 'TDSX.X' - INDICATES TURN DOWN SLAB
  - T/WALL FTG EL = -1'-4" UNO
  - T/SPREAD FTG EL = -1'-4" UNO
  - CONDUIT AND PIPE PENETRATIONS: STEP AND/OR LOWER FOUNDATIONS WHERE SHOWN AND AS NECESSARY TO AVOID INTERFERENCE WITH OTHER TRADES. SEE FOUNDATION DETAILS 7/S3.01 AND 1/S3.02
  - FOUNDATION REINFORCEMENT AT CORNERS AND INTERSECTIONS: SEE 5/S3.01
  - SEE FOUNDATION DETAILS FOR ADDITIONAL INFORMATION.

- GENERAL NOTES
  - SEE GENERAL NOTES SHEETS S1.00 & 1.01 FOR ADDITIONAL INFORMATION

KEYED NOTES:

- RE-ENTRY BARS: PLACE (2) #4 x 4'-0" LONG @ 3" OC AND 3" CLR FROM CORNER AT MID-DEPTH OF THE SLAB
- SLAB ON GRADE CONTROL JOINTS (CJ) SEE DETAIL 1/S3.01
- #5 x 20'-0" HAIRPIN EA LEG, TYP. ALONG GRID LINES H1 & H8 - MUST BE PLACED TIGHT TO EXTERIOR COLUMN FLANGE.
- #5 x 12'-0" HAIRPIN EA LEG, TYP. ALONG GRIDLINES HA & HF - MUST BE PLACED TIGHT TO EXTERIOR COLUMN FLANGE.

1 1ST FLOOR FOUNDATION PLAN - HANGAR  
1/8" = 1'-0"

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FOUNDATION PLAN  
- HANGAR

100% BID DRAWINGS

AIRPORT TERMINAL  
AND HANGAR  
DEVELOPMENT

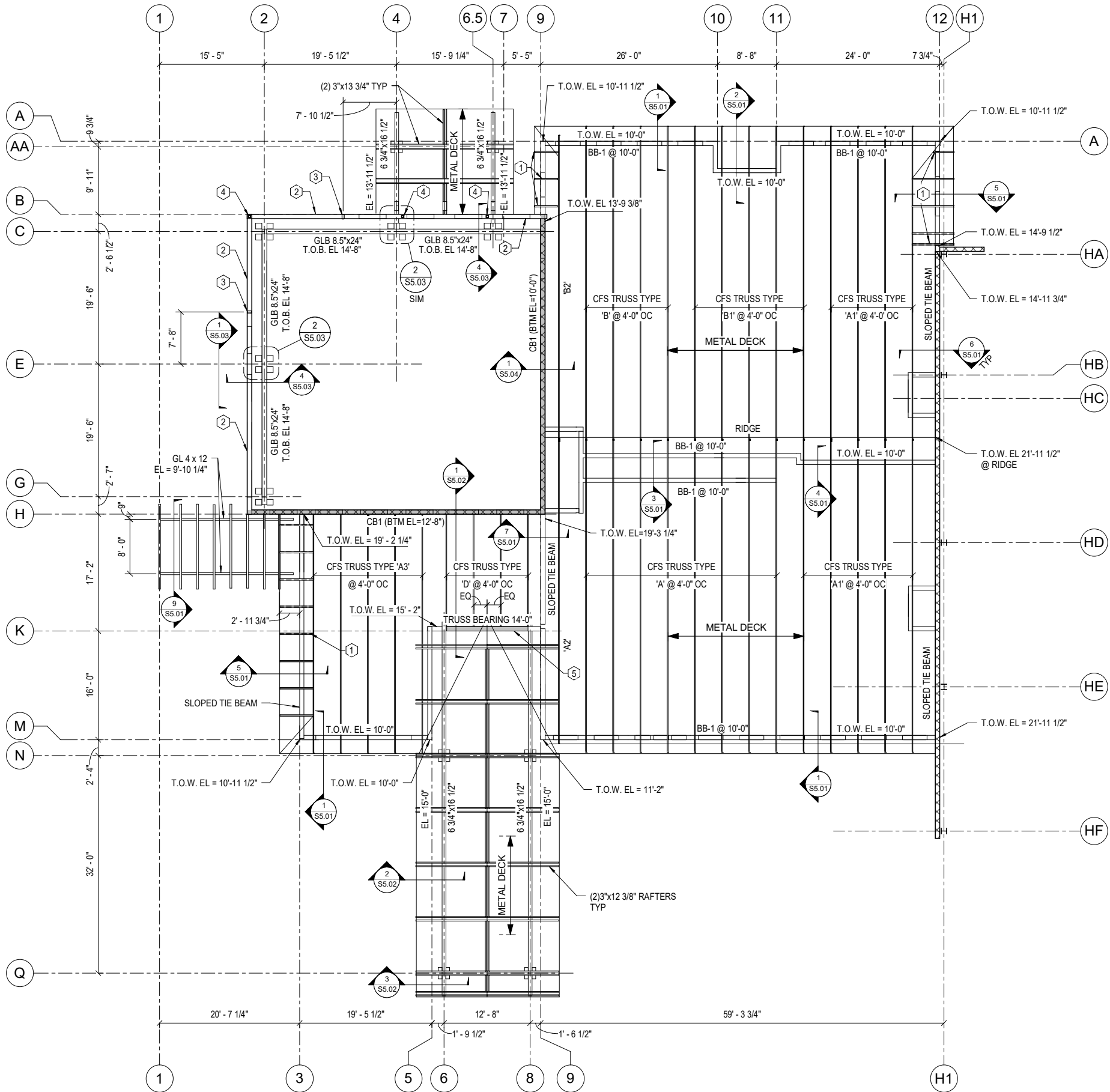
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1 ROOF PLAN - OFFICE  
1/8" = 1'-0"

ROOF PLAN NOTES:

- ROOF CONSTRUCTION SHALL CONSIST OF 1-1/2" x 20 GA. TYPE 'B' GALV G90 METAL DECK ON PRE-ENGINEERED CFS TRUSSES SPACED AT 4'-0" OC, MAX.
  - CONNECT METAL DECK TO TRUSSES WITH #12 TEK SCREWS AT 36/7 PATTERN OF THE SUPPORTS AND 12" OC AT EXTREME ENDS.
  - METAL DECK SIDE LAPS SHALL BE (1) #10 TEK SCREWS BETWEEN EACH SUPPORT.
  - DECK FASTENERS SHALL BE CONCEALED AND NOT VISIBLE FROM THE BOTTOM FINISH OF EXPOSED DECK.
- CANOPY ROOF CONSTRUCTION SHALL CONSIST OF 2" x 20 GA. VERSA-DEK LS ON GLULAM BEAMS SPACED AS SHOWN.
  - CONNECT DECK TO GLB'S WITH #12 TEK SCREWS 3" MINIMUM PENETRATION AT 24/4 PATTERN AT SUPPORTS AND 6" OC AT EXTREME ENDS.
  - SIDE LAP FASTENERS SHALL BE (2) #10 TEK SCREWS BETWEEN EACH SUPPORT.
  - DECK FASTENERS SHALL BE CONCEALED AND NOT VISIBLE FROM THE BOTTOM FINISH OF EXPOSED DECK.
- CFS TRUSSES
  - CFS TRUSSES SHALL BEAR IN MASONRY WALL - PROVIDE BEARING STIFFENER AT EACH BEARING LOCATION
  - ATTACH CFS TRUSSES TO EXTERIOR MASONRY WALLS AS INDICATED ON THE DRAWINGS
  - PROVIDE CONTINUOUS CFS BLOCKING / CHORD MEMBERS BETWEEN TRUSSES ON TOP OF BEARING WALLS. BLOCKING DEPTH SHALL MATCH THE DEPTH OF THE TRUSS AT BEARING. THE TOP FLANGE SHALL HAVE A MINIMUM THICKNESS OF 54 MILS AND SHALL BE SLOPED TO MATCH THE PITCH OF THE ROOF TO PROVIDE CONNECTION FOR THE METAL DECK. THE BLOCKING SHALL BE CONNECTED TO THE TOP OF THE WALL TO RESIST AN IN-PLANE LATERAL LRFD LOAD OF 400 PLF.
  - PROVIDE CONTINUOUS CLOSURE PLATES WITH A MINIMUM THICKNESS OF 54 MILS AT RIDGE, HIP, AND EAVE LOCATIONS. CONNECT THE PLATES TO SUPPORTING MEMBERS WITH #10 TEK SCREWS OR WELDS SPACED NO MORE THAN 12" OC
- GLULAM FRAMING
  - 'GLB-X' DESIGNATED GLULAM BEAM
  - GLULAM COLUMNS SHALL BE LACED PER DETAIL 5/S5.04
- MASONRY/CONCRETE
  - SEE BOND BEAM / CONCRETE BEAM / TIE BEAM SCHEDULE FOR THE FOLLOWING:
    - 'BB-X' - INDICATES BOND BEAM, SEE SCHEDULE 1/S5.06
    - 'TB-X' - INDICATES TIE BEAM, SEE SCHEDULE 1/S5.06
    - 'CB-X' - INDICATES CONCRETE BEAM, SEE SCHEDULE 1/S5.06
  - TOP OF WALL EL SHALL BE AS INDICATED ON THE DRAWINGS
  - TOP OF INTERIOR WALL SHALL BE TERMINATED WITH A 8" x 16" DEEP BOND BEAM W/ (2) #5 CONTINUOUS TOP & BOTTOM OR WITH TIE BEAM AS ON THE EXTERIOR WALL
- GENERAL NOTES
  - SEE GENERAL NOTES FOR ADDITIONAL INFORMATION

KEYED NOTES:

- COLD-FORMED STEEL OUTLOOKERS BY TRUSS MANUFACTURER. 2 1/2" DEEP x 54 MILS MINIMUM @ 2'-0" OC MAX. SEE DETAIL 5/S5.01
- CONT HSS8x4x5/16" @ T/STL = 13'-0". SEE DETAIL 2 & 4/S5.03 FOR CONNECTION TO GLC SEE DETAIL 8/S5.01 FOR CONNECTION TO CMU WALLS. CONNECT TO CORNER GLC IN BOTH DIRECTIONS.
- HSS8x4x5/16" COLUMN BETWEEN CFS WALL & STOREFRONT. WELD TO INTERMEDIATE & TOP HSS BEAM W/BEVEL WELD & PARTIAL PENETRATION GROOVE WELD.
- HSS 4x4x1/4" COLUMNS @ ENTRY JAMBS. WELD TO UNDERSIDE OF HSS8x4x5/16" W/ 1/4" FILLET WELD ALL AROUND.
- HSS12x6x5/16" HEADER, BOTTOM ELEVATION = 7'-8".



TRANSFORMING TODAY'S IDEAS  
INTO TOMORROW'S REALITY

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ROOF PLAN -  
OFFICE

100% BID DRAWINGS

AIRPORT TERMINAL  
AND HANGAR  
DEVELOPMENT

PREPARED FOR  
DEFUNIAK SPRINGS MUNICIPAL AIRPORT

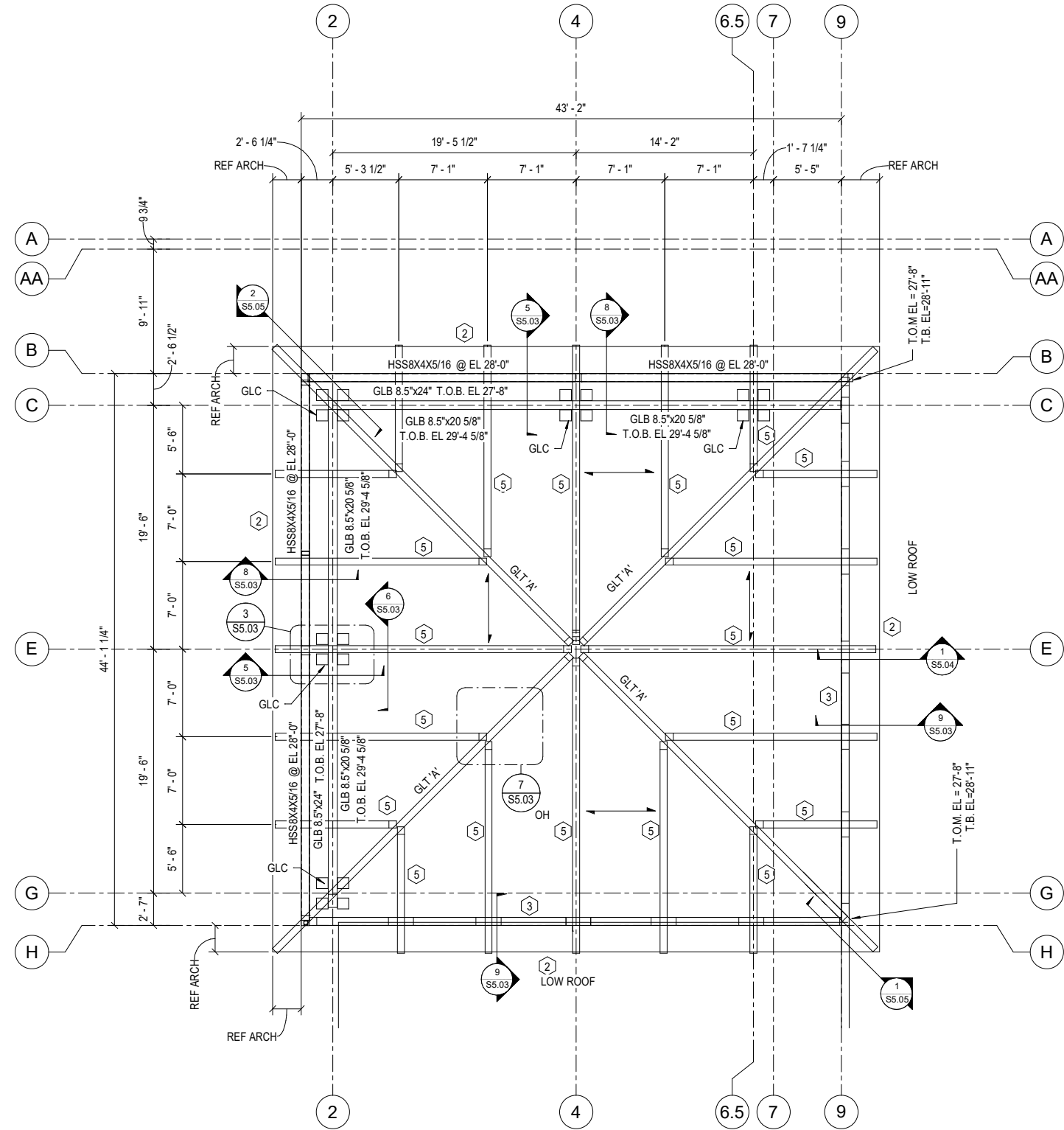
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APPROVED BY: JY  
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1 HIGH ROOF PLAN - OFFICE  
3/16" = 1'-0"

HIGH ROOF PLAN NOTES

- ROOF CONSTRUCTION SHALL CONSIST OF 2" x 20 GA. VERSA-DEK ACOUSTICAL LS METAL DECK ON PRE-ENGINEERED GLULAM TRUSSES, BEAMS OR PURLINS.
  - CONNECT DECK TO GLT'S AND GLB'S WITH #12 TEK SCREWS 3" MINIMUM PENETRATION AT 24/4 PATTERN AT SUPPORTS AND 6" OC AT EXTREME ENDS.
  - CONNECT DECK TO STEEL OR CFS SUPPORTS WITH #12 TEK SCREWS AT 24/4 PATTERN AT SUPPORTS AND 6" OC AT EXTREME ENDS.
  - SIDE LAP FASTENERS SHALL BE (2) #10 TEK SCREWS BETWEEN EACH SUPPORT
  - DECK FASTENERS SHALL BE CONCEALED AND NOT VISIBLE FROM THE BOTTOM FINISH OF EXPOSED DECK.
- MASONRY/CONCRETE
  - SEE BOND BEAM / CONCRETE BEAM / TIE BEAM SCHEDULE FOR THE FOLLOWING:
    - BB-X' - INDICATES BOND BEAM, SEE SCHEDULE 1/S5.06
    - TB-X' - INDICATES TIE BEAM, SEE SCHEDULE 1/S5.06
    - CB-X' - INDICATES CONCRETE BEAM, SEE SCHEDULE 1/S5.06
  - TOP OF WALL EL SHALL BE AS INDICATED ON THE DRAWINGS
  - TOP OF INTERIOR WALL SHALL BE TERMINATED WITH A 8" x 16" DEEP BOND BEAM W/ (2) #5 CONTINUOUS TOP & BOTTOM OR WITH TIE BEAM AS ON THE EXTERIOR WALL.
- GLULAM NOTES
  - GLT "X" - DESIGNATES GLULAM TRUSS
  - GLB - DESIGNATES GLULAM BEAMS
  - GLC - DESIGNATES GLULAM COL. REF DETAIL 3/S5.03
  - HANG GLB 3.5'S FROM GLT "A" & GLB 6 3/4" W/CENTER WEB OF PURLINS & LAG SCREWS THRU TOP OF GLB OR GLT.
  - DOUBLE TAPER TOPS OF GLT'S FOR METAL DECK BEARING AND CONNECTIONS.
  - GLULAM COLUMNS SHALL BE LACED TOGETHER PER DETAIL 5/S5.04
  - TAPER TOPS OF PERIMETER GLB'S FOR METAL DECK BEARING AND CONNECTIONS.
- GENERAL NOTES
  - SEE GENERAL NOTES FOR ADDITIONAL INFORMATION

KEYED NOTES:

- FOR TRUSS CONNX SEE 1/S5.05 FOR INFO.
  - CONNECT METAL DECK TO ALL GLULAM PURLINS, BEAMS, AND TRUSSES W/ #12 TEK SCREWS (3" MIN PENETRATION) @ 36/7 PATTERN AND 12" OC @ EXTREME ENDS.
- CONT BENT PLATE 3x3x5/16" - ATTACH TO TOP OF GLB & GLT W/ 1/2" DIA x 6" LAG SCREW @ EA LOCATION AND AT EA END.
- TB-4 @ EL = 28'-11" BETWEEN GLB ALONG GRID LINES 'H' & '9'. LEAVE 2'-0" WIDE POCKET @ EACH GLB.
- HSS8x4x5/16" COLUMN ATTACHED TO UNDERSIDE OF HSS BEAM.
- GLB 6 3/4" x 17 7/8" (SL)



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HIGH ROOF PLAN  
OFFICE

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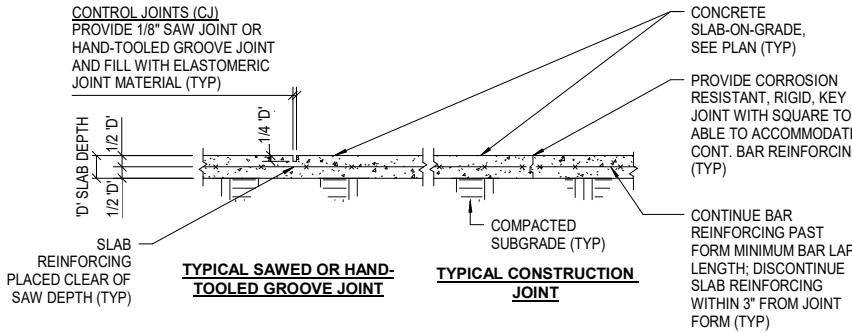
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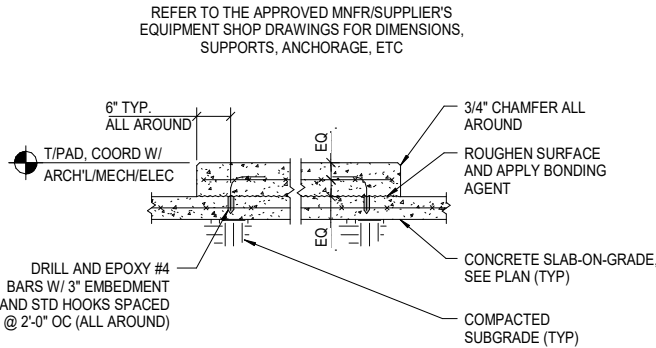
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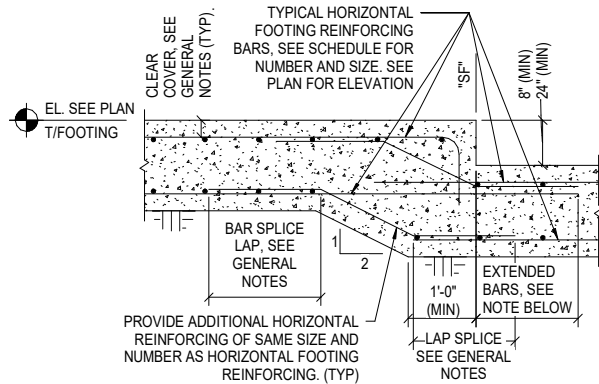


- SLAB-ON-GRADE NOTES**
- FORMED CONSTRUCTION JOINTS AND HAND-TOOLED CONTROL JOINTS ARE EQUAL SUBSTITUTES. PROVIDE HAND-TOOLED GROOVE JOINTS AT EXTERIOR SLABS ONLY AND WHERE SAWED JOINTS SAW BLADE IS OBSTRUCTED.
  - SAWED CONTROL JOINTS SHALL BE PLACED AS SOON AS POSSIBLE WITHOUT RAVELING CONCRETE OR DISLODGING AGGREGATE. CLEAN SAWED/TOOLED JOINT AND APPLYING JOINT MATERIAL PER MANUFACTURER'S INSTRUCTIONS.
  - SEE PLAN FOR JOINT SPACING. DO NOT EXCEED 30xD (EACH DIRECTION) JOINT SPACING OR MAXIMUM 10'-0" FOR 4" SLAB. WHERE TOP OF SLAB SURFACES ARE TO BE FINISHED WITH TILE OR OTHER FEATURES, GENERAL CONTRACTOR SHALL COORDINATE JOINT LOCATIONS WITH TILE MORTAR JOINTS. JOINT PATTERN SHALL NOT EXCEED 1.5 TO 1 ASPECT RATIO.

1 TYPICAL SLAB-ON-GRADE DETAIL  
3/4" = 1'-0"

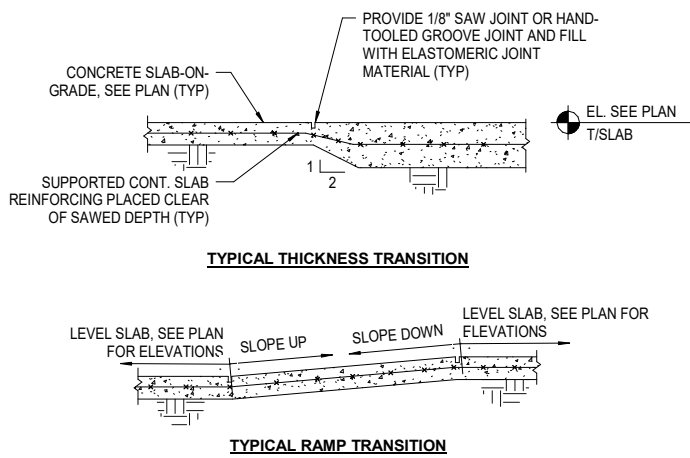


4 TYPICAL HOUSKEEPING PAD DETAIL  
3/4" = 1'-0"



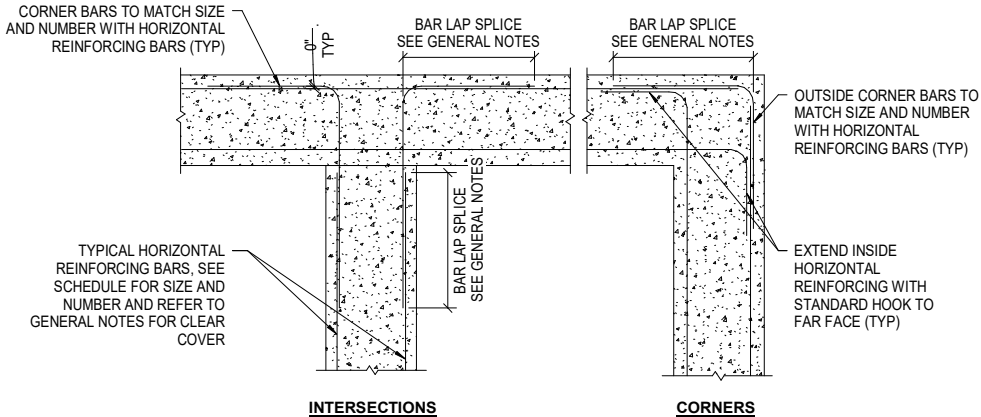
- NOTES:**
- "SF" INDICATES STEP FOOTING (AS REQUIRED). GENERAL CONTRACTOR SHALL COORDINATE STEP FOOTING LOCATIONS WITH WALL PIPE SLEEVES.
  - EXTEND TYPICAL HORIZONTAL BAR 48xBAR DIA. BEYOND STEP FOOTING VERTICAL CONCRETE FACE WHERE POSSIBLE, ELSE PROVIDE STANDARD HOOK.

7 TYPICAL WALL STEP FOOTING DETAIL  
3/4" = 1'-0"

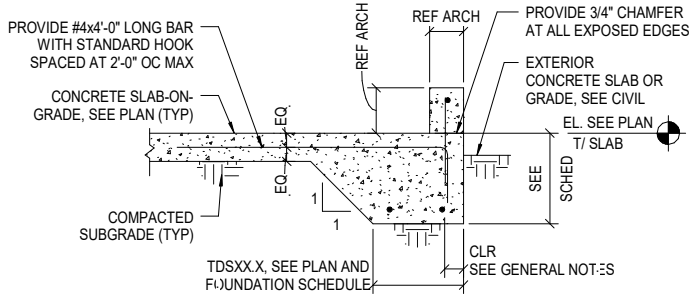


- NOTE:**
- REFER TO TYPICAL SLAB-ON-GRADE DETAIL FOR ALL OTHER INFORMATION.

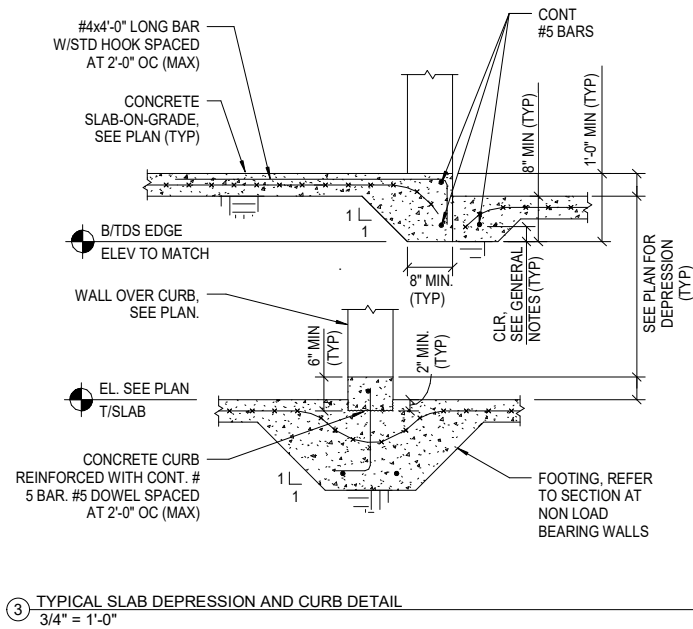
2 TYPICAL SLAB TRANSITION DETAIL  
3/4" = 1'-0"



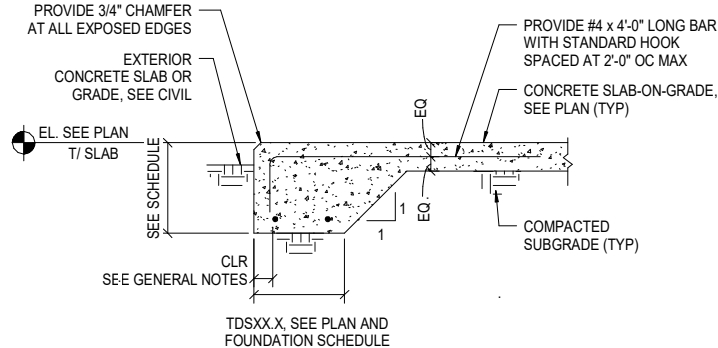
5 TYPICAL HORIZ REINF AT FOOTINGS/BEAMS/WALLS  
3/4" = 1'-0"



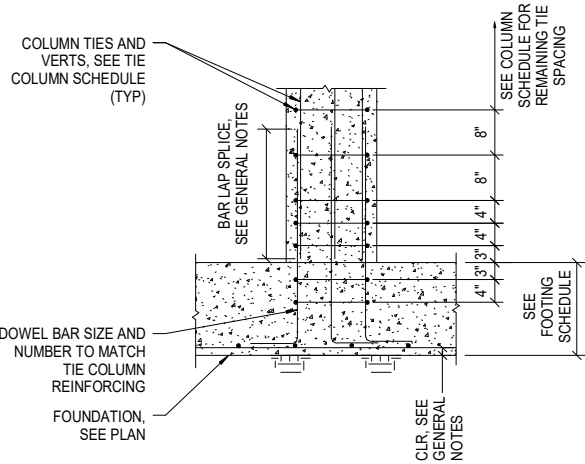
8 TYPICAL TDS EDGE W/CURB DETAIL  
3/4" = 1'-0"



3 TYPICAL SLAB DEPRESSION AND CURB DETAIL  
3/4" = 1'-0"



6 TYPICAL TDS EDGE DETAIL  
3/4" = 1'-0"



9 TYPICAL CONCRETE COLUMN (CC#)  
3/4" = 1'-0"

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FOUNDATION  
DETAILS

100% BID DRAWINGS

AIRPORT TERMINAL  
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DEVELOPMENT

PREPARED FOR

DEFUNIAK SPRINGS MUNICIPAL AIRPORT

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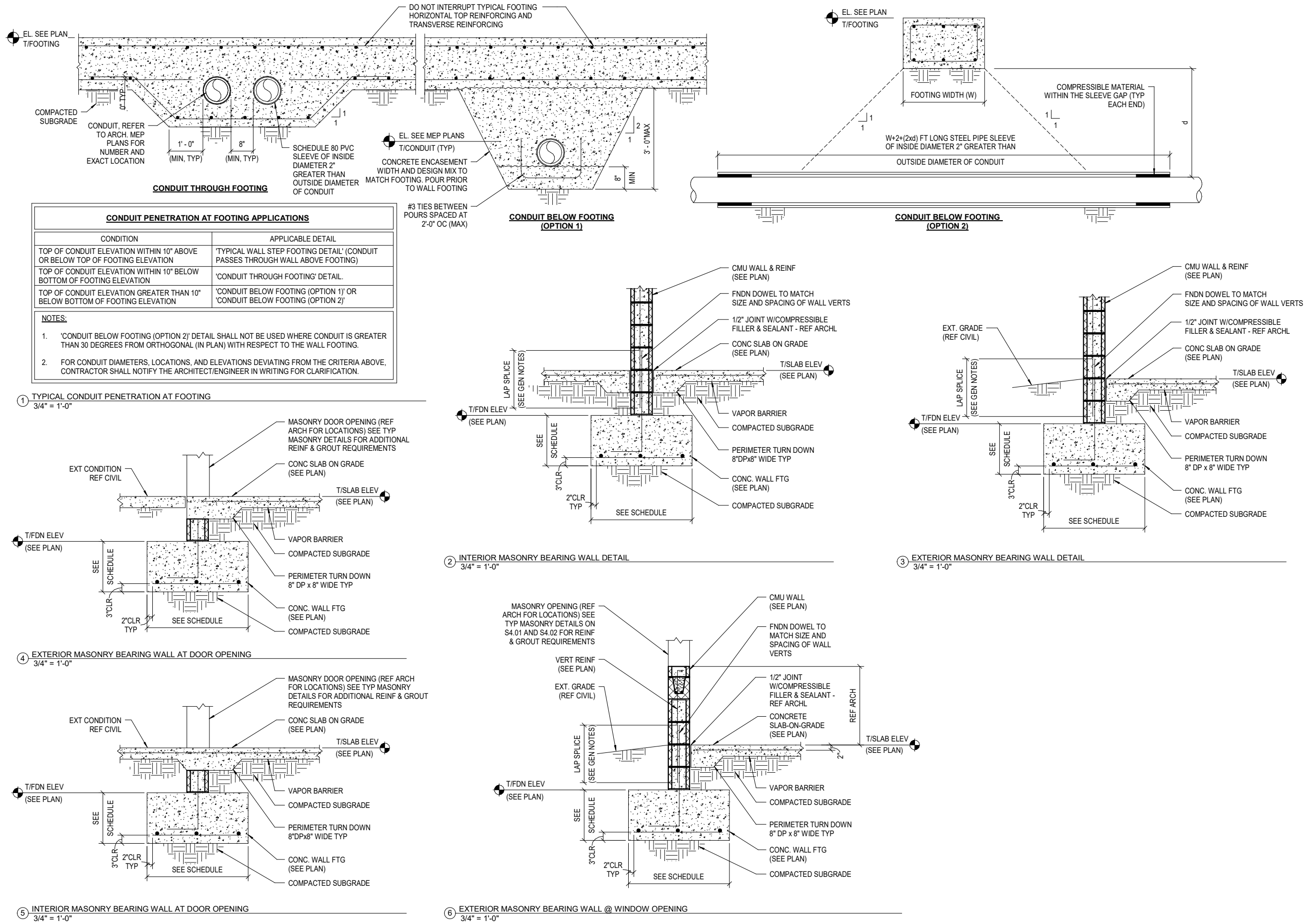
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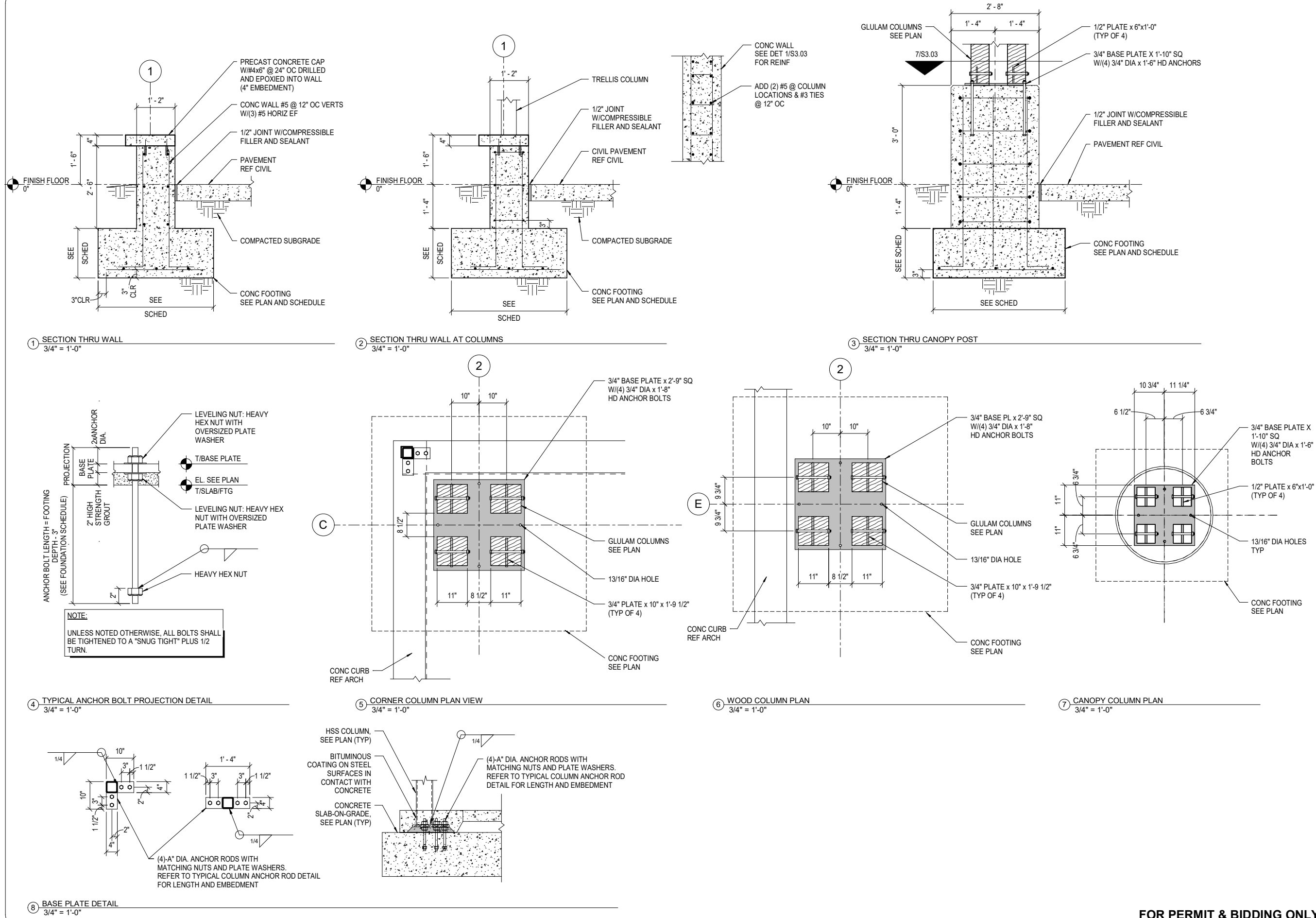
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PEMB DETAILS

100% BID DRAWINGS

**AIRPORT TERMINAL  
AND HANGER  
DEVELOPMENT**

PREPARED FOR

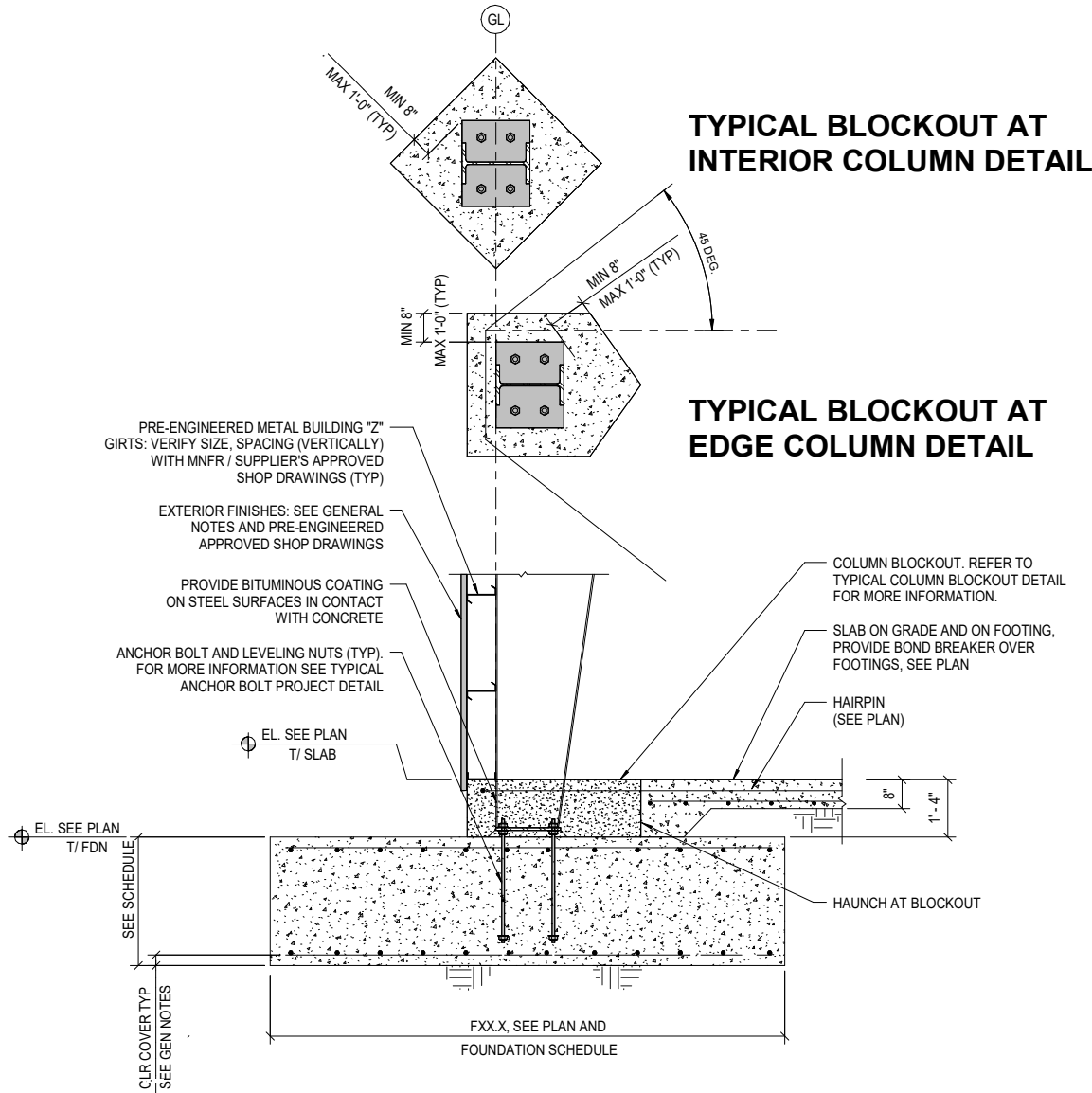
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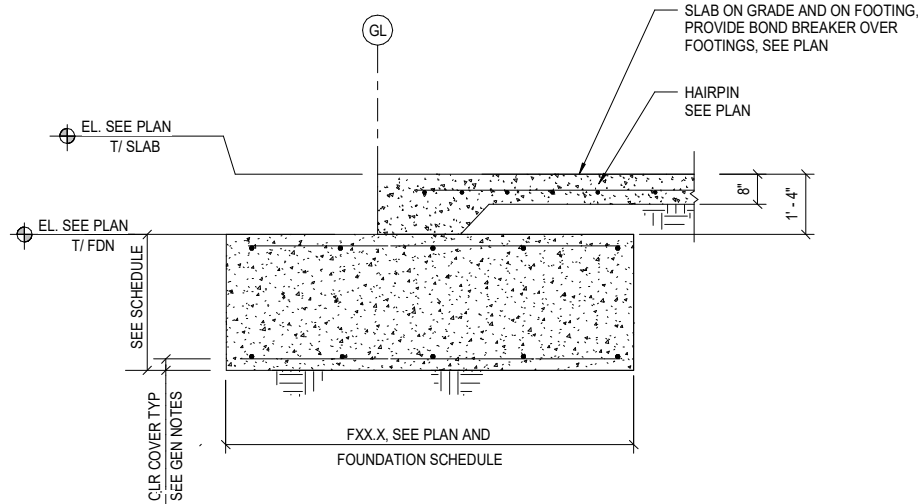
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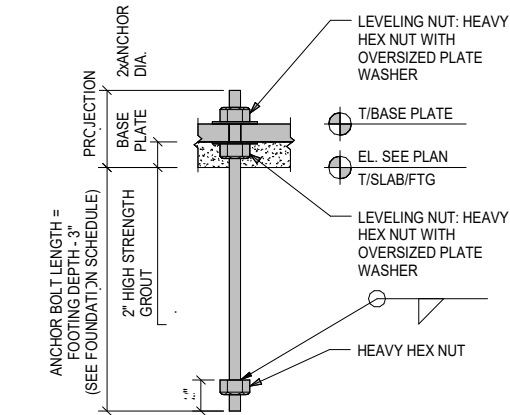
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① PEMB FOUNDATION DETAIL  
1/2" = 1'-0"



② PEMB FOUNDATION DETAIL AT HANGER DOOR  
1/2" = 1'-0"

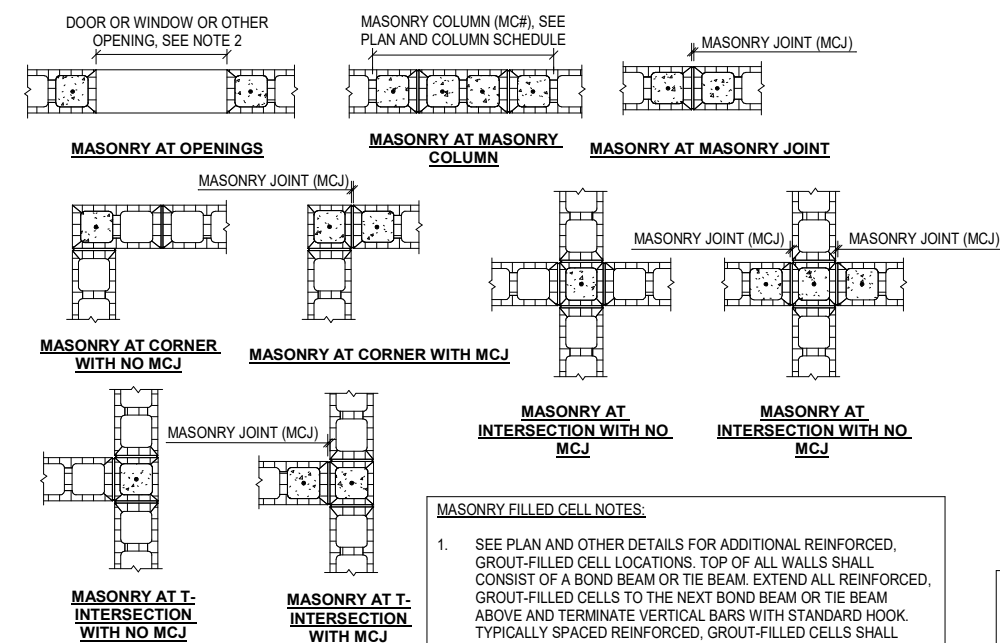


**NOTE:**

- UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE TIGHTENED TO A "SNUG TIGHT" PLUS 1/2 TURN.
- REFER TO THE APPROVED PEMB SHOP DRAWINGS FOR THE NUMBER AND DIAMETER OF ANCHOR BOLTS.

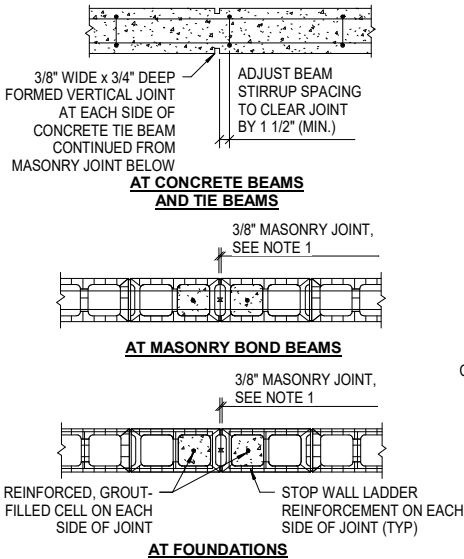
③ PEMB ANCHOR BOLT DETAIL  
3/4" = 1'-0"

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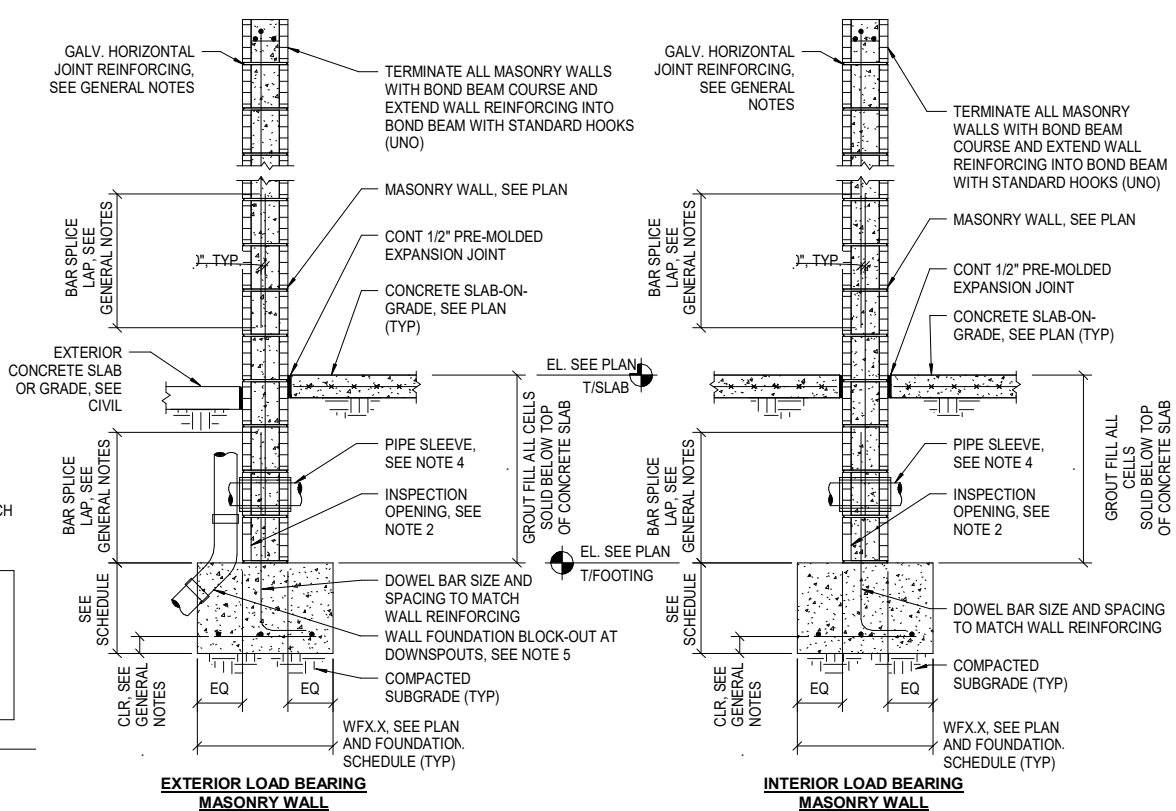
- MASONRY FILLED CELL NOTES:**
- SEE PLAN AND OTHER DETAILS FOR ADDITIONAL REINFORCED, GROUT-FILLED CELL LOCATIONS. TOP OF ALL WALLS SHALL CONSIST OF A BOND BEAM OR TIE BEAM. EXTEND ALL REINFORCED, GROUT-FILLED CELLS TO THE NEXT BOND BEAM OR TIE BEAM ABOVE AND TERMINATE VERTICAL BARS WITH STANDARD HOOK. TYPICALLY SPACED REINFORCED, GROUT-FILLED CELLS SHALL EXTEND AND TERMINATE AT TOP-MOST BOND BEAM OR TIE BEAM.
  - FOR OPENINGS GREATER THAN 12'-0" WIDE, PROVIDE TWO REINFORCED, GROUT-FILLED CELLS ON EACH SIDE OF OPENINGS.

① TYP PLAN LOCATIONS OF MASONRY GROUT FILLED CELLS  
3/4" = 1'-0"



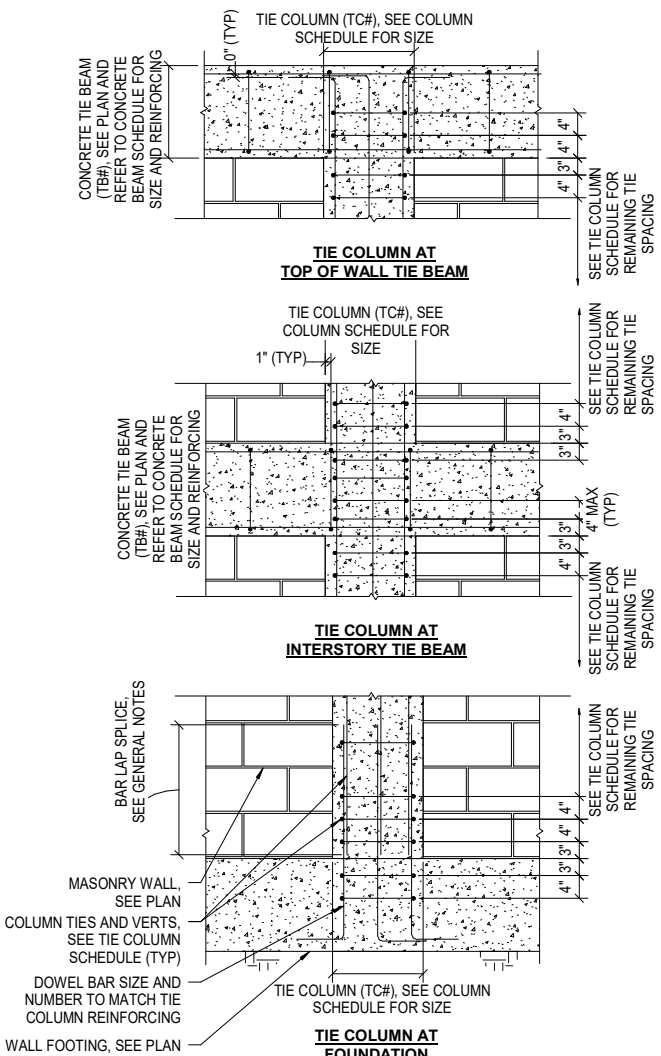
- MASONRY JOINT NOTES:**
- REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION OF JOINT AND TYPE OF SEALANT. MASONRY CONTROL JOINT SPACING SHALL NOT EXCEED 3xWALL HEIGHT OR 30'-0" (MAX.).
  - CONTINUE HORIZONTAL BOND/TIE BEAM REINFORCEMENT THROUGH CONTROL JOINT

② TYPICAL MASONRY JOINT DETAIL  
3/4" = 1'-0"

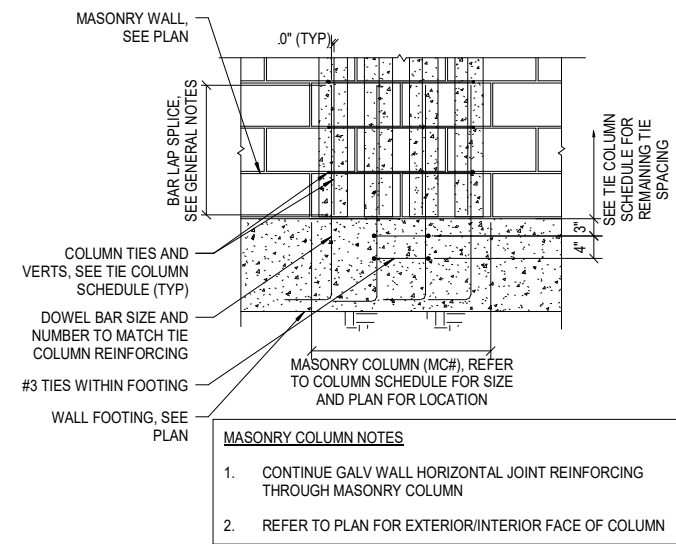


- LOAD BEARING MASONRY WALL NOTES:**
- MAXIMUM POUR LIFT HEIGHT SHALL BE 12'-0".
  - INSPECTION OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED IN EACH POUR IN EXCESS OF 5 FEET IN HEIGHT AND SHALL BE SEALED BEFORE GROUTING, AFTER INSPECTION.
  - CENTER VERTICAL REINFORCING BARS AND MATCHING DOWELS WITHIN THE CELL AND SUPPORTED BARS AT A MAXIMUM 192xBAR DIAMETERS.
  - PROVIDE SCHEDULE 80 PVC SLEEVE OF INSIDE DIAMETER 2" GREATER THAN OUTSIDE DIAMETER OF CONDUIT, TYPICAL
  - COORDINATE CENTER OF DOWNSPOUT BLOCKOUT OF LENGTH PIPE DIAMETER + 4" WITH CENTER OF DOWNSPOUT. INTERRUPT CONTINUOUS FOOTING BAR AND TERMINATE WITH STANDARD HOOK. ADD ONE 6'-0" LONG BAR OF SIZE TO MATCH FOOTING REINFORCING. DO NOT APPLY THIS DETAIL AT COLUMN FOOTINGS.

③ TYPICAL EXTERIOR/INTERIOR LOAD BEARING MASONRY WALL DETAIL  
3/4" = 1'-0"



④ TYPICAL CONCRETE TIE COLUMN (TC#)  
3/4" = 1'-0"



⑤ TYPICAL MASONRY COLUMN (MC#)  
3/4" = 1'-0"



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**MASONRY DETAILS**

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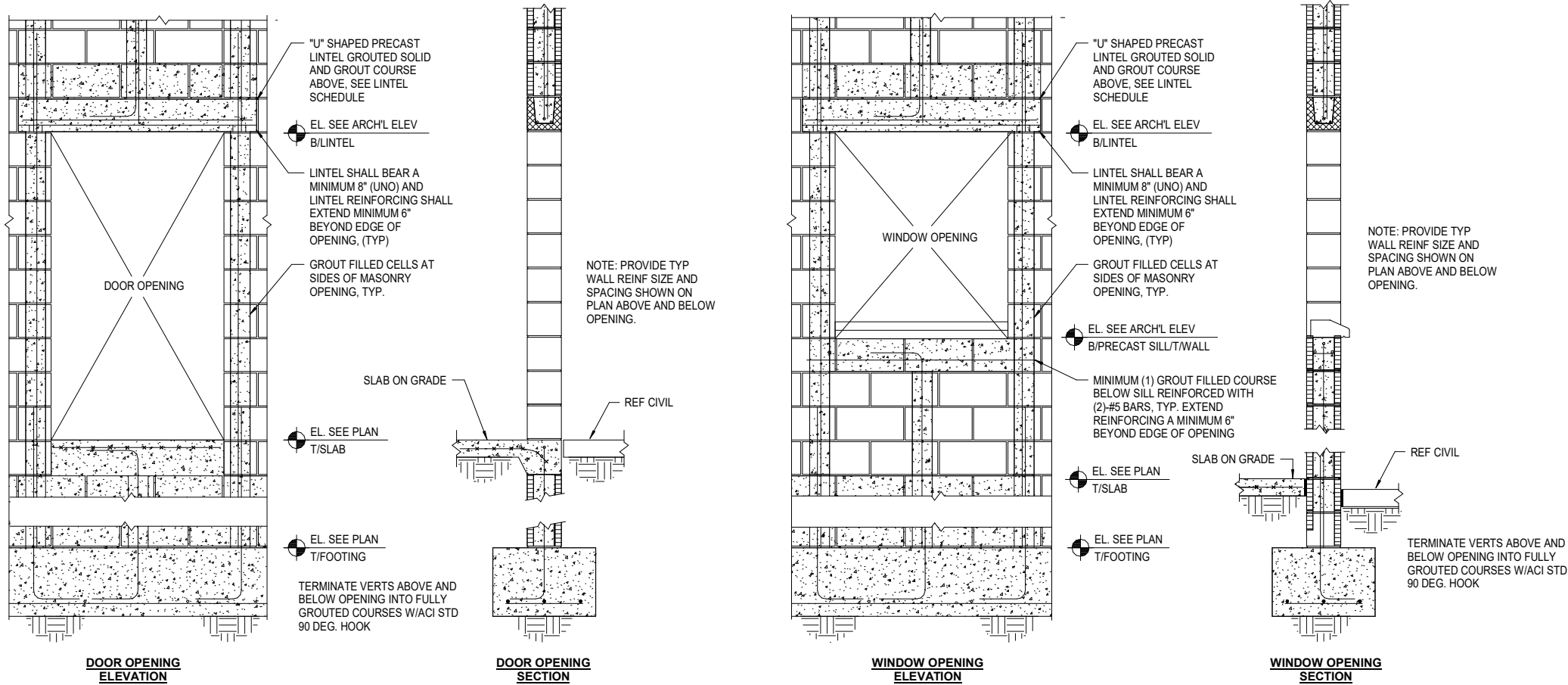
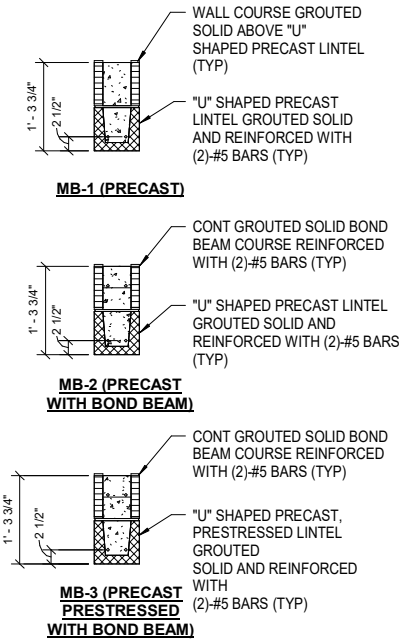
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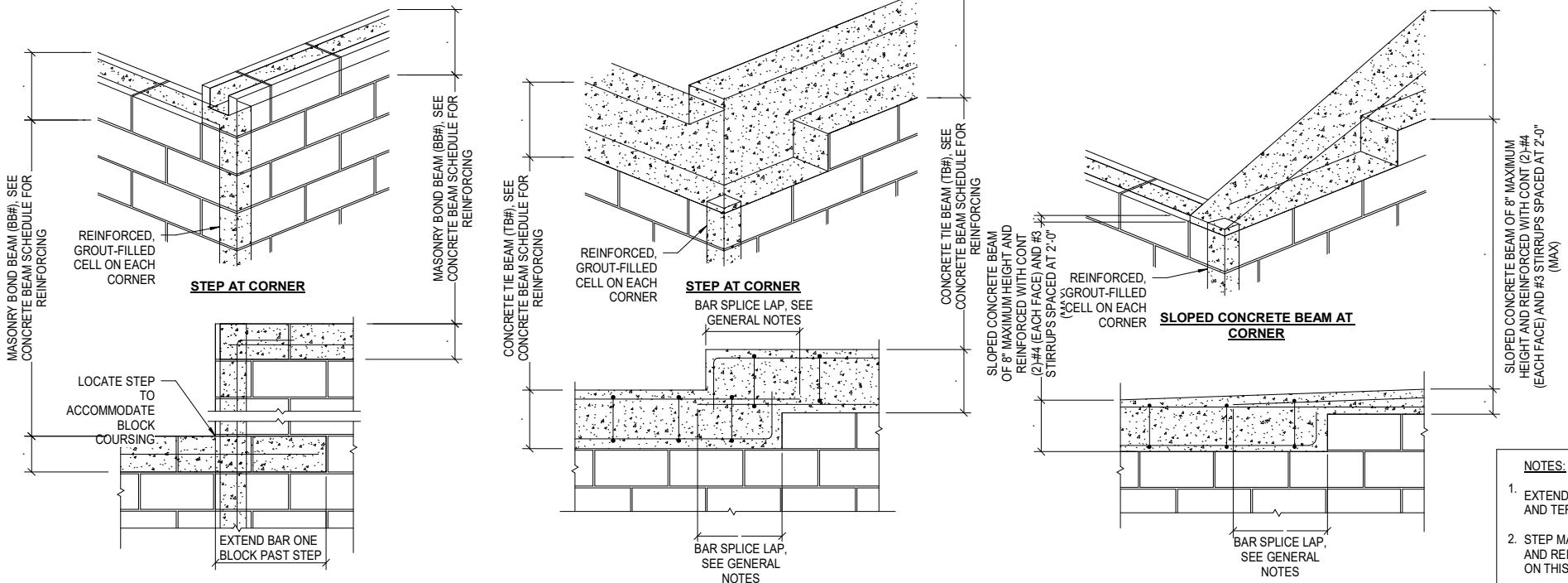


MASONRY LINTEL SCHEDULE		
MARK	LENGTH (L)	CAST-CRETE MARK
MB1	> 2'-10" TO 5'-10"	8F16-1B/OT PRECAST
MB2	> 5'-10" TO 14'-0"	8F16-1B/1T PRECAST
MB3	> 14'-0" TO 21'-4"	8F16-1B/2T PRESTRESSED



- MASONRY LINTEL NOTES:**
- MASONRY LINTEL SUBSTITUTIONS MUST BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO SECURING MATERIALS.
  - SHORE PRECAST "U" LINTELS PER MANUFACTURER'S RECOMMENDATIONS.
  - REFER TO TYPICAL EXTERIOR LOAD BEARING MASONRY WALL DETAIL AND TYPICAL EXTERIOR LOAD BEARING MASONRY WALL AT DOOR FOR MORE INFORMATION.

1 TYPICAL MASONRY LINTELS AT OPENINGS  
3/4" = 1'-0"



2 TYPICAL STEPPED BOND BEAM AND SLOPED CONCRETE BEAMS  
3/4" = 1'-0"

- NOTES:**
- EXTEND ALL WALL VERTICAL BARS TO TOP-MOST COURSE AND TERMINATE WITH STANDARD HOOKS.
  - STEP MASONRY WALL AND PROVIDE MINIMUM BEAM DEPTH AND REINFORCING AT ALL SLOPED TIE BEAMS AS SHOWN ON THIS DETAIL.

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TAMPA, FL 33607  
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JEFFREY YEAGER, P.E.  
FL LICENSE NO.: 62853

**MASONRY DETAILS**

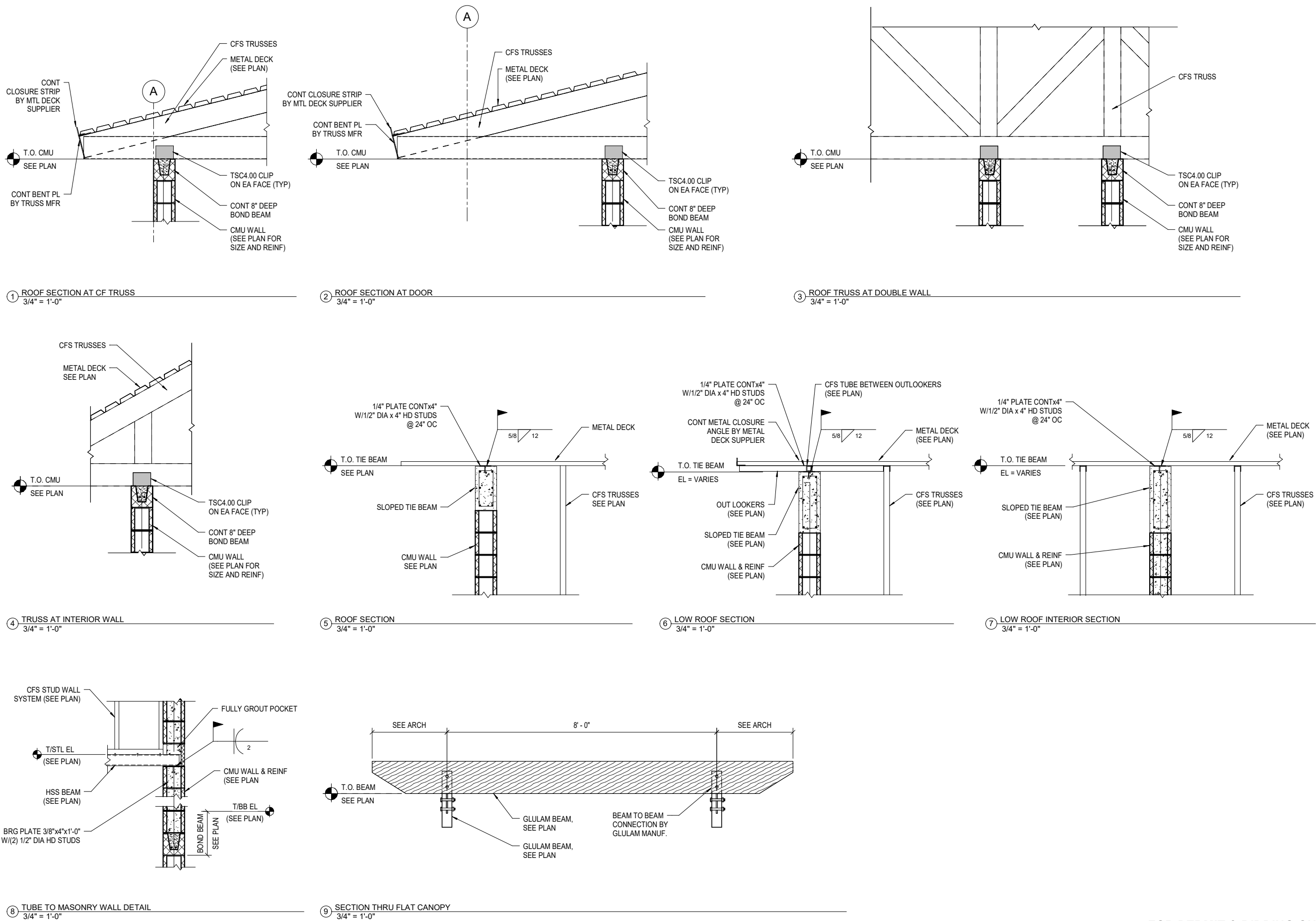
**AIRPORT TERMINAL AND HANGAR DEVELOPMENT**

PREPARED FOR  
DEFUNIAK SPRINGS MUNICIPAL AIRPORT

DESIGNED BY: **AJM**  
DRAWN BY: **AJM**  
CHECKED BY: **JY**  
APPROVED BY: **JY**  
PROJECT NO: **2019.028.01**  
DATE: **11/1/2021**

**SHEET NUMBER**  
**S4.02**

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TRANSFORMING TODAY'S IDEAS  
INTO TOMORROW'S REALITY

Michael Baker  
INTERNATIONAL

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SUITE 500  
TAMPA, FL 33607  
FLORIDA LICENSE #AA 26002484

JEFFREY YEAGER, P.E.  
FL LICENSE NO.: 62853

ROOF DETAILS

100% BID DRAWINGS

AIRPORT TERMINAL  
AND HANGAR  
DEVELOPMENT

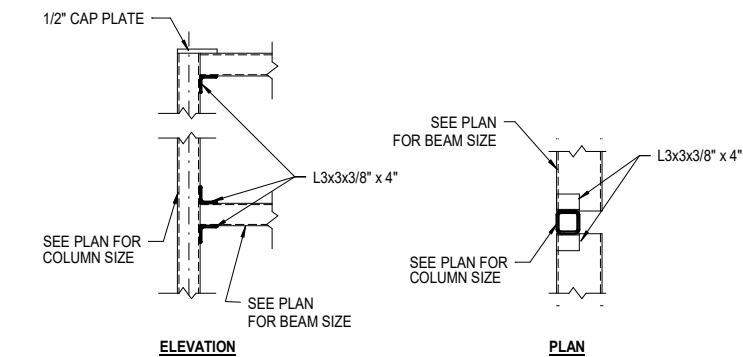
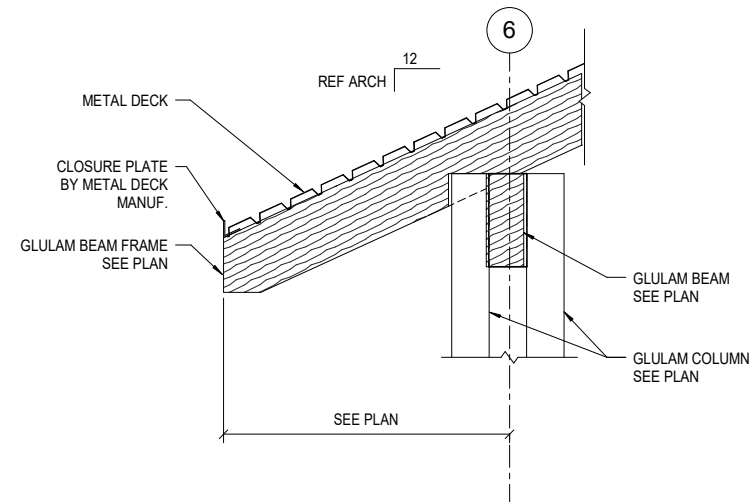
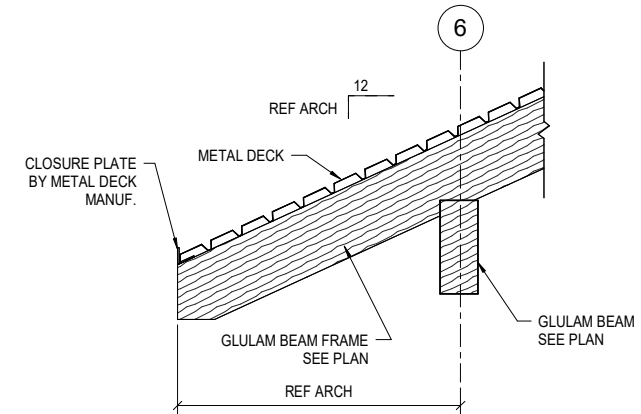
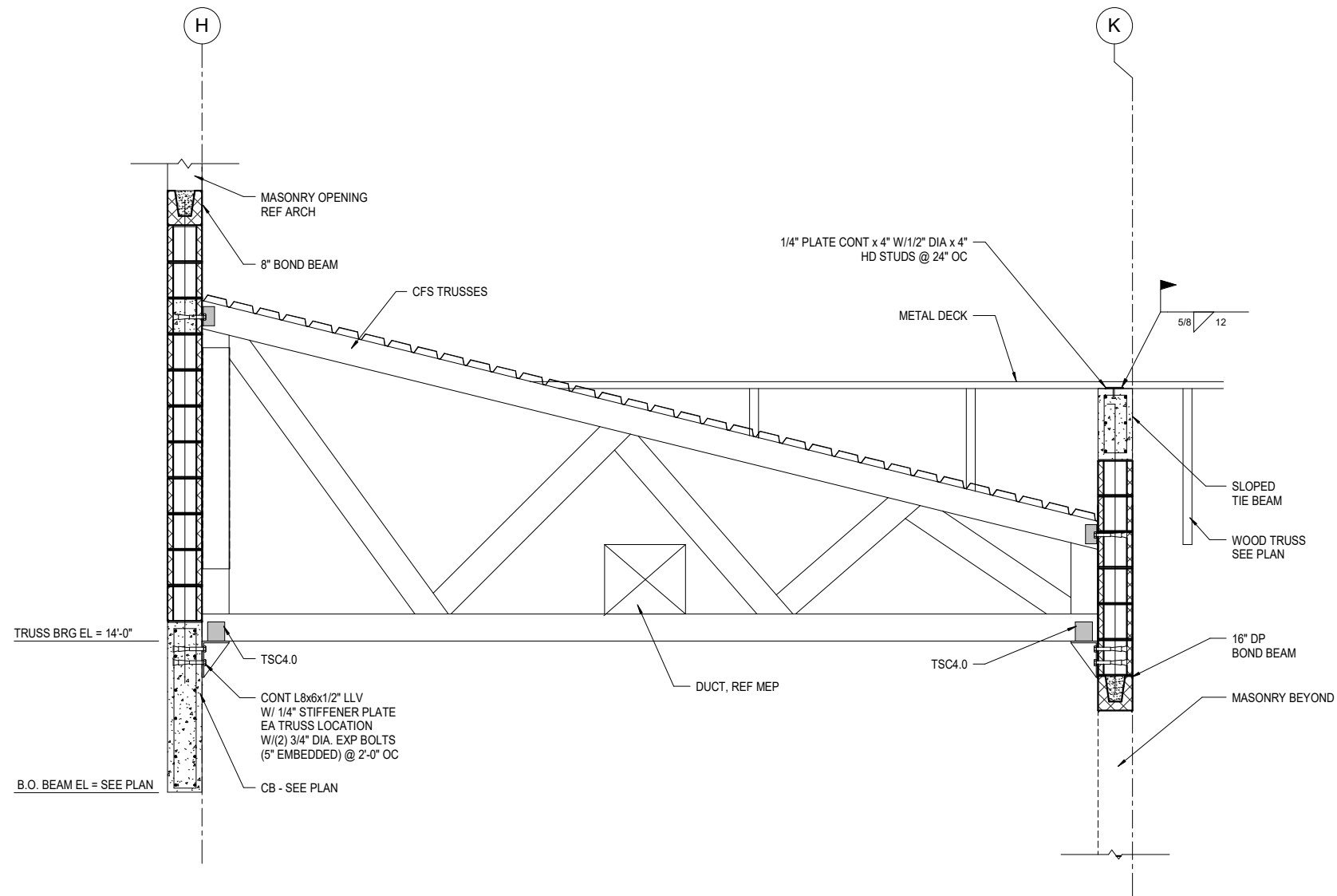
PREPARED FOR  
DEFUNIAK SPRINGS MUNICIPAL AIRPORT

DESIGNED BY: AJM  
DRAWN BY: AJM  
CHECKED BY: JY  
APPROVED BY: JY  
PROJECT NO: 2019.028.01  
DATE: 11/1/2021

SHEET NUMBER

S5.01

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ROOF DETAILS

100% BID DRAWINGS

AIRPORT TERMINAL  
AND HANGAR  
DEVELOPMENT

PREPARED FOR

DEFUNIAK SPRINGS MUNICIPAL AIRPORT

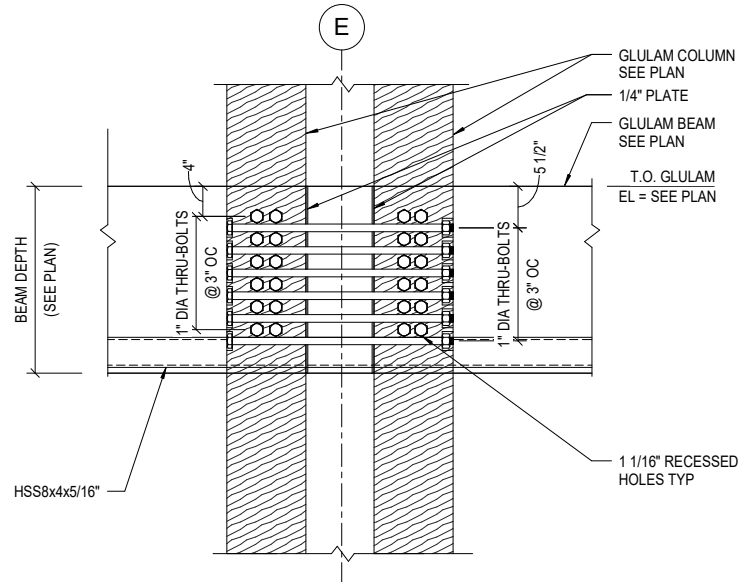
DESIGNED BY: **AJM**  
DRAWN BY: **AJM**  
CHECKED BY: **JY**  
APPROVED BY: **JY**  
PROJECT NO: 2019.028.01  
DATE: 11/1/2021

SHEET NUMBER

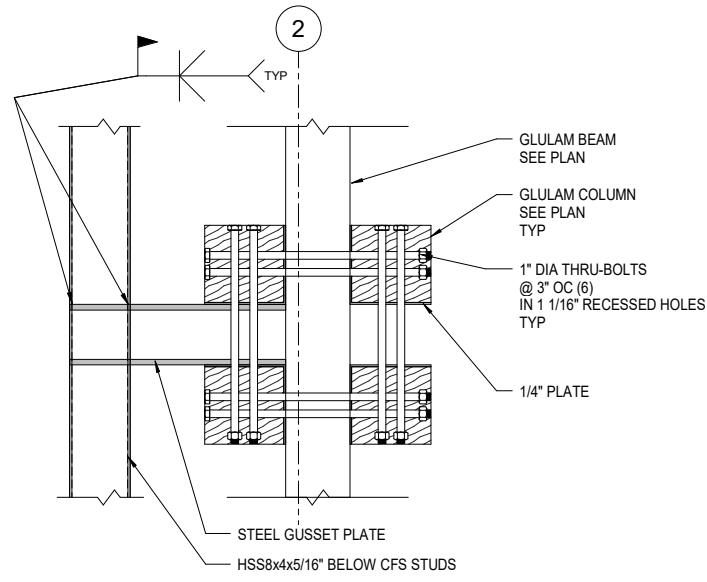
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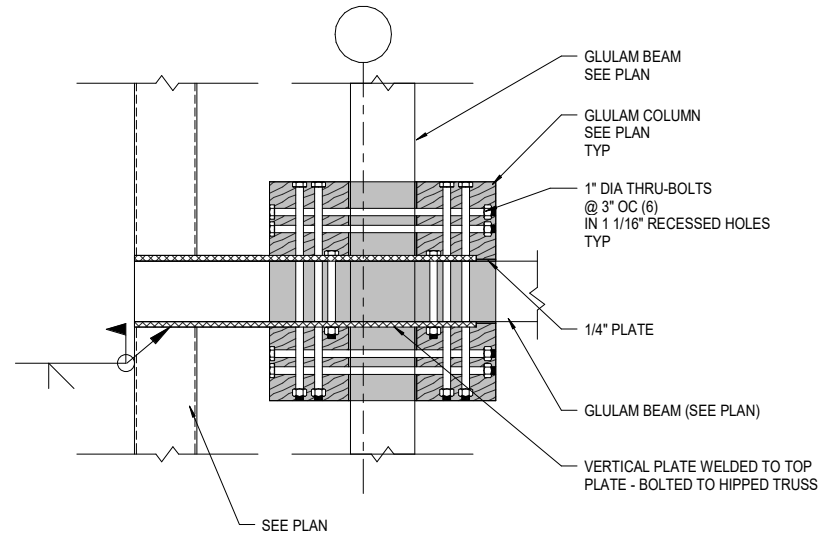




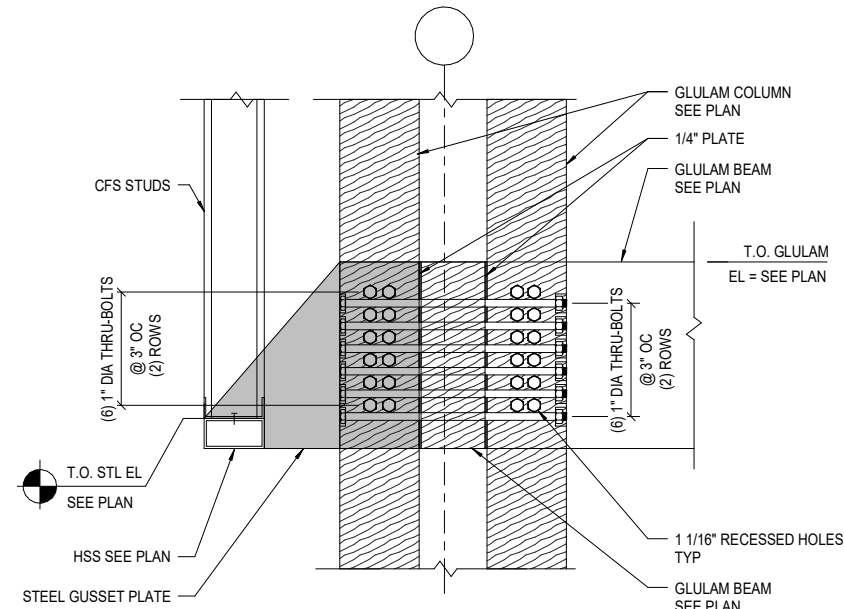
1 INTERMEDIATE BEAM (4 POST CONNX)  
1" = 1'-0"



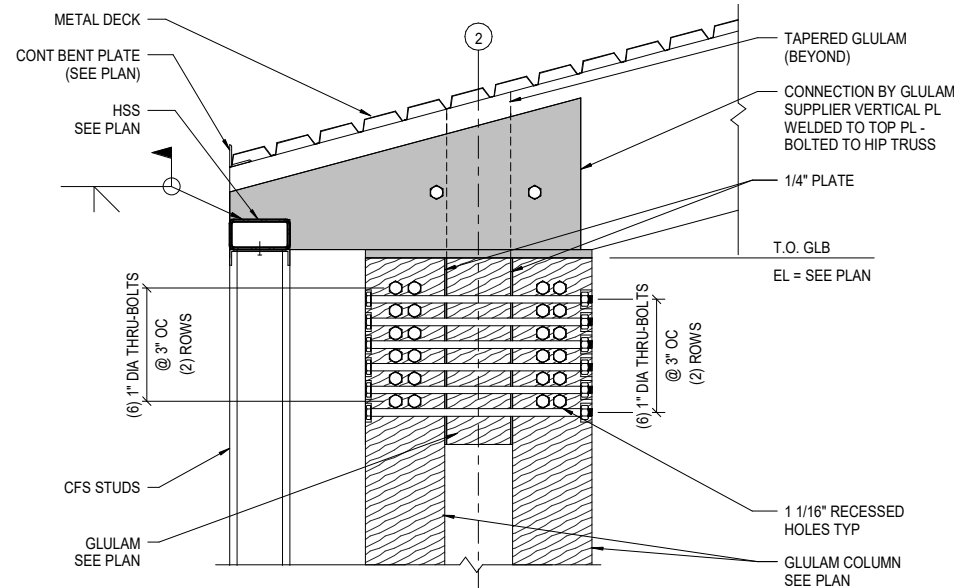
2 INTERMEDIATE BEAM PLAN VIEW (4 POST CONNX)  
1" = 1'-0"



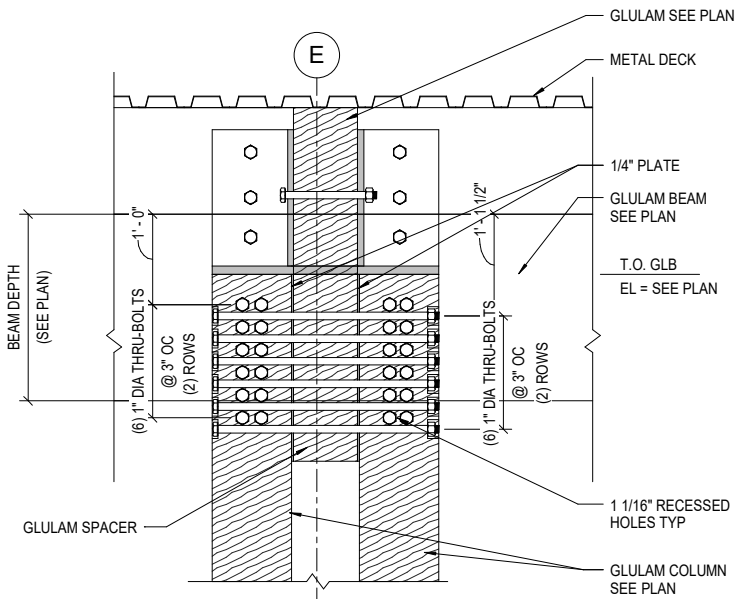
3 TOP BEAM PLAN (4 POST CONNX)  
1" = 1'-0"



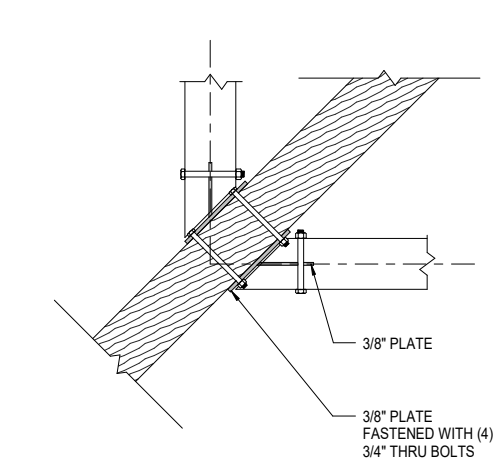
4 INTERMEDIATE BEAM (4 POST CONNX) AT CFS WALL  
1" = 1'-0"



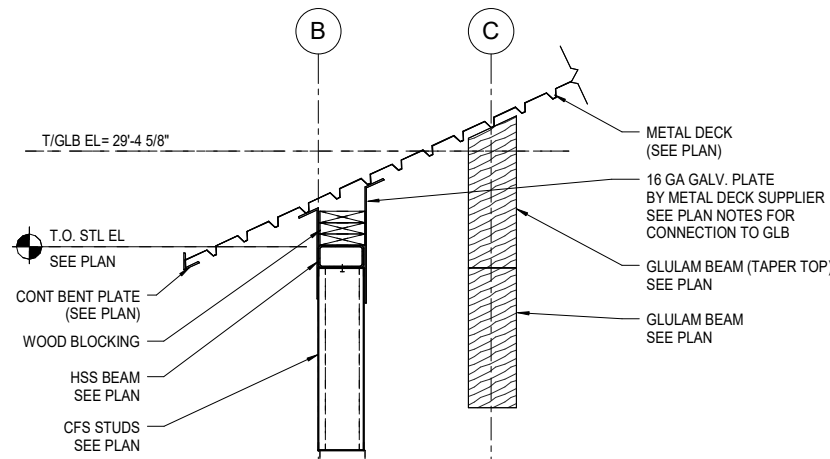
5 TOP BEAM (4 POST CONNX) AT CFS WALL  
1" = 1'-0"



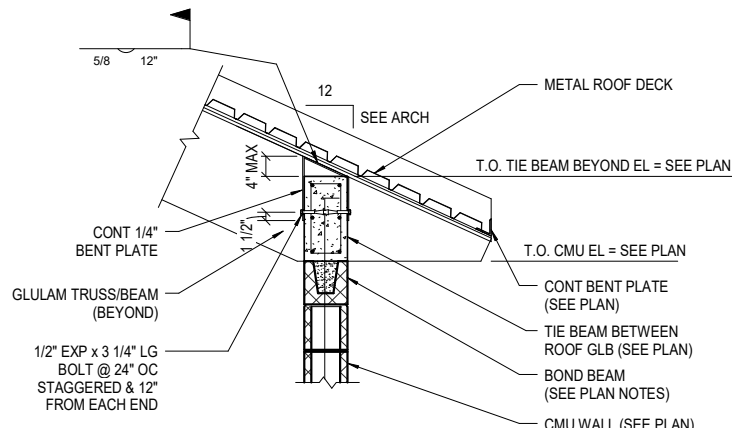
6 TOP BEAM (4 POST CONNX)  
1" = 1'-0"



7 INTERMEDIATE BEAM CONNECTION  
1" = 1'-0"



8 ROOF SECTION NORTH  
3/4" = 1'-0"



9 DETAIL THRU LOBBY - HIGH ROOF  
3/4" = 1'-0"

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ROOF DETAILS

100% BID DRAWINGS

AIRPORT TERMINAL  
AND HANGAR  
DEVELOPMENT

PREPARED FOR

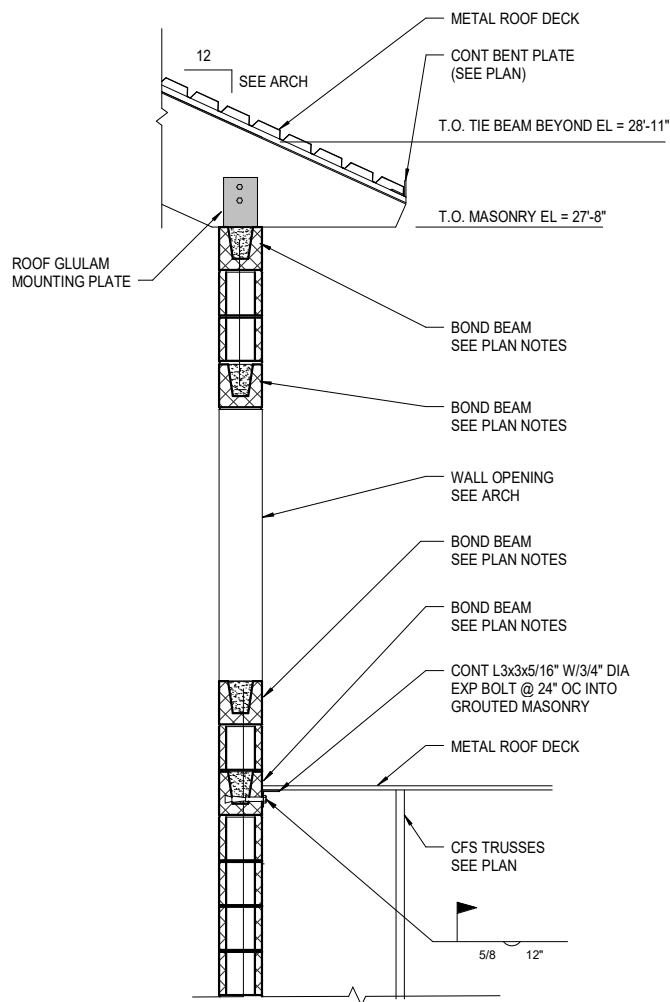
DEFUNIAK SPRINGS MUNICIPAL AIRPORT

DESIGNED BY: AJM  
DRAWN BY: AJM  
CHECKED BY: JY  
APPROVED BY: JY  
PROJECT NO: 2019.028.01  
DATE: 11/1/2021

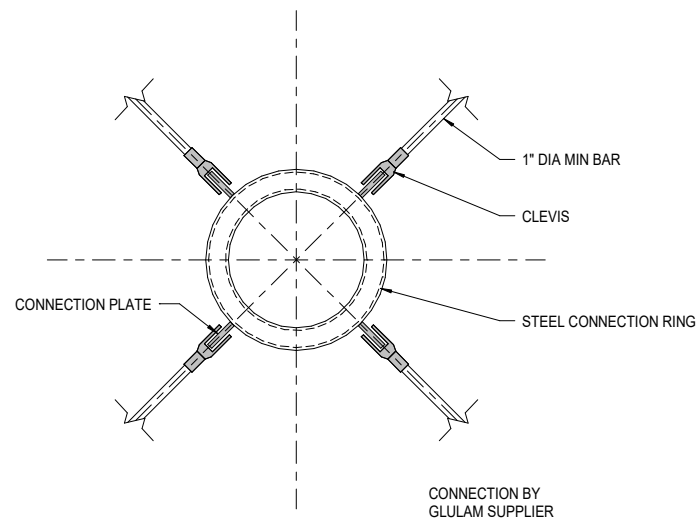
SHEET NUMBER

S5.03

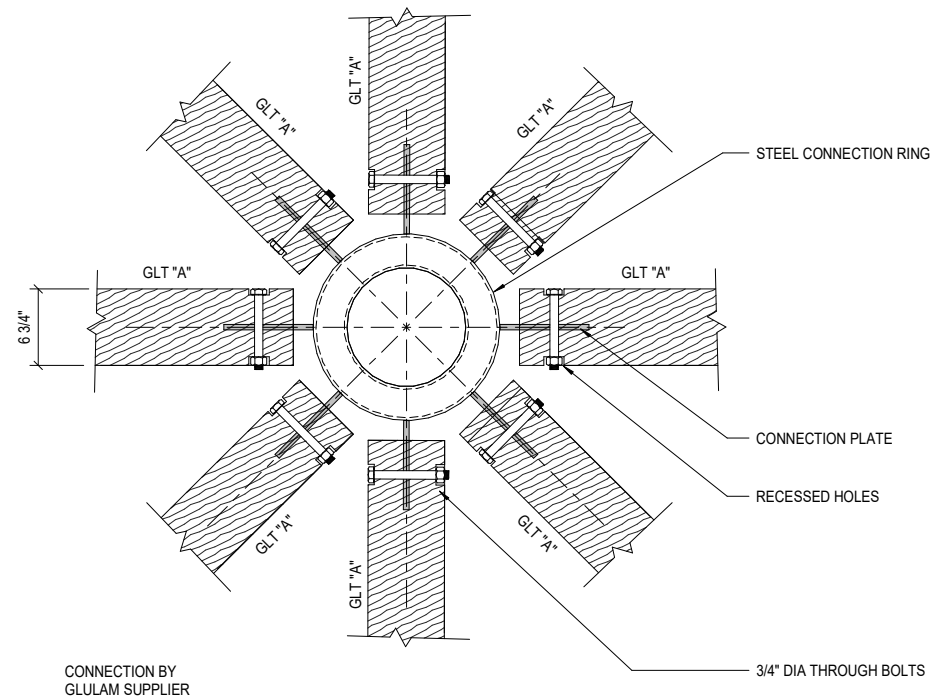
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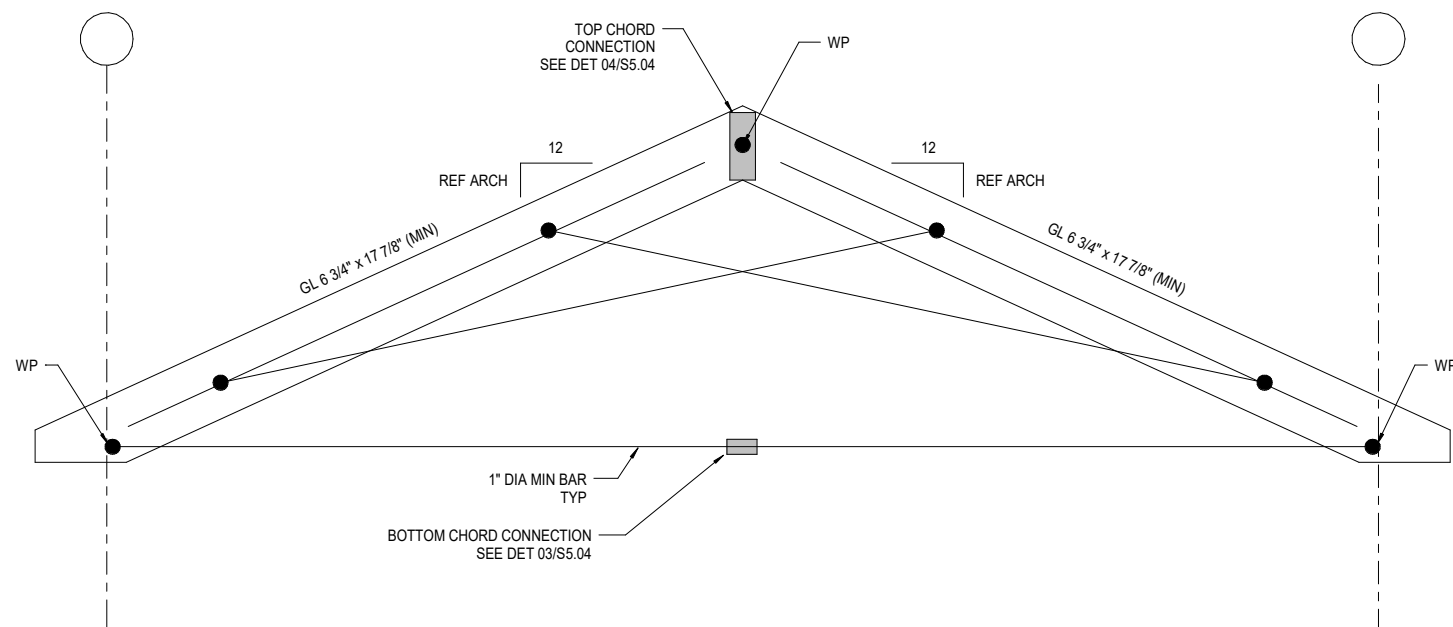
1 DETAIL THRU LOBBY  
3/4" = 1'-0"



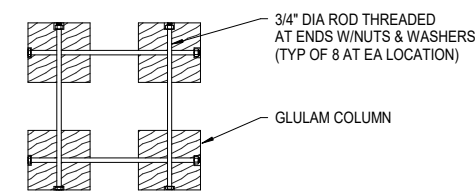
2 GLT "A" BOTTOM CHORD CONNECTION  
1 1/2" = 1'-0"



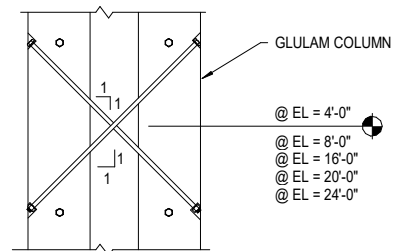
3 GLT "A" TOP CHORD AND GLB CONNECTION  
1 1/2" = 1'-0"



4 GLT "A" PROFILE  
1/2" = 1'-0"



SECTION



ELEVATION

5 COLUMN LACING DETAIL  
3/4" = 1'-0"

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ROOF DETAILS

100% BID DRAWINGS

AIRPORT TERMINAL  
AND HANGAR  
DEVELOPMENT

PREPARED FOR

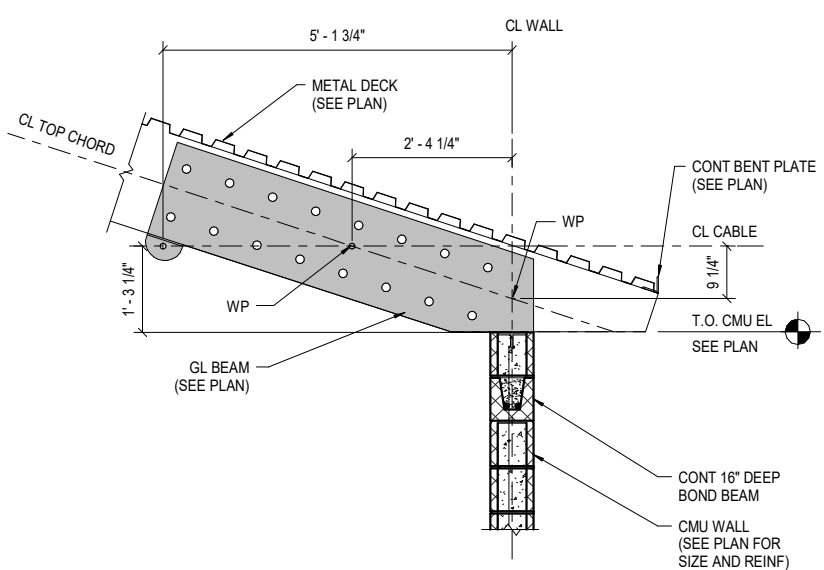
DEFUNIAK SPRINGS MUNICIPAL AIRPORT

DESIGNED BY: **AJM**  
DRAWN BY: **AJM**  
CHECKED BY: **JW**  
APPROVED BY: **JW**  
PROJECT NO: **2019.028.01**  
DATE: **11/1/2021**

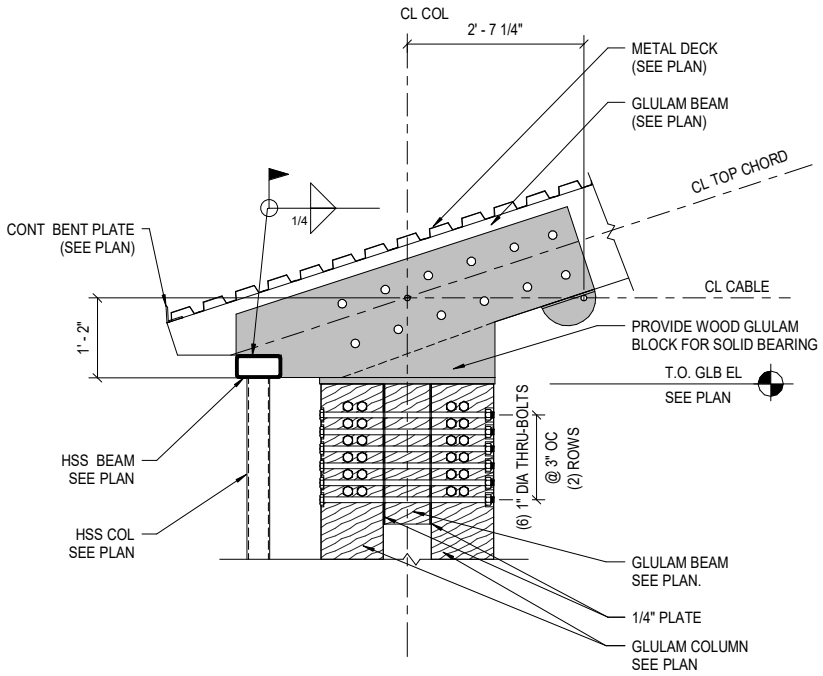
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① TRUSS CONNECTION DETAIL ON CMU  
3/4" = 1'-0"



② TRUSS CONNECTION DETAIL ON GLC  
3/4" = 1'-0"

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TRUSS DETAILS AT  
ROD CONNX

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DEVELOPMENT

PREPARED FOR

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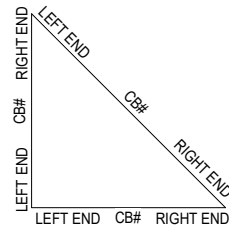
DESIGNED BY: **AJM**  
DRAWN BY: **AJM**  
CHECKED BY: **JY**  
APPROVED BY: **JY**  
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SHEET NUMBER

S5.05

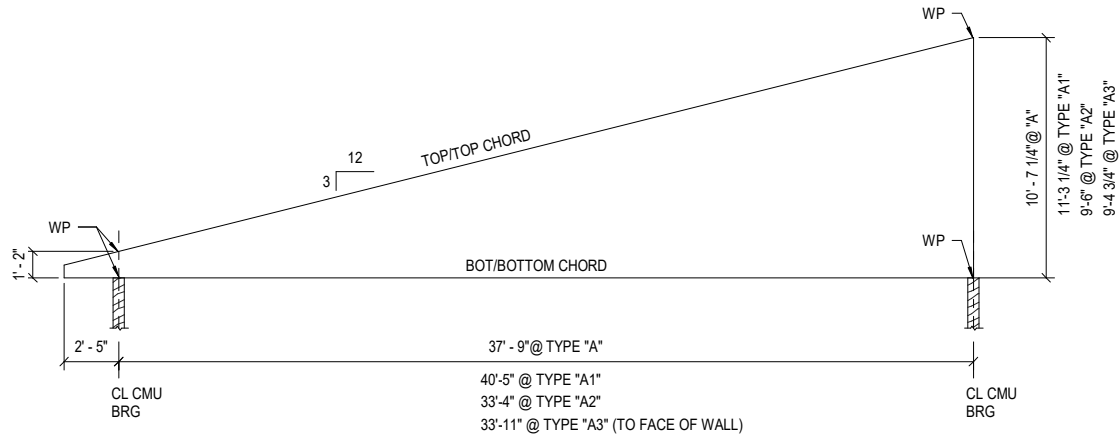
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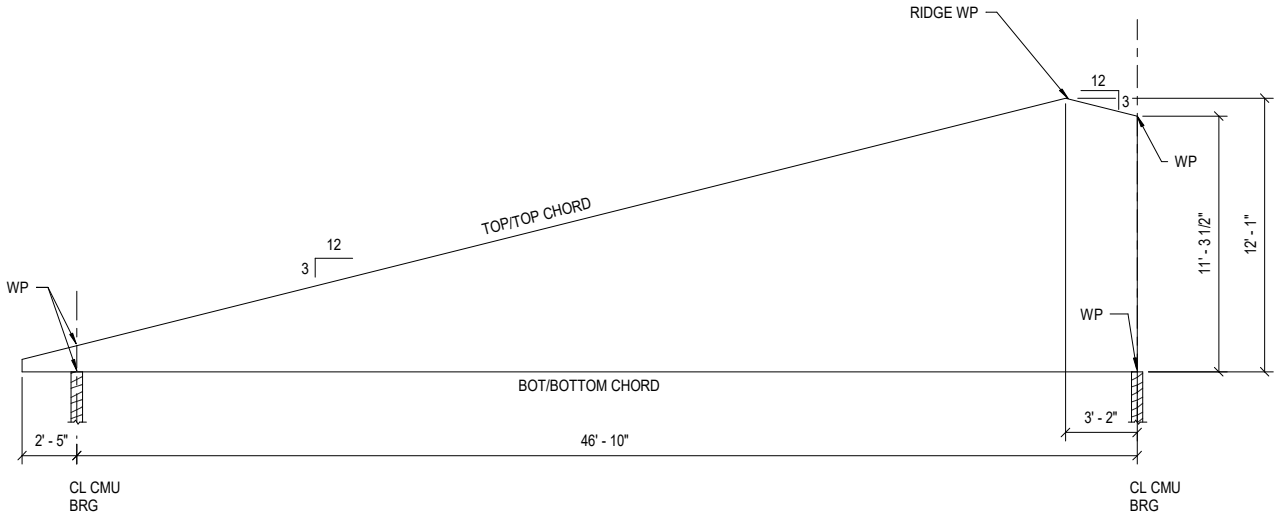


1. ALL "SIMPSON COMPANY" HANGERS, ETC. (OR APPROVED EQUAL) SHALL BE INSTALLED AS RECOMMENDED BY THE MANUFACTURER'S REQUIREMENTS.
2. EMBEDDED ANCHOR BOLTS SHALL BE STRAIGHT, THREADED RODS WITH A PLATED END AS SHOWN IN THE "TYPICAL ANCHOR BOLT DETAIL".

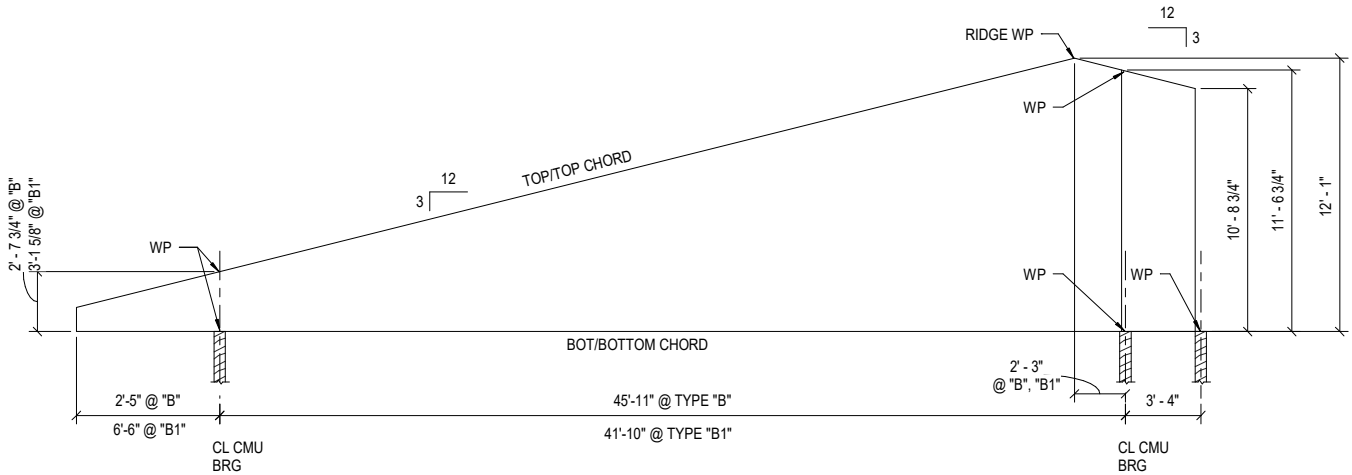
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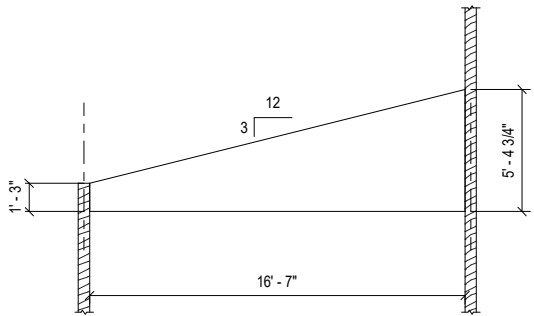
TRUSS PROFILES TYPE "A", "A1", "A2", "A3"



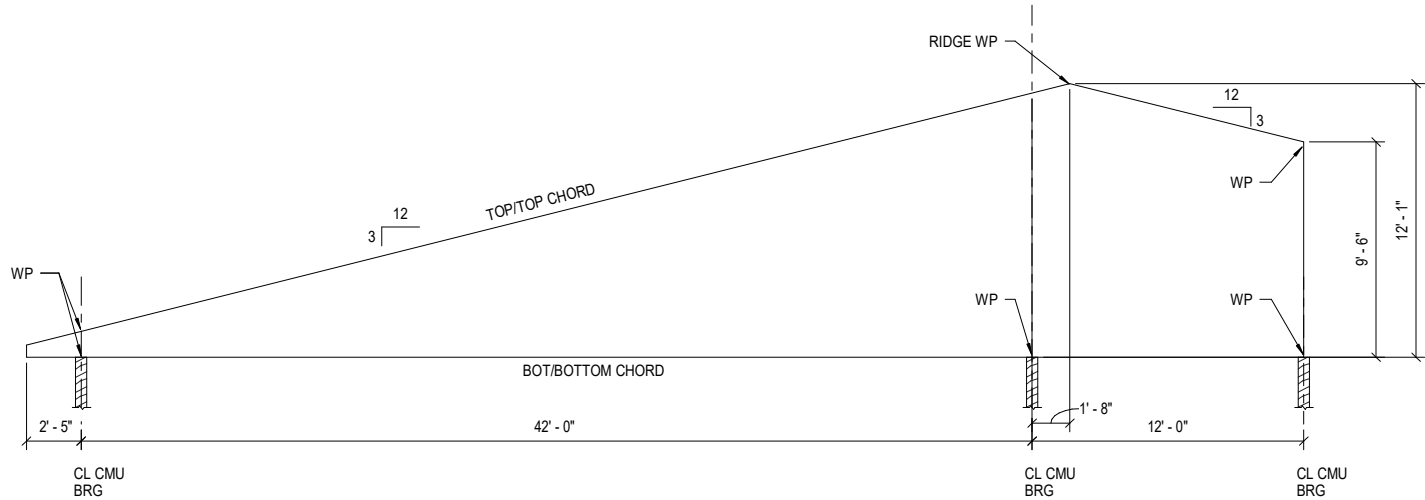
TRUSS PROFILE TYPE "C"



TRUSS PROFILES TYPE "B", "B1"



TRUSS PROFILE TYPE "D"



TRUSS PROFILE TYPE "B2"

1 TRUSS PROFILES  
1/4" = 1'-0"

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TRUSS PROFILES

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