ADDENDUM NUMBER 01

August 05, 2022

PROJECT: ADDITION TO DESHLER HIGH SCHOOL

AD1-1 GENERAL:

- A. The following revisions and/or additions to the Drawings and Project Manual are hereby made a part of same, and shall be incorporated in the Work of the Contract the same as if originally included in the Bid and Construction Documents.
- B. Bidders shall acknowledge receipt of this Addendum in writing, as provided on the Proposal Form.
- C. Clarification 1: <u>The pre-bid opening is August 9th, 2022</u> for the Addition to the Deshler High School. The pre-bid is mandatory to attend, and will start at 10:00a.m.
- D. Clarification 2: <u>The bid opening is August 30th, 2022</u> for the Addition to the Deshler High School. The bids are due by 2:00pm on August 30th, and will then be subsequently opened at the Tuscumbia Board of Education Office.
- E. Clarification 3: A bidder registration form must be submitted if you are planning on submitting a bid. The signed form can be sent to Walter King at (<u>walter.king@gmcnetwork.com</u>).
- F. Clarification 4: The project manual has been updated with specification section 05 3100. This section shall be replaced in its entirety.
- G. Clarification 5: The structural and architectural drawings have been updated to show support for the precast along the exterior elevation.
- H. Clarification 6: The structural and architectural drawings have been updated along the connection of the existing corridor and the new constructed corridor.
- I. Clarification 7: The reflected ceiling plans have been updated.

AD1-2 PROJECT MANUAL AND SPECIFICATIONS:

A. Section 05 3100 Steel Deck has been added to the project manual.

AD1-3 DRAWINGS:

- A. S1.01
- B. S1.02
- C. \$1.03
- D. S1.04
- E. S2.01
- F. S2.02
- G. S2.03 H. S3.01
- H. 53.01

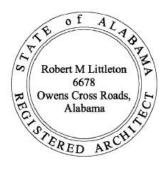
- I. S3.02
- J. \$3.03
- K. S4.01
- L. S4.02
- M. A0.01
- N. A1.01A
- O. A1.02
- P. A1.21
- Q. A2.01
- R. A2.02
- S. A3.01
- T. A3.02
- U. A4.01
- V. A4.02
- W. A5.11
- X. A5.12
- Y. A5.13
- Z. A5.31
- AA. A6.01
- BB. A6.11

AD1-4 ATTACHMENTS:

PREPARED BY

GMC

117 Jefferson Street North | Huntsville, Alabama 35801 Tel 256.539.3431 | GMCNETWORK.COM Goodwyn, Mills and Cawood, Inc.



1. GENERAL:

1.1. CODES AND SPECIFICATIONS: A. GENERAL BUILDING CODE:

INTERNATIONAL BUILDING CODE, 2015 EDITION.

- B. ASCE 7-10: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES. C. CONCRETE:
- BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14) D. STRUCTURAL STEEL: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AMERICAN INSTITUTE OF STEEL
- CONSTRUCTION AISC 360-10. ALLOWABLE STRESS DESIGN (ASD) E. STEEL JOISTS: STANDARD SPECIFICATIONS, LOAD TABLES AND WEIGHT TABLES FOR STEEL JOISTS AND JOIST GIRDERS, STEEL JOIST INSTITUTE SJI 100-10
- F. STEEL DECK: STEEL DECK INSTITUTE DESIGN STANDARDS FOR NON-COMPOSITE STEEL FLOOR DECK, STEEL ROOF DECK, COMPOSITE STEEL FLOOR DECK-SLABS, AND QUALITY CONTROL AND QUALITY ASSURANCE FOR INSTALLATION OF STEEL DECK

ANSI/SDI-11. G. MASONRY:

> 1. BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES TMS 402-13. 2. SPECIFICATIONS FOR MASONRY STRUCTURES.

H. COLD-FORMED METAL FRAMING: NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL

- STRUCTURAL MEMBERS, AMERICAN IRON AND STEEL INSTITUTE AISI \$100-12. 1.2. THE GENERAL NOTES ARE NOT A SUBSTITUTE OR A REPLACEMENT FOR THE PROJECT SPECIFICATIONS. THESE NOTES ARE INTENDED AS A GUIDE TO THE DESIGN AND/OR CONSTRUCTION REQUIREMENTS ESTABLISHED FOR THIS PROJECT. NO CONTRACTOR SHOULD ATTEMPT TO DESIGN, BID, OR CONSTRUCT ANY PORTION OF THE WORK HEREIN WITHOUT CONSULTING THE PROJECT SPECIFICATIONS. THE MORE STRINGENT REQUIREMENT SHALL APPLY WHERE CONFLICTS OCCUR BETWEEN THESE NOTES AND THE SPECIFICATIONS,
- UNLESS A WRITTEN CLARIFICATION IS ISSUED BY THE STRUCTURAL ENGINEER. 1.3. STRUCTURAL DRAWINGS ARE INTENDED TO BE USED IN CONJUNCTION WITH THE DRAWINGS OF OTHER CONSULTANTS AND TRADES. THE CONTRACTOR SHALL COORDINATE THE VARIOUS REQUIREMENTS.
- 1.4. DO NOT SCALE THESE DRAWINGS.
- 1.5. ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS, UNLESS NOTED 1.6. CONTRACTOR SHALL REVIEW AND VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR
- TO FABRICATION/CONSTRUCTION. THE STRUCTURAL ENGINEER AND ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION.
- 1.7. FIREPROOFING OF STRUCTURAL ELEMENTS IS NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO THE DRAWINGS OF OTHER CONSULTANTS FOR SUCH INFORMATION.
- 1.8. ALL ARCHITECTURAL AND UTILITY HALFTONE BACKGROUNDS ON PLANS, SECTIONS, AND ELEVATIONS ARE FOR REFERENCE ONLY AND ARE NOT UNDER THE PROFESSIONAL ENGINEERING SEAL RELATED TO THE STRUCTURAL ELEMENTS INDICATED IN THESE DRAWINGS. REFERENCE ARCHITECTURAL AND UTILITY DRAWINGS FOR NON-STRUCTURAL ELEMENTS INDICATED.
- 1.9. DESIGN LOADS:
- A. DEAD LOADS: ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE GENERAL CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE.
- B. LIVE LOADS: LIVE LOAD REDUCTIONS HAVE BEEN APPLIED TO THE STRUCTURAL MEMBERS IN ACCORDANCE WITH THE BUILDING CODE. PARTITION LIVE LOAD OF 15 PSF HAS BEEN INCLUDED AND IS INDICATED AS "+15" BELOW.

	TAS BELIN INCLUDED AND IS INDICATED AS INTO BELON	v.
	ROOF	20
	STAIRS, EXITWAYS	100
	CORRIDORS ABOVE 1ST FLOOR	80
C.	SNOW LOAD:	
	1. GROUND SNOW LOAD (Pg.)	10 PSF
D.	wind loads:	
	1. ULTIMATE DESIGN WIND SPEED (Vult)	115 MPH
	2. RISK CATEGORY	II
	3. WIND EXPOSURE CATEGORY	С
	4. INTERNAL PRESSURE COEFFICIENT	+/- 0.18
	5. DESIGN WIND PRESSURE FOR COMPONENTS	
	AND CLADDING	SEE TABLE
E.	seismic loads:	
	1. RISK CATEGORY	II
	2. SEISMIC IMPORTANCE FACTOR (Ie)	1.0
	3. MAPPED SPECTRAL RESPONSE ACCELERATION PARA	METERS:
	Ss	0.299
	S1	0.143
	4. SITE CLASS	С
	5. DESIGN SPECTRAL RESPONSE ACCELERATION PARAM	METERS:
	Sds	0.239

- 0.158 6. SEISMIC DESIGN CATEGORY ------ C 7. BASIC SEISMIC-FORCE RESISTING SYSTEM:
- BEARING WALL SYSTEM: INTERMEDIATE REINFORCED MASONRY SHEAR WALLS 8. DESIGN BASE SHEAR: ----- 27 kips
- 9. SEISMIC RESPONSE COEFFICIENT (Cs) ------ 0.07 10.RESPONSE MODIFICATION FACTOR (R) ------ 3.5
- 11.ANALYSIS PROCEDURE:
- EQUIVALENT LATERAL FORCE METHOD
- 2. SPECIAL INSPECTIONS: (BY TESTING AGENCY):
- 2.1. SPECIAL INSPECTOR (SI) SHALL BE RETAINED AND PAID BY THE OWNER.
- 2.2. THE SPECIAL INSPECTOR SHALL BE FULLY QUALIFIED, APPROVED BY THE BUILDING OFFICIAL REGISTERED BY APPLICABLE REGISTRATION BOARD IF REQUIRED AND ACCEPTABLE TO THE ARCHITECT 2.3. THE DUTIES OF THE SPECIAL INSPECTOR SHALL INCLUDE, BUT ARE NOT LIMITED TO,
- VERIFICATION OF CONSTRUCTION QUALITY CONTROL, TESTING, COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS, BUILDING CODE REQUIREMENTS, AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.
- 2.4. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE PROPER NOTIFICATION TO THE SPECIAL INSPECTOR AND PROCEED WITH THE CONSTRUCTION ONLY AFTER THE SPECIAL INSPECTOR'S REVIEW AND APPROVAL.
- 2.5. SPECIAL INSPECTOR'S SHALL KEEP RECORDS OF ALL INSPECTIONS AND TESTING. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE CODE OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CODE OFFICIAL AND THE DESIGN PROFESSIONAL OF RECORD. A FINAL REPORT OF INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTION AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY. INTERIM REPORTS SHALL BE SUBMITTED PERIODICALLY WITH MINIMUM FREQUENCY OF TWO WEEKS. 2.6. SPECIAL INSPECTIONS ARE REQUIRED FOR, BUT NOT LIMITED TO, THE ACTIVITIES AS
- INDICATED ON SHEET \$1.02 PER THE 2015 INTERNATIONAL BUILDING CODE. 2.7. FAILURE TO NOTIFY THE SPECIAL INSPECTOR MAY RESULT IN THE CONTRACTOR HAVING TO
- REMOVE WORK FOR THE PURPOSE OF INSPECTION AT THE CONTRACTOR'S EXPENSE. PREMATURE NOTIFICATION FOR INSPECTIONS WILL RESULT IN AN ADDITIONAL INSPECTION WITH THE EXPENSES AND FEES PAID BY THE CONTRACTOR.
- 3. CONSTRUCTION AND SAFETY: 3.1. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL SAFETY REGULATIONS, PROGRAMS, AND
 - PRECAUTIONS TO ALL WORK, PERSONS, AND PROPERTY ON AND/OR ADJACENT TO THE PROJECT AND SHALL PROTECT AGAINST ANY DAMAGE, INJURY, OR LOSS.

GENERAL NOTES

- 3.2. MEANS AND METHODS OF CONSTRUCTION AND ERECTION OF STRUCTURAL MATERIALS ARE SOLELY THE CONTRACTOR'S RESPONSIBILITY.
- 3.3. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE STRUCTURE. SUCH LOADS SHALL NOT EXCEED THE DESIGN LOAD OF THE STRUCTURE AT ANY TIME.
- 3.4. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION, AND ANY TEMPORARY BRACING OR SUPPORT REQUIRED TO ACCOMMODATE THE CONTRACTOR'S MEANS AND METHODS ARE THE RESPONSIBILITY OF THE CONTRACTOR.

4. SUBMITTALS:

- 4.1. ALL SHOP DRAWINGS MUST BE REVIEWED FOR "APPROVAL" AND STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL. 4.2. SUBMIT EACH SET OF SHOP DRAWINGS DIGITALLY. THE REVIEWED PDF FILE WILL BE
- RETURNED TO THE CONTRACTOR
- 4.3. THE GENERAL CONTRACTOR SHALL SUBMIT, FOR ENGINEER REVIEW, SHOP DRAWINGS FOR THE FOLLOWING ITEMS. ITEMS MARKED (*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED. ITEMS MARKED (#) SHALL BE SUBMITTED FOR ENGINEER 'S RECORD ONLY.
- A. STRUCTURAL STEEL (*)
- B. STEEL JOISTS (*) C. REINFORCING STEEL
- D. STEEL STAIRS (*)
- E. ARCHITECTURAL PRECAST (*)
- F. STEEL DECK (*)
- G. CONCRETE MIX DESIGNS
- H. SHOP FABRICATED COLD-FORMED METAL TRUSSES (*)
- 4.4. DESIGN CALCULATIONS: THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER'S REVIEW TWO SETS OF DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED, FOR THE FOLLOWING ITEMS: A. STRUCTURAL STEEL CONNECTIONS
- B. STEEL JOISTS
- C. STEEL STAIRS
- D. SHOP FABRICATED COLD-FORMED METAL TRUSSES
- E. ARCHITECTURAL PRECAST CONNECTIONS

5. FOUNDATION:

- 5.1. GEOTECHNICAL REPORT: FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL REPORT BY GOODWYN MILLS CAWOOD, TITLED "DESHLER HIGH SCHOOL ADDITION AND RENOVATION REPORT OF GEOTECHNICAL EXPLORATION", GMC PROJECT NO. GHUN220003". THE GENERAL CONTRACTOR SHALL OBTAIN A COPY OF THE GEOTECHNICAL REPORT FROM THE OWNER AND FOLLOW ALL REQUIREMENTS WITHIN THE RECOMMENDATIONS SECTION.
- 5.2. MAXIMUM BEARING PRESSURES (PSF): TYPICAL ---------- 2500
- 5.3. ALL FOUNDATION BEARING SURFACES SHALL BE REVIEWED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE THEIR COMPLIANCE WITH PRESSURES NOTED. ALL BOTTOM ELEVATIONS ARE ESTIMATED AND MAY BE ADJUSTED IN THE FIELD BY THE GEOTECHNICAL ENGINEER
- 5.4. ALL AREAS TO HAVE SLABS ON GRADE SHALL BE PROOF ROLLED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT UNDER OBSERVATION OF THE GEOTECHNICAL ENGINEER AND APPROVED PRIOR TO PREPARATION FOR CONCRETE PLACEMENT.
- 5.5. COMPACTED FILL WITHIN THE BUILDING AREA SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT.
- 5.6. BACKFILL FOR FOUNDATION AND RETAINING WALLS SHALL BE A FREE DRAINING GRANULAR MATERIAL, SUCH AS SIZE #57 STONE. BACKFILL SHALL BE COMPACTED SUFFICIENTLY TO PREVENT SUBSIDENCE OF SURFACE ADJACENT TO WALL. THE GRANULAR MATERIAL SHALL BE PLACED IN A 45 DEGREE WEDGE EXTENDING FROM THE BASE OF THE WALL (TOP OF FOOTING) TO WITHIN 1'-0" OF FINISHED GRADE.
- 6. FLOOR TOLERANCES
- 6.1. AFTER PLACING SLABS, FINISH SURFACE TO THE TOLERANCES OF F(F) (FLOOR FLATNESS) AND F(L) (FLOOR LEVELNESS) MEASURED ACCORDING TO ASTM E 1155 LISTED IN THE SPECIFICATIONS.
- 6.2. SPECIFIED OVERALL F-NUMBERS APPLY TO THE WHOLE FLOOR, TAKEN AS ONE. MINIMUM LOCAL F-NUMBERS APPLY TO EACH SLAB, BOUNDED BY CONSTRUCTION JOINTS.
- 7. CONCRETE:
- 7.1. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS:
 - 3000 PSI NORMAL WT. FOOTINGS, SLAB ON GRADE 4000 PSI NORMAL WT. - BEAMS AT TOWER, CONCRETE ON METAL DECK
- 7.2. CONTINUOUS WALL FOOTINGS: 2'-0" WIDE X 1'-0" THICK, REINFORCED WITH (3)#5 CONTINUOUS AND #3 TIES @ 24", UNLESS SHOWN OTHERWISE.
- 7.3. CONCRETE WALLS: REINFORCE 8" WALLS WITH #4 @ 10" EACH WAY IN CENTER OF WALL; REINFORCE 12" WALLS WITH #5 @ 12" EACH WAY EACH FACE, UNLESS NOTED.
- 7.4. PEDESTAL, COLUMN AND WALL VERTICAL REINFORCING: DOWEL TO FOUNDATION WITH
- HOOKED BARS OF SAME SIZE AND SPACING AS VERTICAL REINFORCING. 7.5. BEAM TOP BARS: EXTEND INTO SUPPORT IN ACCORDANCE WITH ACI STANDARD 318 (CODE). WHERE SUCH EXTENSION IS NOT OBTAINABLE, TERMINATE THE BAR IN A STANDARD
- RECOMMENDED 90 DEGREE HOOK. 7.6. EARTH SUPPORTED SLABS: 4" THICK, REINFORCED WITH 6X6 W2.9/W2.9 WWR (SHEETS) AT TOP THIRD OF SLAB DEPTH, UNLESS NOTED.
- 7.7. CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.
- 7.8. BEAMS (AT PERIMETER OF TOWER) SHALL HAVE NO HORIZONTAL OR VERTICAL JOINTS. 7.9. CAMBER HORIZONTAL CONCRETE MEMBERS SUCH THAT AFTER CONCRETE PLACEMENT AND PRIOR TO FORM AND SHORING REMOVAL, THE FINISHED CAMBER (IN INCHES) EQUALS THE FOLLOWING PERCENTAGES x THE SPAN (IN FEET) X 0.0125 FOR ALL SPANS.
 - LESS THAN 20 FEET ----- NO CAMBER
 - 20 FEET TO 30 FEET -----50%
- A. ADDITIONAL ALLOWANCES FOR FORM AND SHORING SYSTEM DEFLECTION, RESULTING FROM CONCRETE PLACEMENT, IS THE RESPONSIBILITY OF THE CONTRACTOR.

8. CONCRETE REINFORCING STEEL:

- 8.1. REINFORCING BARS: ASTM A615 GRADE 60.
- 8.2. WELDED WIRE REINFORCEMENT (WWR): ASTM A1064. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2 INCHES OR 6 INCHES.
- 8.3. REINFORCING STEEL SHOWN IN SECTIONS IS A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES AND GENERAL NOTES FOR ACTUAL REINFORCING
- REQUIRED 8.4. REINFORCING BAR PLACING ACCESSORIES IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE. WHERE CONCRETE IS EXPOSED IN FINISHED BUILDING, PROVIDE ACCESSORIES WITH RUSTPROOF LEGS. WHERE CONCRETE IS SAND-BLASTED OR BUSH-HAMMERED, PROVIDE ACCESSORIES OF STAINLESS STEEL
- 8.5. DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315. REINFORCEMENT SHALL NOT BE WELDED UNLESS NOTED OR APPROVED BY THE ENGINEER. 8.6. ALL SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.
- 8.7. ALL REINFORCING MARKED "CONTINUOUS" SHALL BE SPLICED WITH CLASS "B" TENSION LAP
- SPLICE, UNLESS NOTED. 8.8. PROVIDE CORNER BARS WITH CLASS "B" LAP SPLICE AT ALL FOUNDATION INTERSECTIONS AND CORNERS.
- 8.9. CONCRETE COVERAGE OF REINFORCEMENT (UNO ON DRAWINGS): FOOTINGS------3" BOTTOM & SIDES, 2" TOP
 - BEAMS-----1-1/2" CLEAR OF STIRRUPS
- SLABS NOT EXPOSED TO WEATHER------3/4" BOTTOM & TOP
- 8.10. FIELD BENDING OF CONCRETE REINFORCING STEEL IS NOT PERMITTED WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.

9. ARCHITECTURAL PRECAST CONCRETE:

- 9.1. REFER TO ARCHITECT'S DRAWINGS AND SPECIFICATIONS FOR DIMENSIONAL, FINISH, AND OTHER REQUIREMENTS OF ARCHITECTURAL PRECAST. 9.2. PRECAST MANUFACTURER IS TO BE RESPONSIBLE FOR THE DESIGN OF ALL PRECAST
- MEMBERS AND THEIR CONNECTIONS TO THE STRUCTURE. CALCULATIONS AND SHOP DRAWINGS SHALL BE SUBMITTED BEARING THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.

- 9.3. ANY CONNECTIONS SHOWN ON CONTRACT DRAWINGS ARE SHOWN FOR GENERAL ARRANGEMENT ONLY. THE CONTRACTOR SHALL COORDINATE ALL PRECAST CONNECTIONS AND EMBEDDED ITEMS WITH THE PRECAST MANUFACTURER.
- 9.4. ERECTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL TEMPORARY BRACING UNTIL ALL CONNECTIONS HAVE BEEN MADE AND TOPPING HAS BEEN CAST.
- 9.5. PRECAST MANUFACTURER SHALL PROVIDE STABILIZING ANGLES, AS REQUIRED, IN ALL PRECAST WORK.
- 9.6. ALL EXPOSED STEEL CONNECTIONS AND SUPPORT ANGLES, PLATES, BARS AND BOLTS IN CONJUNCTION WITH ALL PRECAST CONCRETE SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION AND FIELD TOUCHED UP WITH ZINC RICH PAINT. 9.7. ADJUSTMENT AND POSSIBLY RESETTING OF PRECAST MAY BE REQUIRED TO ALIGN PRECAST
- DUE TO SUPPORT DEFLECTION AND/OR ROTATION. 9.8. SUPPORTING BEAMS AND STRUCTURE WILL DEFLECT AND/OR ROTATE. PRECAST MANUFACTURER AND ERECTOR SHALL COORDINATE CONNECTION/ERECTION SEQUENCE TO ACCOUNT FOR THIS MOVEMENT AND MAKE FINAL ADJUSTMENTS TO ALIGN AND PLUMB
- PRECAST. THIS MAY REQUIRE ADJUSTING CONNECTIONS OR RECONNECTING. **10. CONCRETE FORMWORK:**
- ACCORDING TO STANDARD 347 RECOMMENDED PRACTICE FOR CONCRETE FORMWORK. 10.2. RESPONSIBILITY:
- A. THE DESIGN, CONSTRUCTION, AND SAFETY OF ALL FORMWORK SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. ALL FORMS, SHORES, BACK SHORES, FA LSE WORK, BRACING, AND OTHER TEMPORARY SUPPORTS SHALL BE ENGINEERED TO
- CONSTRUCTION EQUIPMENT, LIVE LOADS, LATERAL LOADS DUE TO WIND AND WET CONCRETE IMBALANCE. 10.3. DESIGN:
- A. THE DESIGN OF ALL CONCRETE FORMWORK SHALL BE PERFORMED BY A REGISTERED PROFESSIONAL STRUCTURAL ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED.
- 10.4. TOLERANCES:
- A. UNLESS SPECIFIED OTHERWISE, ALL TOLERANCES FOR CONCRETE FORMWORK SHALL CONFORM TO STANDARD 117, STANDARD TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS. THE CONTRACTOR SHALL ENGAGE A LICENSED SURVEYOR TO VERIFY THAT WORK IS WITHIN SPECIFIED TOLERANCES.
- 10.5. ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER, INCLUDING COLUMNS, BEAMS, SLABS, AND WALLS.
- 11. STRUCTURAL STEEL:
- 11.2. WELDED CONNECTIONS: E70XX ELECTRODES, MINIMUM SIZE FILLET WELD 3/16".
- 11.3. HEADED STUDS: ASTM A108, GRADE 1015 OR 1020, COLD-FINISHED CARBON STEEL WITH DIMENSIONS COMPLYING WITH AISC.
- 11.4. BOLTED CONNECTIONS: ASTM F3125 GRADE A325-N IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS." BOLTS THROUGH 4" WIDE BEAM FLANGES SHALL BE 5/8" DIAMETER. OTHER BOLTS SHALL BE 3/4" DIAMETER. 11.5. BOLTS SHOWN IN SECTIONS AND DETAILS ARE A SCHEMATIC INDICATION THAT BOLTS MAY
- BE USED. ACTUAL NUMBER, UNLESS SPECIFIED, TO BE IN ACCORDANCE WITH AISC. 11.6. FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES. 11.7. THE STEEL FRAME IS "NON-SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE
- PROVIDED BY THE CONTRACTOR UNTIL REQUIRED CONNECTIONS OR ELEMENTS ARE IN
- PLACE.
- 12. STEEL JOIST: 12.1. DESIGN, FABRICATE, AND ERECT STEEL JOISTS IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE
- 12.2. ALL JOISTS TO HAVE 2 1/2" DEEP BEARING SEAT AT EACH END, TYPICAL. 12.3. PROVIDE A MINIMUM END BEARING ON STEEL SUPPORTS AS REQUIRED BY THE STEEL JOIST INSTITUTE. STAGGER THE ENDS OF JOIST IF NECESSARY AND GENERAL CONTRACTOR COORDINATE METAL DECK SPLICE LOCATION TO CENTER OVER JOIST. 12.4. PROVIDE HORIZONTAL AND DIAGONAL BRIDGING IN ACCORDANCE WITH SJI TO PROVIDE
- ADEQUATE JOIST CHORD BRACING.
- 12.5. AT JOIST PARALLEL TO MASONRY WALL, WELD EACH BRIDGING ROW TOP AND BOTTOM TO AN ANGLE L3X3X3/16X0'-6". ANCHOR ANGLE WITH TWO 3/8" DIAMETER SLEEVE ANCHORS WITH A TWO-INCH EMBEDMENT INTO WALL. 12.6. AT JOISTS PARALLEL TO BEAMS, ANCHOR BRIDGING ROWS BY WELDING TO BEAMS.
- 12.7. DESIGN ROOF JOISTS TO RESIST THE FOLLOWING WIND NET UPLIFT PRESSURES: A. EDGE ZONES (REGIONS WITHIN Z DISTANCE OF ROOF EDGE) - 35 PSF. B. CORNER ZONES (REGIONS WITHIN Z DISTANCE OF TWO INTERSECTING ROOF EDGES) -50 PSF. C. INTERIOR ZONES (REGIONS THAT ARE NOT EDGE OR CORNER ZONES) — 20 PSF.
- D. DISTANCE Z 12 FT.
- 13. STEEL DECK:
- 13.1. DECK PROPERTIES AND ATTACHMENTS SHALL BE IN ACCORDANCE WITH THE STEEL DECK
- INSTITUTE. 13.2. DECK SHALL BE CONTINUOUS OVER THREE OR MORE SPANS.
- GALVANIZED
- 13.3. ROOF DECK: WIDE RIB TYPE "WR", STEEL ROOF DECK, 22 GA GAGE, 1-1/2" DEEP, A. DECK SHALL BE WELDED TO SUPPORTS WITH A 5/8" DIAMETER PUDDLE WELD ON A 36/4 PATTERN WITH (4) #10 TEK SCREW SIDELAP FASTENERS.
- 13.4. FORM DECK: 2 1/2" THICK CONCRETE SLAB ON 24 GAGE, 9/16" NON-COMPOSITE STEEL FORM DECK (3" TOTAL THICKNESS) WITH 6X6 W2.1/W2.1 WWR AT MID-DEPTH. A. DECK SHALL BE WELDED TO SUPPORTS WITH A 5/8" DIAMETER PUDDLE WELD ON A 30/4 PATTERN WITH (2) #10 TEK SCREW SIDELAP FASTENERS. 13.5. LIGHTGAGE METAL FRAMING, SUSPENDED CEILINGS, LIGHT FIXTURES AND DUCTS OR OTHER
- UTILITIES SHALL NOT BE SUPPORTED BY THE METAL ROOF DECK.
- 13.6. DO NOT SHORE FORM DECK DURING CONCRETE PLACEMENT.

14. MASONRY:

14.1. MASONRY CONSTRUCTION SHALL CONFORM TO TMS 602 SPECIFICATION. 14.2. COMPRESSIVE STRENGTH OF MASONRY (f'm) SHALL BE 2000 PSI AT 28 DAYS. MASONRY UNIT

C. ALL MASONRY CELLS OR CAVITIES BELOW GRADE SHALL BE GROUTED SOLID UNLESS

D. VERTICAL GROUTING SHALL BE LOW LIFT OR HIGH LIFT AS FOLLOWS:

PROVIDED AT THE BASE OF EACH GROUTED CELL.

14.6. ALL MASONRY SHALL BE RUNNING BOND, UNLESS NOTED.

SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS. MORTAR FILL IS NOT PERMITTED.

1. LOW LIFT GROUTING SHALL BE USED FOR ALL CAVITY WALLS AND MAY BE USED FOR

2. HIGH LIFT GROUTING IS PERMISSIBLE ONLY FOR FILLING OF CELLULAR MASONRY

14.5. MORTAR SHALL CONFORM TO ASTM C 270. MORTAR SHALL BE TYPE "M" FOR BELOW GRADE

14.7. ALL BLOCK CELLS AND CAVITIES BELOW GRADE SHALL BE FILLED WITH CONCRETE OR

APPLICATIONS AND TYPE "S" FOR ABOVE GRADE APPLICATIONS AND SHALL HAVE A 28-DAY

ALL WALLS AT THE OPTION OF THE CONTRACTOR. LIFTS SHALL NOT EXCEED 4'0' IN

UNITS AND SHALL NOT EXCEED ONE STORY IN HEIGHT. CLEAN OUT HOLES SHALL BE

- STRENGTH OF 2000 PSI IS REQUIRED TO ACHIEVE REQUIRED fm.
- 14.3. MASONRY GROUT FILL SHALL CONFORM TO ASTM C 476. GROUT EITHER FINE (SAND) OR COURSE (SAND + #76 STONE) AGGREGATE SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI. MASONRY CONCRETE FILL SHALL CONFORM TO THE REQUIREMENTS NOTED UNDER "CONCRETE" IN THE GENERAL NOTES. 14.4. GROUTING
- A. ALL BOND BEAMS SHALL BE FILLED WITH GROUT AND REINFORCED AS INDICATED ON

HFIGHT

GROUT.

COMPRESSIVE STRENGTH 1900 PSI.

THE DRAWINGS (DETAILS OR SCHEDULES). MORTAR FILL IS NOT PERMITTED. B. ALL MASONRY WALL CELLS OR CAVITIES INDICATED AS REINFORCED SHALL BE GROUTED FOR THE FULL HEIGHT OF THE WALL, UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS. UN-REINFORCED WALLS INDICATED AS GROUTED SHALL BE GROUTED FULL HEIGHT, UNLESS SPECIFICALLY NOTED OTHERWISE. MORTAR FILL IS NOT

- 11.1. STRUCTURAL STEEL: ASTM A992 FOR WIDE FLANGE SHAPES; ASTM A36 ELSEWHERE.

- SUPPORT ALL LOADS IMPOSED INCLUDING THE WET WEIGHT OF CONCRETE,
- 10.1. ALL FORMWORK SHALL BE DESIGNED, ERECTED, SUPPORTED, BRACED, AND MAINTAINED

- 14.9. REINFORCING:

PRODUCTS FROM OTHER SUPPLIERS.

CONTROL JOINTS.

CONCRETE MASONRY UNITS.

JOINTS:

AND WALL ENDS.

15. POST-INSTALLED ANCHOR:

- SCHEDULE' IN TYPICAL DETAILS.
- OR VERTICAL CMU CORES. GREATER SLOPES WILL REQUIRE REPLACEMENT OF THE
- FOUNDATION DOWELS.
- C. SPLICED REINFORCING SHALL BE LAPPED UNDER "REINFORCING" ABOVE OR AS SHOWN

MASONRY AND SHALL BE HELD IN POSITION TOP AND BOTTOM AND AT INTERVALS NOT

EXCEEDING 4"-0". ACCESSORIES FOR SUCH SUPPORT SHALL BE USED, PROVIDE "AA WIRE

PRODUCTS COMPANY" (OR APPROVED EQUAL) REBAR POSITIONER AA225 OR AA239

FOR VERTICAL BARS AND AA28 FOR HORIZONTAL BARS OR APPROVED EQUAL

- B. FOUNDATION DOWELS MAY SLOPE A MAXIMUM OF 1:6 TO ALIGN WITH WALL CAVITIES

- A. ALL BARS MARKED "CONTINUOUS" SHALL BE LAPPED PER "MASONRY WALL LAP SPLICE

ON DRAWINGS, WHICHEVER IS GREATEST. ALL SPLICES SHALL BE WIRED TOGETHER.

D. VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 3/4" FROM

E. HORIZONTAL JOINT REINFORCING SHALL BE LAPPED NO LESS THAN 6" AT ALL SPLICES,

G. HORIZONTAL REINFORCING IN BOND BEAMS SHALL BE CONTINUOUS THROUGH

A. PROVIDE MAXIMUM SPACING OF 25'-0" O.C. ALONG CONTINUOUS RUNS OF

14.10. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND DETAILS OF MASONRY CONTROL

14.11. WHEN REINFORCING IS SPECIFIED, PROVIDE AT EACH SIDE OF CONTROL JOINTS, OPENINGS

14.12. ALL MASONRY WALLS SHOWN ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS HAVE

15.2. ACCEPTABLE MANUFACTURERS SHALL INCLUDE BUT ARE NOT LIMITED TO HILTI, INC. AND

SIMPSON STRONG-TIE COMPANY, INC. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER

THAN THOSE SHOWN IN THE DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR ALONG

WITH PREPARED DOCUMENTATION DEMONSTRATING EQUAL SUBSTITUTION THAT THE

PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES OF THE

SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S)

EXISTING MILD STEEL REINFORCING. DO NOT CUT EXISTING REINFORCING DURING THE INS

TALLATION OF POST-INSTALLED ANCHORS. THE CONTRACTOR SHALL CREATE A TEMPLATE AT

EACH POST-INSTALLED ANCHOR CONNECTION LOCATION PRIOR TO FABRICATING HOLES

IN CONNECTION PLATES. TEMPLATES SHALL BE MADE BY LOCATING EXISTING MILD STEEL

REINFORCING WITH TESTING EQUIPMENT AND LOCATING POST-INSTALLED ANCHOR HOLES

SUCH THAT NO CONFLICTS EXIST WITH THE EXISTING MILD STEEL REINFORCING.

POST-INSTALLED ANCHOR LOCATIONS IN THE FIFLD MAY BE RELOCATED A MAXIMUM OF 1

1/2" FROM THE DIMENSIONS SHOWN ON THE DRAWINGS TO AVOID CONFLICTS WITH THE

EXISTING MILD STEEL REINFORCING. DO NOT MOVE ANY POST-INSTALLED ANCHOR CLOSER

WRITTEN INSTRUCTIONS. ALL ABANDONED HOLES DRILLED IN CONCRETE OR MASONRY

CONCRETE BEING DRILLED. THE MAXIMUM TOLERANCE FOR DEVIATION FROM

PERPENDICULAR SHALL BE 10 DEGREES. ALL POST-INSTALLED ANCHORS INSTALLED OUTSIDE

ERECTION PURPOSES, THE CONTRACTOR MUST PROVIDE PLATE WASHERS. PLATE WASHERS

FOR THE FIRST INSTALLATION OF EACH TYPE OF ANCHOR USED TO DEMONSTRATE AND

INSTRUCT TO THE CONTRACTOR'S INSTALLER(S) THE PROPER METHOD OF INSTALLATION.

SHOULD THE CONTRACTOR CHANGE INSTALLER(S) DURING THE INSTALLATION OF THE

ANCHOR(S), THE MANUFACTURER'S REPRESENTATIVE SHALL BE NOTIFIED BY THE

INSPECTOR. OBSERVATIONS SHALL BE MADE TO CONFIRM THE FOLLOWING FOR EACH

A. EXISTING REINFORCING WAS LOCATED AROUND THE PROPOSED ANCHOR HOLES AND

C. HOLE DRILLING TECHNIQUES, SIZE AND DEPTH ARE AS RECOMMENDED BY THE

D. REINFORCING IN THE EXISTING CONCRETE WAS NOT DAMAGED BY DRILLING

F. FINAL TORQUING OF THE POST-INSTALLED ANCHORS IS PER THE MANUFACTURER'S

16.1. STRUCTURAL PROPERTIES OF TRUSS MEMBERS SHALL BE COMPUTED IN ACCORDANCE WITH

16.2. THE TRUSS FABRICATOR IS RESPONSIBLE FOR THE DESIGN AND DETAILING OF ALL

16.3. THE TRUSS MANUFACTURER SHALL DESIGN FOR THE FOLLOWING SUPERIMPOSED LOADS (IN

16.4. DESIGN ROOF TRUSSES TO RESIST THE FOLLOWING WIND SUPERIMPOSED NET UPLIFT WIND

C. INTERIOR ZONES (REGIONS THAT ARE NOT EDGE OR CORNER ZONES) — 20 PSF.

16.5. SUBMIT CALCULATIONS AND SHOP DRAWINGS FOR DETAILS FABRICATION, AND ERECTION

16.6. SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR PRIOR TO

16.7. ALL TEMPORARY AND PERMANENT BRACING MEMBERS AND CONNECTIONS REQUIRED FOR

16.8. TEMPORARY BRACING SHALL NOT IMPOSE ANY FORCE ON THE SUPPORTING STRUCTURE.

OF COLD-FORMED METAL TRUSS FRAMING. DRAWINGS SHALL INCLUDE LAYOUT, SPACING,

TYPE, MATERIAL/MEMBER PROPERTIES, TEMPORARY BRACING, PERMANENT BRACING, AND

ALL DETAILS OF CONNECTIONS FOR ALL COLD-FORMED METAL TRUSS FRAMING INDICATED

SUBMISSION. DRAWINGS SHALL BEAR THE CONTRACTOR'S APPROVAL STAMP ACCEPTING

RESPONSIBILITY FOR DIMENSIONS, QUANTITIES, AND COORDINATION WITH THE OTHER

PREFABRICATED COLD-FORMED METAL TRUSSES SHALL BE DESIGNED AND DETAILED ON THE

TRUSS MANUFACTURER'S ERECTION PLANS. BRACING MEMBERS SHALL BE FURNISHED AND

PERMANENT BRACING FORCES SHALL BE TRANSFERRED TO THE ROOF DIAPHRAGM BY THE

B. CORNER ZONES (REGIONS WITHIN Z DISTANCE OF TWO INTERSECTING ROOF EDGES) -50

ENGINEER SHALL SEAL AND SIGN BOTH CALCULATIONS AND SHOP DRAWINGS.

A. TOP CHORD DEAD LOAD------ 15 PSF

B. BOTTOM CHORD DEAD LOAD----- 10 PSF

C. TOP CHORD LIVE LOAD----- 20 PSF

A. EDGE ZONES (REGIONS WITHIN Z DISTANCE OF ROOF EDGE) - 35 PSF.

AISI "NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL

PREFABRICATED COLD-FORMED METAL TRUSS FRAMING AND SHALL RETAIN A

PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. THE

ADDITION TO ANY LOADS NOTED ON THE STRUCTURAL DRAWINGS OR OTHER CONSUL

15.4. THE CONTRACTOR SHALL FOLLOW ALL MANUFACTURER'S INSTALLATION GUIDELINES,

15.5. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S

15.6. POST-INSTALLED ANCHORS SHALL BE INSTALLED PERPENDICULAR TO THE FACE OF THE

15.7. IF LARGE DIAMETER HOLES OR SLOTTED HOLES ARE USED IN CONNECTION PLATES FOR

15.8. A REPRESENTATIVE OF THE POST-INSTALLED ANCHOR MANUFACTURER SHALL BE PRESENT

15.9. INSTALLATION OF POST-INSTALLED ANCHORS SHALL BE OBSERVED BY THE SPECIAL

MUST BE WELDED TO THE CONNECTION PLATE TO TRANSFER THE SPECIFIED LOAD.

CONTRACTOR TO RETURN AND PROVIDE INSTRUCTION TO THE NEW INSTALLER(S).

TO THE EDGE OF A MEMBER THAN SHOWN ON THE DRAWINGS.

OF THE SPECIFIED TOLERANCE SHALL BE CONSIDERED UNACCEPTABLE.

15.3. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH

BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES IN THE

FINAL CONSTRUCTED CONFIGURATION ONLY. IT IS THE RESPONSIBILITY OF THE

CONTRACTOR TO ADEQUATELY BRACE THE WALLS FOR VERTICAL AND LATERAL LOADS THAT

INCLUDING CORNERS AND TEES WHERE NO CONTROL JOINT IS USED.

F. ALL HORIZONTAL JOINT REINFORCING SHALL STOP AT CONTROL JOINTS.

B. CONTROL JOINTS SHALL NOT BE WITHIN 4'-0" FROM ANY CORNER.

COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF CONSTRUCTION.

14.13. PROVIDE HORIZONTAL LADDER-TYPE JOINT REINFORCEMENT AT 16" O.C., U.N.O.

15.1. POST-INSTALLED ANCHORS SHALL COMPLY WITH ACI-318.

AS REQUIRED BY THE BUILDING CODE.

SPECIFICATIONS, AND RECOMMENDATIONS.

SHALL BE FILLED WITH NON-SHRINK EPOXY GROUT.

POST-INSTALLED ANCHOR (CONTINUOUS OBSERVATION):

MANUFACTURER.

RECOMMENDATIONS.

16. PREFABRICATED COLD-FORMED METAL TRUSS:

TANTS/TRADES DRAWINGS):

PRESSURES

TRADES.

D. DISTANCE Z - 12 FT.

ON THE STRUCTURAL DRAWINGS.

INSTALLED BY THE GENERAL CONTRACTOR.

BRACING DESIGN PROVIDED BY THE TRUSS MANUFACTURER.

OPERATIONS.

DOES NOT CONFLICT WITH THE POST-INSTALLED ANCHORS.

B. LOCATION OF ANCHOR HOLES ARE AS INDICATED ON THE DRAWINGS.

E. POST-INSTALLED ANCHOR SIZE AND LENGTH ARE PER THE DRAWINGS.

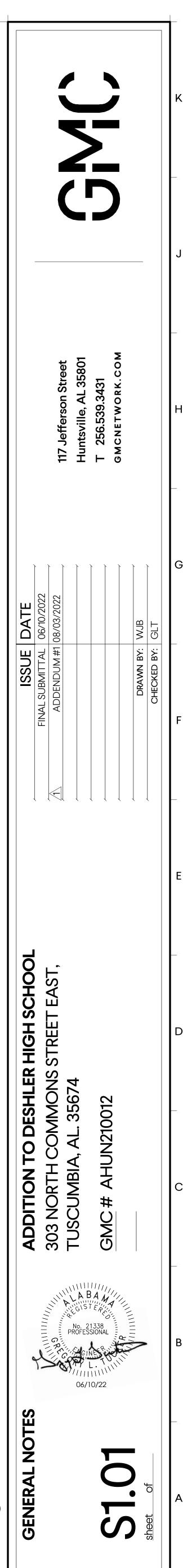
G. PROVIDE COPIES OF EACH DAY'S REPORT TO THE ARCHITECT.

15.10. ANCHORAGE TO HOLLOW CONCRETE MASONRY NOT PERMITTED

3300 CAHABA ROAD, SUITE 210 BIRMINGHAM, ALABAMA 35223 PHONE: 205.879.5660

FAX: 205.879.5606

TUCKER · JONES ENGINEERS ASSOCIATED. P.C. PROJECT #22025



GENERAL REVISIONS

Key not			
е 1 2	MATERIAL/ACTIVITY INSPECTION OF FABRICATORS (IBC 1704.2.5) VERIFY FABRICATION/QUALITY CONTROL PROCEDURES	PERIODIC	INSTRUCTIONS/FREQUENCY PERFORM IN-PLANT REVIEW OF FABIRCATOR QUALITY CONTROL PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO THE FABRICATOR'S SCOPE OF WORK WHERE FABRICATION OF STRUCTURAL LOAD-BEARING OR LATERAL LOAD-RESISTING MEMBERS OR ASSEMBLIE DEINC CONDUCTED ON THE FABRICATOR'S REFAMISES
3 4	SPECIAL CASES (IBC 1705.1.1) WORK UNUSUAL IN NATURE, INCLUDING BUT NOT LIMITED TO ALTERNATIVE MATERIALS AND SYSTEMS, UNUSUAL DESIGN APPLICATIONS, AND/OR MATERIALS AND SYSTEMS WITH SPECIAL MANUFACTURER'S DECUMPERATING	AS NOTED	BEING CONDUCTED ON THE FABRICATOR'S PREMISES. PERFORM SUBMITTAL REVIEW, SHOP AND/OR FIELD INSPECTION PER DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE OR REPORT FROM A ACCEPTED ENTITY.
5	REQUIREMENTS STEEL CONSTRUCTION (IBC 1705.2) FABRICATOR AND ERECTOR DOCUMENTS (VERIFY REPORTS AND CERTIFICATES AS LISTED IN AISC 360, CHAPTER N, SECTION N3 FOR COMPLIANCE WITH	each submittal	TO BE COMPLETED IN SUBMITTAL REVIEW PROCESS.
7	CONSTRUCTION DOCUMENTS) MATERIAL VERIFICATION OF STRUCTURAL STEEL VERIFY DIAMETER, GRADE, TYPE, LENGTH, AND EMBEDMENT OF ANCHOR RODS FOR CONFORMANCE WITH CONSTRUCTION DOCUMENTS	PERIODIC CONTINUOUS	PERFORM SHOP AND FIELD INSPECTION.
9	VERIFY MEMBER LOCATIONS, BRACES, STIFFENERS, AND APPLICATION OF JOINT DETAILS AT EACH CONNECTION COMPLY WITH CONSTRUCTION DOCUMENTS STRUCTURAL STEEL (IBC 1705.2.1)	CONTINUOUS	
11	PRIOR TO WELDING (TABLE N5.4-1, AISC 360-10): VERIFY WELDING PROCEDURES (WPS) AND CONSUMABLE CERTIFICATES MATERIAL IDENTIFICATION	PERFORM	TO BE COMPLETED IN SUBMITTAL REVIEW PROCESS. PERFORM SHOP AND FIELD INSPECTION TO VERIFY TYPE AND GRADE O
14	WELDER IDENTIFICATION FIT-UP GROOVE WELDS	OBSERVE	MATERIAL. A SYSTEM SHALL BE MAINTAINED BY WHICH A WELDER WHO HAS WELE A JOINT OR MEMBER CAN BE IDENTIFIED. VERIFY JOINT PREPARATION, DIMENSIONS, CLEANLINESS, TACKING, AN
16 17	ACCESS HOLES FIT-UP OF FILLET WELDS	OBSERVE OBSERVE	BACKING. VERIFY CONFIGURATION AND FINISH. VERIFY ALIGNMENT, GAPS AT ROOT, CLEANLINESS OF STEEL SURFACES AND TACK WELD QUALITY AND LOCATION.
18 19 20	DURING WELDING (TABLE N5.4-2, AISC360-10): USE OF QUALIFIED WELDERS CONTROL AND HANDLING OF WELDING CONSUMABLES	OBSERVE OBSERVE	VERIFY THAT WELDERS ARE APPROPRIATELY QUALIFIED. VERIFY PACKAGING AND EXPOSURE CONTROL.
21 22 23	CRACKED TACK WELDS ENVIRONMENTAL CONDITIONS WPS FOLLOWED	OBSERVE OBSERVE OBSERVE	VERIFY THAT WELDING DOES NOT OCCUR OVER CRACKED TACK WELD VERIFY WIND SPEED IS WITHIN LIMITS AS WELL AS PRECIPITATION AND TEMPERATURE. VERIFY ITEMS SUCH AS SETTINGS ON WELDING EQUIPMENT, TRAVEL SPE
24	WELDING TECHNIQUES AFTER WELDING (TABLE N5.4-3, AISC 360-10):	OBSERVE	WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED, AND PROPER POSITIC VERIFY INTERPASS AND FINAL CLEANING, EACH PASS IS WITHIN PROFIL LIMITATIONS, AND QUALITY OF EACH PASS.
26 27 28	WELDS CLEANED SIZE, LENGTH, AND LOCATION OF WELDS WELDS MEET VISUAL ACCEPTANCE CRITERIA	OBSERVE PERFORM PERFORM	VERIFY THAT WELDS HAVE BEEN PROPERLY CLEANED. VERIFY THAT WELDS MEET VISUAL ACCEPTANCE CRITERIA INCLUDING: CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, AND POROSITY.
	ARC STRIKES K-AREA	PERFORM PERFORM	WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENE HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AI FOR CRACKS WITHIN 3 INCHES OF THE WELD.
31 32 33 34	BACKING & WELD TABS REMOVED REPAIR ACTIVITIES DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT/MEMBER NONDESTRUCTIVE TESTING (SECTION N5.5, AISC 360-10):	PERFORM PERFORM PERFORM	
35	CJP GROOVE WELDS NDT (RISK CAT. II)	PERFORM	ULTRASONIC TESTING SHALL BE PERFORMED ON 10% OF CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS SUBJECT TO TRANSVERSELY APPLIED TENSION LOADING IN MATERIALS 5/16-INCH THICK OR GREAT TESTING RATE MUST BE INCREASED IF >5% OF WELDS TESTED HAVE
36 37	CJP GROOVE WELDS NDT (RISK CAT. III OR IV) ACCESS HOLES NDT	PERFORM	UNACCEPTABLE DEFECTS. A REDUCTION IN THE RATE OF ULTRASONIC TESTING IS ALLOWED PER SECTION N5.5E. THERMALLY CUT SURFACES OF ACCESS HOLES MUST BE TESTED FOR
38	WELDED JOINTS SUBJECT TO FATIGUE	PERFORM	CRACKS USING MT OR PT WHEN THE FLANGE THICKNESS OF A ROLLED SHAPE OR THE WEB OF A BUILT-UP SHAPE EXCEEDS 2 INCHES. WHEN REQUIRED BY APPENDIX 3, TABLE A-3.1, WELDED JOINTS REQUIR WELD SOUNDNESS TO BE ESTABLISHED BY RADIOGRAPHIC OR ULTRASONIC INSPECTION SHALL BE TESTED AS PRESCRIBED.
39 40 41 42	PRIOR TO BOLTING (TABLE N5.6-1, AISC 360-10): >>NOT REQUIRED IF ONLY SNUG-TIGHT JOINTS ARE SPECIFIED [PER SECTION N5.6(1) OF AISC 360-10]. CERTIFICATIONS OF FASTENERS FASTENERS MARKED	PERFORM OBSERVE	VERIFY THAT FASTENERS HAVE BEEN MARKED IN ACCORDANCE WITH
43	PROPER FASTENERS FOR JOINT PROPER BOLTING PROCEDURE	OBSERVE	ASTM REQUIREMENTS. VERIFY GRADE, TYPE, AND BOLT LENGTH IF THREADS ARE EXCLUDED FROM THE SHEAR PLANE. VERIFY PROPER PROCEDURE IS USED FOR THE JOINT DETAIL.
45 46	CONNECTING ELEMENTS PRE-INSTALLATION VERIFICATION TESTING	OBSERVE OBSERVE	VERIFY APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET REQUIREMENTS. OBSERVE AND DOCUMENT VERIFICATION TESTING BY INSTALLATION PERSONNEL FOR FASTENER ASSEMBLIES AND METHODS USED.
47 48	PROPER STORAGE DURING BOLTING (TABLE N5.6-2, AISC 360-10):	OBSERVE	VERIFY PROPER STORAGE OF BOLTS, NUTS, WASHERS, AND OTHER FASTENER COMPONENTS.
49 50	>>NOT REQUIRED IF ONLY SNUG-TIGHT JOINTS ARE SPECIFIED [PER SECTION N5.6(1) OF AISC 360-10]. >>NOT REQUIRED FOR PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT METHOD WITH MATCH- MARKING, DIRECT-TENSION INDICATORS,		
51	OR TWIST-OFF TYPE TENSION CONTROL METHOD [PER SECTION N5.6(2) OF AISC 360-10]. FASTENER ASSEMBLIES	OBSERVE	VERIFY THAT FASTENER ASSEMBLIES ARE OF SUITABLE CONDITION, PLAC IN ALL HOLES, AND WASHERS ARE POSITIONED AS REQUIRED.
52 53	SNUG-TIGHT PRIOR TO PRETENSIONING FASTENER COMPONENT	OBSERVE OBSERVE	VERIFY THAT JOINTS ARE BROUGHT TO SNUG-TIGHT CONDITION PRIOR PRETENSIONING OPERATION. VERIFY THAT FASTENER COMPONENT IS NOT TURNED BY THE WRENCH
54	PRETENSIONED FASTENERS	OBSERVE	PREVENTED FROM ROTATING. VERIFY THAT FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MO RIGID POINT TOWARD THE FREE EDGES.
55 56 57	AFTER BOLTING (TABLE N5.6-3, AISC 360-10): DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS OTHER STEEL INSPECTIONS (SECTION N5.7, AISC 360-10; TABLES J8-1& J10-1, AISC 341-10):	PERFORM	
58 59	STRUCTURAL STEEL DETAILS ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL		ALL FABRICATED STEEL OR STEEL FRAMES SHALL BE INSPECTED TO VERI COMPLIANCE WITH THE DETAILS SHOWN IN THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS, AN PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. SHALL BE ON THE PREMISES DURING THE PLACEMENT OF ANCHOR ROU AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR
,-			COMPLIANCE WITH CONSTRUCTION DOCUMENTS. VERIFY THE DIAMET GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM AND THE EXTENT OR DEPTH OF EMBEDMENT PRIOR TO PLACEMENT OF CONCRETE.
60 61	REDUCED BEAM SECTIONS (RBS) PROTECTED ZONES	PERFORM	VERIFY CONTOUR AND FINISH AS WELL AS DIMENSIONAL TOLERANCES (SEE TABLE J8-1 OF AISC 341-10). VERIFY THAT NO HOLES OR UNAPPROVED ATTACHMENTS ARE MADE
-	H-PILES	PERFORM	WITHIN THE PROTECTED ZONE (SEE TABLE J8-1 OF AISC 341-10). VERIFY THAT NO HOLES OR UNAPPROVED ATTACHMENTS OCCUR WITH THE PROTECTED ZONES OF PILING (SEE TABLE J10-1 OF AISC 341-10).
63 64 65	STEEL ELEMENTS OF COMPOSITE CONSTRUCTION (TABLE N6.1, AISC 360-10; TABLES J9-3, AISC 341-10): PLACEMENT AND INSTALLATION OF STEEL DECK PLACEMENT AND INSTALLATION OF STEEL HEADED	PERFORM PERFORM	
66 67	STUD ANCHORS DOCUMENT ACCEPTANCE OR REJECTION OF STEEL ELEMENTS REINFORCING STEEL	PERFORM	VERIFY APPROPRIATE REINFORCEMENT TYPE/GRADE, SIZE, SPACING, A
68	COMPOSITE MEMBER SIZE	Continuous	ORIENTATION; THAT IT HAS NOT BEEN RE-BENT IN THE FIELD; THAT IT IS CORRECTLY TIED AND SUPPORTED; AND THAT REQUIRED STEEL CLEARANCES HAVE BEEN PROVIDED. VERIFY THAT COMPOSITE MEMBER IS THE REQUIRED SIZE.
69 70 71	COLD-FORMED STEEL DECK (IBC 1705.2.2) PRIOR TO DECK PLACEMENT (SDI QA/QC - 2011, TABLE 1.1): VERIFY COMPLIANCE OF MATERIALS (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION	PERFORM	
72	DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES	PERFORM	
73	AFTER DECK PLACEMENT (SDI QA/QC - 2011, TABLE 1.2):		

2015 IBC - SCHEDULE OF SPECIAL INSPECTIONS

EXTENT INSTRUCTIONS/FREQUENCY MATERIAL/ACTIVITY VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL PERFORM CERTIFICATIONSTHAT COMPLY WITH THE CONSTRUCTION DOCUMENTS. DOCUMENT ACCEPTANCE OR REJECTION OF PERFORM INSTALLATION OF DECK AND DECK ACCESSORIES PRIOR TO WELDING (SDI QA/QC - 2011, TABLE 1.3): 78 WELDING PROCEDURE SPECIFICATIONS (WPS) OBSERVE AVAILABLE MANUFACTURER CERTIFICATIONS FOR WELDING OBSERVE CONSUMABLES AVAILABLE MATERIAL IDENTIFICATION (TYPE/GRADE) OBSERVE CHECK WELDING EQUIPMENT OBSERVE DURING WELDING (SDI QA/QC - 2011, TABLE 1.4): USE OF QUALIFIED WELDERS OBSERVE 4 CONTROL AND HANDLING OF WELDING OBSERVE CONSUMABLES ENVIRONMENTAL CONDITIONS (WIND SPEED, OBSERVE MOISTURE, TEMPERATURE) WPS FOLLOWED OBSERVE AFTER WELDING (SDI QA/QC - 2011, TABLE 1.5 88 VERIFY SIZE AND LOCATION OF WELDS, INCLUDING PERFORM SUPPORT, SIDELAP, AND PERIMETER WELDS. WELDS MEET VISUAL ACCEPTANCE CRITERIA PFRFORM VERIFY REPAIR ACTIVITIES PERFORM DOCUMENT ACCEPTANCE OR REJECTION OF WELDS PERFORM PRIOR TO MECHANICAL FASTENING (SDI QA/QC -2011, TABLE 1.6): MANUFACTURER INSTALLATION INSTRUCTIONS OBSERVE AVAILABLE FOR MECHANICAL FASTENERS PROPER TOOLS AVAILABLE FOR FASTENER OBSERVE INSTALLATION PROPER STORAGE FOR MECHANICAL FASTENERS OBSERVE DURING MECHANICAL FASTENING (SDI QA/QC - 2011 TABLE 1.7): FASTENERS ARE POSITIONED AS REQUIRED OBSERVE 98 FASTENERS ARE INSTALLED IN ACCORDANCE WITH OBSERVE MANUFACTURER'S INSTRUCTIONS 99 AFTER MECHANICAL FASTENING (SDI QA/QC - 2011, TABLE 1.8): 00 CHECK SPACING, TYPE, AND INSTALLATION OF PERFORM SUPPORT FASTENERS 1 CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP PERFORM FASTENERS 02 CHECK SPACING, TYPE, AND INSTALLATION OF PERFORM PERIMETER FASTENERS 3 VERIFY REPAIR ACTIVITIES PERFORM 104 DOCUMENT ACCEPTANCE OR REJECTION OF PERFORM MECHANICAL FASTENERS 105 OPEN-WEB STEEL JOISTS AND JOIST GIRDERS (IBC 1705.2.3) 106 INSTALLATION OF OPEN-WEB STEEL JOISTS AND JOIST GIRDERS (IBC TABLE 1705.2.3): 07 END CONNECTIONS - WELDED OR BOLTED PERIODIC SJI SPECIFICATIONS LISTED IN SECTION 2207.1 SJI SPECIFICATIONS LISTED IN SECTION 2207.1 FOR STANDARD BRIDGING. 108 BRIDGING - HORIZONTAL OR DIAGONAL PERIODIC SPECIAL INSPECTION ALSO REQUIRED FOR BRIDGING THAT DIFFERS FROM THESE SJI SPECIFICATIONS. 109 COLD-FORMED STEEL CONSTRUCTION 110 TRUSSES SPANNING 60 FEET OR GREATER (IBC 1705.2.4) PERFORM VERIFY THAT THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING ARE INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE. 111 COLD-FORMED STEEL WIND RESISTANCE (1705.11.2) REQUIRED IN AREAS OF WIND EXPOSURE CATEGORY B, WHERE V(ASD) = 20MPH OR GREATER OR AREAS OF WIND EXPOSURE CATEGORY C OR D, WHERE V(ASD) IS 110MPH OR GREATER. 2 WELDING OPERATIONS OF ELEMENTS OF THE MAIN PERIODIC WINDFORCE-RESISTING SYSTEM 3 SCREW ATTACHMENT, BOLTING, ACHORING AND PERIODIC EXCEPTIONS: (1) IF THE SHEATHING IS GYPSUM BOARD OR FIBERBOARD OTHER FASTENING OF ELEMENTS OF THE MAIN (2) IF THE SHEATHING IS WOOD STRUCTURAL PANEL OR STEEL SHEETS ON ONLY ONE SIDE OF THE SHEAR WALL, SHEAR PANEL OR DIAPHRAGM WINDFORCE-RESISTING SYSTEM ASSEMBLY, AND THE FASTENER SPACING >4IN ON CENTER. 114 COLD-FORMED STEEL SEISMIC RESISTANCE (1705.12.3) APPLICABLE TO SEISMIC FORCE-RESISTING SYSTEMS WITH SEISMIC DESIGN CATEGORY C, D, E, OR F. 15 WELDING OPERATIONS OF ELEMENTS OF THE SEISMIC PERIODIC FORCE-RESISTING SYSTEM 16 SCREW ATTACHMENT, BOLTING, ACHORING AND PERIODIC EXCEPTIONS: (1) IF THE SHEATHING IS GYPSUM BOARD OR FIBERBOARD OTHER FASTENING OF ELEMENTS OF THE SEISMIC (2) IF THE SHEATHING IS WOOD STRUCTURAL PANEL OR STEEL SHEETS ON FORCE-RESISTING SYSTEM ONLY ONE SIDE OF THE SHEAR WALL, SHEAR PANEL OR DIAPHRAGM ASSEMBLY, AND THE FASTENER SPACING >4IN. WHERE APPLICABLE, SEE ALSO SECTION 1705.12, SPECIAL INSPECTIONS 17 CONCRETE CONSTRUCTION (IBC 1705.3 AND IBC FOR SEISMIC RESISTANCE. TABLE 1705.3) 118 INSPECT REINFORCEMENT, INCLUDING PRESTRESSING PERIODIC REFERENCE ACI 318 CH. 20, 25.2, 25.3, 26.5.1-26.5.3 AND IBC 1908.4 TENDONS, AND VERIFY PLACEMENT. 9 WELDING OF REINFORCING BARS (IBC 1705.3.1): REFERENCE AWS D1.4 AND ACI 318: 26.5.4 120 VERIFY WELDABILITY OF REINFORCING BARS OTHER PERIODIC THAN ASTM A 706 1 INSPECT SINGLE-PASS FILLET WELDS (MAX = 5/16") PERIODIC 22 INSPECT ALL OTHER WELDS CONTINUOUS 23 INSPECT ANCHORS CAST IN CONCRETE PERIODIC REFERENCE ACI 318: 17.8.2 124 INSPECT ANCHORS POST-INSTALLED IN HARDENED SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN CONCRETE MEMBERS: THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPOVED SOURCE IN ACCORDANCE WITH ACI 318: 17.8.2 OR OTHER QUALIFICATION PROCEDURES. WHERE NOT PROVIDED, CONSULT WITH REGISTERED DESIGN PROFESSIONAL FOR REQUIREMENTS TO BE APPROVED BY BUILDING OFFICIAL PRIOR TO PROCEEDING. 25 ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR CONTINUOUS REFERENCE ACI 318: 17.8.2.4 UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS 126 MECHANICAL ANCHORS AND ADHESIVE ANCHORS PERIODIC REFERENCE ACI 318: 17.8.2 NOT DEFINED ABOVE 7 VERIFY USE OF REQUIRED DESIGN MIX. PERIODIC REFERENCE ACI 318: CH. 19, 26.4.3, 26.4.4 AND IBC 19104.1-2, 1908.2-3 128 PRIOR TO CONCRETE PLACEMENT, FABRICATE CONTINUOUS REFERENCE ASTM C172, C31; ACI 318: 26.4.5, 26.12; IBC 1908.10 SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. 129 INSPECT CONCRETE AND SHORTCRETE PLACEMENT CONTINUOUS REFERENCE ACI 318: 26.4.5 AND IBC 1908.6-7, 1908.9 FOR PROPER APPLICATION TECHNIQUES. 130 VERIFY MAINTENANCE OF SPECIFIED CURING PERIODIC REFERENCE ACI 318: 26.4.7-9 AND IBC 1908.9 TEMPERATURE AND TECHNIQUES. INSPECT PRESTRESSED CONCRETE FOR: 2 APPLICATION OF PRESTRESSING FORCES CONTINUOUS REFERENCE ACI 318: 26.9.2.1 3 GROUTING OF BONDED PRESTRESSING TENDONS CONTINUOUS REFERENCE ACI 318: 26.9.2.3 134 INSPECT ERECTION OF PRECAST CONCRETE MEMBERS. PERIODIC REFERENCE ACI 318: 26.8 135 VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO PERIODIC REFERENCE ACI 318: 26.10.2 STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS. 136 INSPECT FORMWORK FOR SHAPE, LOCATION AND PERIODIC REFERENCE ACI 318: 26.10.1 (B) DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED. 137 MASONRY CONSTRUCTION (IBC 1705.4) APPLICABLE TO RISK CATEGORIES I, II, III 138 LEVEL B QUALITY ASSURANCE (TMS 402-13/ACI 530-13/ASCE 5-13, TABLE 3.1.2) 139 VERIFY SLUMP FLOW AND VISUAL STABILITY INDEX (SVI) MINIMUM TEST AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATION ARTICAL 1.5 B.1.B.3 FOR SELF-CONSOLIDATING GROUT. 140 VERIFY F'M AND F'AAC IN ACCORDANCE WITH MINIMUM TEST SPECIFICATION ARTICLE 1.4 B PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THIS CODE. PERIODIC REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 1.5 141 VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS. 142 AS MASONRY CONSTRUCTION BEGINS, VERIFY THE FOLLOWING ARE IN COMPLIANCE: PERIODIC REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 2.1, 2.6 A 43 PROPORTIONS OF SITE-PREPARED MORTAR 144 CONSTRUCTION OF MORTAR JOINTS PERIODIC | REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.3 B 145 GRADE AND SIZE OF PRESTRESSING TENDONS AND PERIODIC REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 2.4 B, 2.4 H ANCHORAGES 146 LOCATION OF REINFORCEMENT, CONNECTORS, AND PERIODIC REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.4, 3.6 A PRESTRESSING TENDONS AND ANCHORAGES 147 PRESTRESSING TECHNIQUE PERIODIC REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.6 B 148 PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY CONTINUOUS IS REQUIRED FOR THE FIRST 5000SF OF AAC MASONRY; /PERIODIC | PERIODIC IS REQUIRED AFTER THE FIRST 5000SF OF AAC MASONRY. REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 2.1 C 149 PRIOR TO GROUTING, VERIFY THE FOLLOWING ARE IN COMPLIANCE: PERIODIC REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.2 D, 3.2 F 50 GROUT SPACE REFERENCE TMS 402/ACI 530/ASCE 5 SEC. 6.1 AND TMS 602/ACI GRADE, TYPE, AND SIZE OF REINFORCEMENT AND PERIODIC ANCHOR BOLTS, AND PRESTRESSING TENDONS AND 530.1/ASCE 6, ART. 2.4, 3.4 ANCHORAGES REFERENCE TMS 402/ACI 530/ASCE 5 SEC. 6.1, 6.2.1, 6.2.6-7 AND TMS 52 PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PERIODIC PRESTRESSING TENDONS AND ANCHORAGES 602/ACI 530.1/ASCE 6, ART. 3.2 E, 3.4, 3.6 A 53 PROPORTIONS OF SITE-PREPARED GROUT AND REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 2.6 B, 2.4 G.1.B PERIODIC PRESTRESSING GROUT FOR BONDED TENDONS PERIODIC REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.3 B 154 CONSTRUCTION OF MORTAR JOINTS 55 VERIFY DURING CONSTRUCTION: 156 SIZE AND LOCATION OF STRUCTURAL ELEMENTS PERIODIC REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.3 F

Key			F SPECIAL INSPECTIONS
not e	MATERIAL/ACTIVITY	EXTENT	INSTRUCTIONS/FREQUENCY
	TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	PERIODIC	REFERENCE TMS 402/ACI 530/ASCE 5 SEC. 1.2.1(E), 6.1.4.3, 6.2.1
	WELDING OF REINFORCEMENT PREPARATION, CONSTRUCTION, AND PROTECTION OF	CONTINUOUS PERIODIC	REFERENCE TMS 402/ACI 530/ASCE 5 SEC. 8.1.6.7.2, 9.3.3.4(C), 11.3.3.4(B) REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 1.8 C, 1.8 D
	MASONRY DURING COLD WEATHER (<40°F) OR HOT WEATHER (>90°F)		
	APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE		REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.6 B
	PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE PLACEMENT OF AAC MASONRY UNITS AND		REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.5, 3.6 C CONTINUOUS IS REQUIRED FOR THE FIRST 5000SF OF AAC MASONRY;
63	CONSTRUCTION OF THIN-BED MORTAR JOINTS OBSERVE PREPARATION OF GROUT SPECIMENS,	/PERIODIC	PERIODIC IS REQUIRED AFTER THE FIRST 5000SF OF AAC MASONRY. REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.3 B.9, 3.3 F.1.B REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 1.4 B.2.A.3, 1.4 B.2.B.3, 1.4
64	MORTAR SPECIMENS, AND/OR PRISMS LEVEL C QUALITY ASSURANCE (TMS 402-13/ACI 530-13/ASCE 5-13, TABLE 3.1.3)		B.2.C.3, 1.4 B.3, 1.4 B.4 APPLICABLE TO RISK CATEGORY IV
	VERIFY F'M AND F'AAC IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.4 B PRIOR TO CONSTRUCTION AND FOR EVERY 5000SF DURING CONSTRUCTION		MINIMUM TEST
	VERIFY PROPORTIONS OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT, AS DELIVERED TO THE PROJECT SITE		MINIMUM TEST
	VERIFY SLUMP FLOW AND VISUAL STABILITY INDEX (SVI) AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.5 B.1.B.3 FOR SELF-CONSOLIDATING GROUT		MINIMUM TEST
	VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS.	PERIODIC	REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 1.5
70	VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE: PROPORTIONS OF SITE-MIXED MORTAR, GROUT AND	PERIODIC	REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 2.1, 2.6 A, 2.6 B, 2.6 C, 2.4
	PRESTRESSING GROUT FOR BONDED TENDONS GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND	PERIODIC	G.1.B REFERENCE TMS 402/ACI 530/ASCE 5 SEC. 6.1 AND TMS 602/ACI 530.1/ASCE 6, ART. 2.4, 3.4
72	ANCHORAGES PLACEMENT OF MASONRY UNITS AND CONSTRUCTION OF MORTAR JOINTS	PERIODIC	REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.3 B
73	PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	CONTINUOUS	REFERENCE TMS 402/ACI 530/ASCE 5 SEC. 6.1, 6.2.1, 6.2.6-7 AND TMS 602/ACI 530.1/ASCE 6, ART. 3.2 E, 3.4, 3.6 A
75	GROUT SPACE PRIOR TO GROUTING PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	CONTINUOUS	REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.2 D, 3.2 F REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.5, 3.6 C
	SIZE AND LOCATION OF STRUCTURAL ELEMENTS TYPE, SIZE, AND LOCATION OF ANCHORS INCLUDING	PERIODIC CONTINUOUS	REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.3 F REFERENCE TMS 402/ACI 530/ASCE 5 SEC. 1.2.1(E), 6.1.4.3, 6.2.1
	OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, AND OTHER CONSTRUCTION		
	WELDING OF REINFORCEMENT PREPARATION, CONSTRUCTION, AND PROTECTION OF	CONTINUOUS PERIODIC	REFERENCE TMS 402/ACI 530/ASCE 5 SEC. 8.1.6.7.2, 9.3.3.4(C), 11.3.3.4(B) REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 1.8 C, 1.8 D
	APPLICATION AND MEASUREMENT OF PRESTRESSING		REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.6 B
	FORCE		
	PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY		REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 3.3 B.9, 3.3 F.1.B REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 2.1 C.1
	OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	CONTINUOUS	REFERENCE TMS 602/ACI 530.1/ASCE 6, ART. 1.4 B.2.A.3, 1.4 B.2.B.3, 1.4 B.2.C.3, 1.4 B.3, 1.4 B.4
	FIELD GLUING OPERATIONS OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM	CONTINUOUS	
	NAILING, BOLTING, AND ANCHORING AND OTHER FASTENING OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, AND SHEAR PANELS AND HOLD-DOWNS	PERIODIC	EXCEPTION: NO REQUIRED FOR WOOD SHEAR WALLS, SHEAR PANELS, AND DIAPHRAGMS, INCLUDING NAILING, BOLTING, ANCHORING AND OTHER FASTENING TO OTHER ELEMENTS OF THE MAIN WINDFORCE-RESISITNG SYSTEM WHERE THE FASTENER SPACING OF THE SHEATHING >4IN ON CENTER.
	SOILS (IBC 1705.6) VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC	
95	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC	
	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	PERIODIC	
	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	CONTINUOUS	
	SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC	
	STRUCTURAL OBSERVATIONS (IBC 1704.6) ITEM TO BE OBSERVED:		NAME OF OBSERVER:
65	FOOTINGS & PIERS DEEP FOUNDATIONS		
67	GRADE BEAMS		
	CONCRETE WALLS MASONRY WALLS		
	WOOD WALLS STEEL MOMENT FRAMES		
72	STEEL BRACED FRAMES CONCRETE MOMENT FRAMES		
74 75	Concrete diaphragms steel deck diaphragms		
77	WOOD DIAPHRAGMS POST- TENSIONED DECK		
79	OTHER: OTHER:		
	other: Pection agents firm	ADDRES	S TELEPHONE NO.
	DWNER'S TESTING AGENCY		
2.			
3.			
4. NOT	E: THE INSPECTION AND TESTING AGENT(S) SHALL BE EI		
			HE OWNER OR THE OWNER'S AGENT, AND NOT BY THE OR TESTED, ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO

NOTE: THE INSPECTION AND TESTING AGENT(S) SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT, AND NOT BY THE CONTRACTOR OF SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK. THE QUALIFICATIONS OF THE INSPECTION AGENT(S) MAY BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL. DATE:

<u>NOTES:</u>

- 1. STRUCTRUAL STEEL WELDING: A. COMPLETE AND PARTIAL PENETRATION GROOVE WELDS - CONTINUOUS.
- B. MULTIPLE PASS FILLET WELDS CONTINUOUS. C. PLUG AND SLOT WELDS - CONTINUOUS. D. SINGLE - PASS FILLET WELDS < 5/16" - PERIODIC.
- E. DECK WELDS PERIODIC.
- 2. REINFORCING STEEL WELDING: A. VERIFICATION OF WELDABILITY - PERIODIC. B. REINFORCING STEEL RESISTING FLEXURAL AND AXIAL IN INTERMEDIATE AND SPECIAL MOMENT FRAMES AND BOUNDARY ELEMENTS IN SPECIAL WALLS OR SHEAR REINF -CONTINUOUS.
- C. SHEAR REINFORCEMENT CONTINUOUS. D. OTHER REINFORCING - PERIODIC.

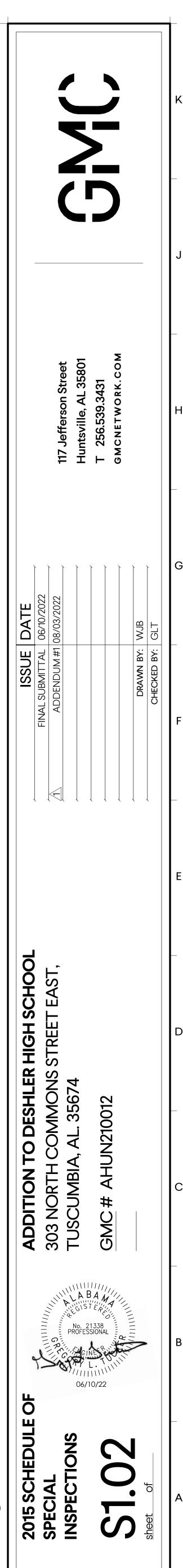
WORK. ACCEPTANCE.

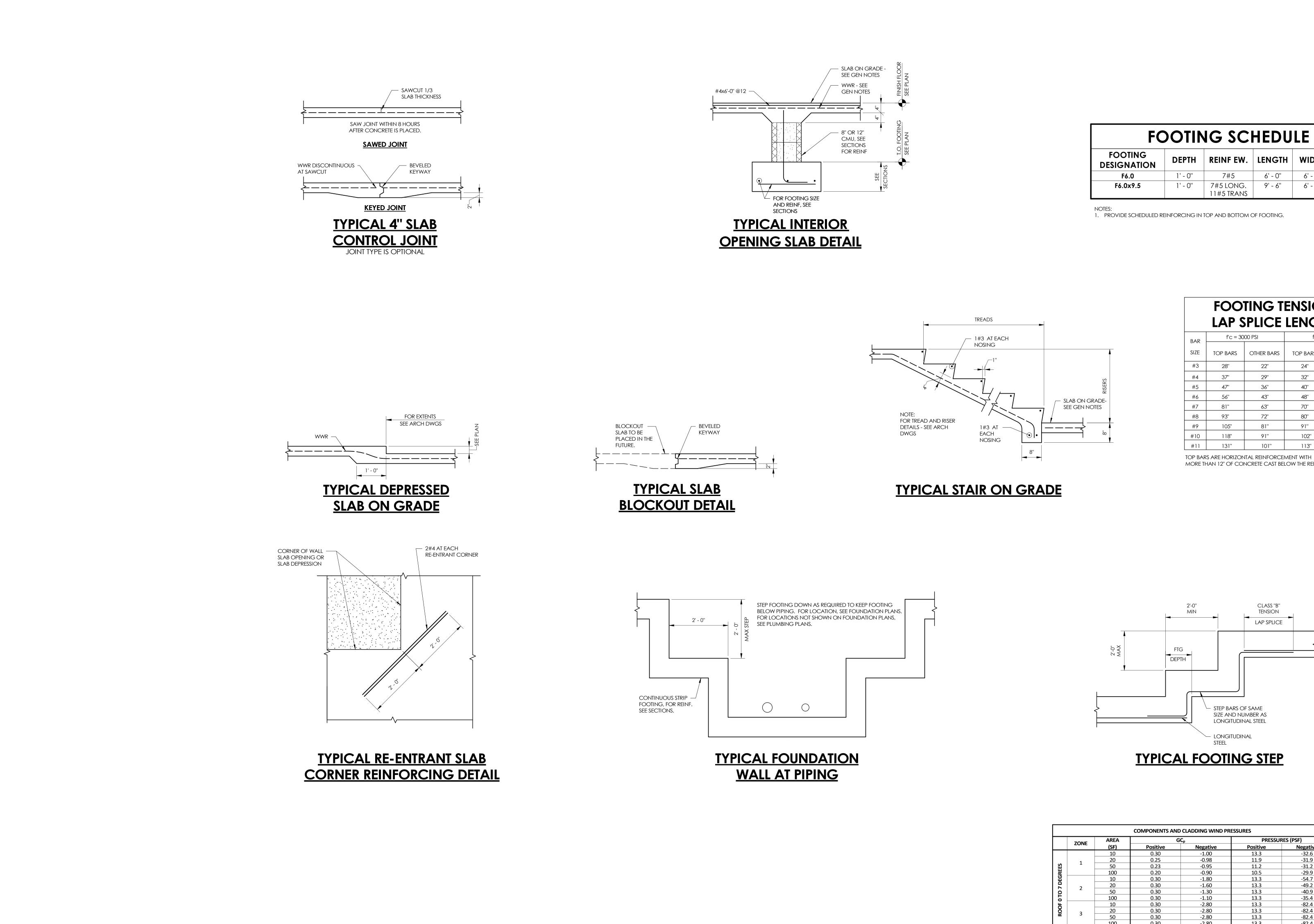
2015 IBC - SCHEDULE OF SPECIAL INSPECTIONS

- 3. EXCEPTIONS: SPECIAL INSPECTIONS SHALL NOT BE REQUIRED FOR: A. ISOLATED SPREAD CONCRETE FOOTINGS OF BUILDINGS THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK.
- B. CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF BUILDINGS THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK WHERE: 1. FOOTINGS SUPPORT WALLS OF LIGHT-FRAME CONSTRUCTION. 2. FOOTINGS ARE DESIGNED IN ACCORDANCE WITH TABLE 1809.7.
- 3. THE STRUCTURAL DESIGN OF THE FOOTING IS BASED ON A SPECIFIED COMPRESSIVE STRENGTH, F'C, NO GREATER THAN 2500 POUNDS PER SQUARE INCH (PSI), REGARDLESS OF THE COMPRESSIVE STRENGTH SPECIFIED IN THE CONSTRUCTION DOCUMENTS OR USED IN THE FOOTING CONSTRUCTION.
- C. NONSTRUCTURAL CONCRETE SLABS SUPPORTED DIRECTLY ON THE GROUND, INCLUDING PRESTRESSED SLABS ON GRADE, WHERE THE
- EFFECTIVE PRESTRESS IN THE CONCRETE IS LESS THAN 150 PSI. D. CONCRETE FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH TABLE 1807.1.6.2.
- E. CONCRETE PATIOS, DRIVEWAYS AND SIDEWALKS ON GRADE. PERIODIC - PART-TIME OR INTERMITTENT OBSERVATION OF WORK THAT HAS BEEN/IS BEING PERFORMED AND AT THE COMPLETION OF THE
- CONTINUOUS FULL-TIME OBSERVATION OF WORK. INSPECTOR IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. OBSERVE - INSPECT THESE ITEMS ON A RANDOM/INTERMITTENT BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS. PERFORM - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER, BOLTED CONNECTION, OR STEEL ELEMENT PRIOR TO FINAL









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FC	OOTIN	IG SC	HEDU	LE	
FOOTING DESIGNATION	DEPTH	REINF EW.	LENGTH	WIDTH	NOTES
F6.0	1' - 0''	7#5	6' - 0''	6' - 0''	1
F6.0x9.5	1' - 0''	7#5 LONG. 11#5 TRANS	9' - 6''	6' - 0''	1

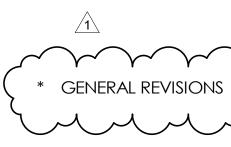
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FOOTING TENSION LAP SPLICE LENGTH

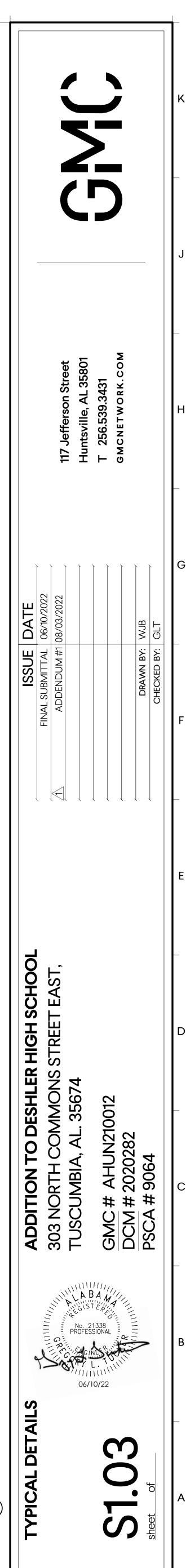
BAR	f'c = 30	00 PSI	f'c =	4000 PSI
SIZE	top bars	OTHER BARS	top bars	OTHER BARS
#3	28''	22''	24''	19"
#4	37''	29''	32''	25"
#5	47''	36"	40''	31"
#6	56"	43''	48''	37''
#7	81"	63''	70''	54"
#8	93"	72''	80''	62"
#9	105''	81"	91''	70''
#10	118"	91"	102''	79''
#11	131"	101''	113"	87''

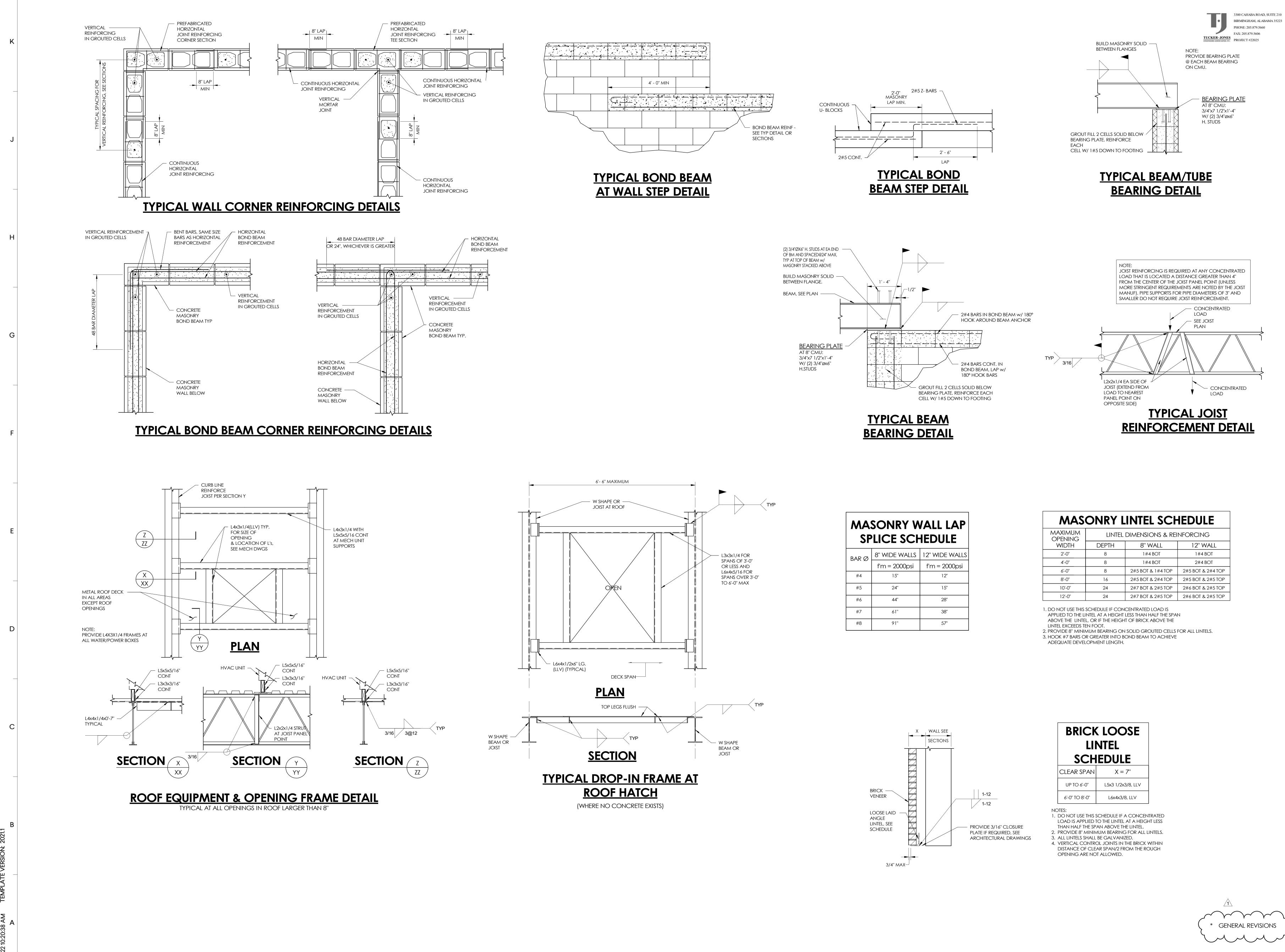
MORE THAN 12" OF CONCRETE CAST BELOW THE REINF.

ZONE J.M. (SF) Positive Negative Positive Negative 1 10 0.30 -1.00 13.3 -32.6 20 0.25 -0.98 11.9 -31.9 50 0.23 -0.95 11.2 -31.2 100 0.30 -1.60 13.3 -54.7 100 0.30 -1.60 13.3 -49.2 20 0.30 -1.60 13.3 -49.2 50 0.30 -1.10 13.3 -49.2 50 0.30 -1.10 13.3 -49.2 3 100 0.30 -2.80 13.3 -82.4 100 0.30 -2.80 13.3 -82.4 100 0.30 -2.80 13.3 -82.4 100 0.30 -2.80 13.3 -82.4 100 0.30 -2.80 13.3 -82.4 100 0.00 -1.10 32.6 -35.4 <td< th=""><th>70115</th><th>AREA</th><th>G</th><th>C_p</th><th>PRESSU</th><th>RES (PSF)</th></td<>	70115	AREA	G	C _p	PRESSU	RES (PSF)
1 10 0.30 -1.00 13.3 -32.6 20 0.25 -0.98 11.9 -31.9 50 0.23 -0.95 11.2 -31.2 100 0.20 -0.90 10.5 -29.9 2 10 0.30 -1.80 13.3 -54.7 2 20 0.30 -1.60 13.3 -49.2 50 0.30 -1.30 13.3 -49.2 50 0.30 -1.10 13.3 -49.2 3 100 0.30 -1.10 13.3 -49.2 3 100 0.30 -2.80 13.3 -49.2 3 100 0.30 -2.80 13.3 -82.4 4 100 0.30 -2.80 13.3 -82.4 100 1.00 -1.10 32.6 -35.4 20 0.95 -1.05 31.2 -34.0 50 0.90 -1.00 29.9	ZONE	(SF)				
1 50 0.23 -0.95 11.2 -31.2 100 0.20 -0.90 10.5 -29.9 2 10 0.30 -1.80 13.3 -54.7 20 0.30 -1.60 13.3 -49.2 50 0.30 -1.60 13.3 -49.2 3 20 0.30 -1.10 13.3 -49.2 3 10 0.30 -2.80 13.3 -49.2 3 20 0.30 -2.80 13.3 -82.4 10 0.30 -2.80 13.3 -82.4 3 50 0.30 -2.80 13.3 -82.4 100 0.30 -2.80 13.3 -82.4 4 100 0.30 -2.80 13.3 -82.4 4 100 1.00 -1.10 32.6 -35.4 4 100 1.00 -1.10 32.6 -35.4 50 0.90 <th< td=""><td></td><td>10</td><td>0.30</td><td></td><td>13.3</td><td></td></th<>		10	0.30		13.3	
50 0.23 -0.95 11.2 -31.2 100 0.20 -0.90 10.5 -29.9 2 10 0.30 -1.80 13.3 -54.7 20 0.30 -1.60 13.3 -49.2 50 0.30 -1.10 13.3 -49.2 3 100 0.30 -1.10 13.3 -49.2 3 100 0.30 -2.80 13.3 -49.2 3 10 0.30 -2.80 13.3 -82.4 3 50 0.30 -2.80 13.3 -82.4 4 100 1.00 -1.10 32.6 -35.4 4 100 1.00 -1.10 32.6 -35.4 4 50 0.90 -1.00 29.9 -32.6 100 1.00 -1.40 32.6 -43.7 500 0.90 -1.30 31.2 -34.0 50 0.90 -1.10	1	20	0.25	-0.98	11.9	-31.9
Image: Solution of the second system of the secon	3 1	50	0.23	-0.95	11.2	-31.2
Image: Solution of the second system of the secon	2	100	0.20	-0.90	10.5	-29.9
Image: Solution of the second system of the secon	2	10	0.30	-1.80	13.3	-54.7
Image: Solution of the second system of the secon		20	0.30	-1.60	13.3	-49.2
50 0.30 -2.80 13.3 -82.4 100 0.30 -2.80 13.3 -82.4 100 1.00 -1.10 32.6 -35.4 20 0.95 -1.05 31.2 -34.0 4 50 0.90 -1.00 29.9 -32.6 100 0.80 -0.90 27.1 -29.9 500 0.70 -0.80 24.3 -27.1 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1 20NE 1 - INTERIOR ROOF 20.00 0.70 -0.80 24.3 -27.1		50	0.30	-1.30	13.3	-40.9
50 0.30 -2.80 13.3 -82.4 100 0.30 -2.80 13.3 -82.4 100 1.00 -1.10 32.6 -35.4 20 0.95 -1.05 31.2 -34.0 4 50 0.90 -1.00 29.9 -32.6 100 0.80 -0.90 27.1 -29.9 500 0.70 -0.80 24.3 -27.1 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1 20NE 1 - INTERIOR ROOF 20.00 0.70 -0.80 24.3 -27.1	-	100	0.30	-1.10	13.3	-35.4
S0 0.30 -2.80 13.3 -82.4 100 0.30 -2.80 13.3 -82.4 4 10 1.00 -1.10 32.6 -35.4 20 0.95 -1.05 31.2 -34.0 500 0.90 -1.00 29.9 -32.6 100 0.80 -0.90 27.1 -29.9 500 0.70 -0.80 24.3 -27.1 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1 20NE 1 - INTERIOR ROOF 200 0.70 -0.80 24.3 -27.1		10	0.30	-2.80	13.3	-82.4
S0 0.30 -2.80 13.3 -82.4 100 0.30 -2.80 13.3 -82.4 4 10 1.00 -1.10 32.6 -35.4 20 0.95 -1.05 31.2 -34.0 500 0.90 -1.00 29.9 -32.6 100 0.80 -0.90 27.1 -29.9 500 0.70 -0.80 24.3 -27.1 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1 20NE 1 - INTERIOR ROOF 200 0.70 -0.80 24.3 -27.1	5 2	20	0.30	-2.80	13.3	-82.4
4 10 1.00 -1.10 32.6 -35.4 20 0.95 -1.05 31.2 -34.0 50 0.90 -1.00 29.9 -32.6 100 0.80 -0.90 27.1 -29.9 500 0.70 -0.80 24.3 -27.1 5 100 1.00 -1.40 32.6 -43.7 20 0.95 -1.30 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1 20 0.955 -1.30 31.2 -40.9 500 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1		50	0.30	-2.80	13.3	-82.4
4 20 0.95 -1.05 31.2 -34.0 50 0.90 -1.00 29.9 -32.6 100 0.80 -0.90 27.1 -29.9 500 0.70 -0.80 24.3 -27.1 500 0.70 -1.40 32.6 -43.7 20 0.95 -1.30 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1 20 0.95 -1.30 31.2 -40.9 500 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1		100	0.30	-2.80	13.3	-82.4
4 50 0.90 -1.00 29.9 -32.6 100 0.80 -0.90 27.1 -29.9 500 0.70 -0.80 24.3 -27.1 10 1.00 -1.40 32.6 -43.7 20 0.95 -1.30 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1		10	1.00	-1.10	32.6	-35.4
100 0.80 -0.90 27.1 -29.9 500 0.70 -0.80 24.3 -27.1 10 1.00 -1.40 32.6 -43.7 20 0.95 -1.30 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1		20	0.95	-1.05	31.2	-34.0
500 0.70 -0.80 24.3 -27.1 10 1.00 -1.40 32.6 -43.7 20 0.95 -1.30 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1	4	50	0.90	-1.00	29.9	-32.6
10 1.00 -1.40 32.6 -43.7 20 0.95 -1.30 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1		100	0.80	-0.90	27.1	-29.9
20 0.95 -1.30 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1	1	500	0.70	-0.80	24.3	-27.1
20 0.95 -1.30 31.2 -40.9 5 50 0.90 -1.15 29.9 -36.8 100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1		10	1.00	-1.40	32.6	-43.7
100 0.80 -1.05 27.1 -34.0 500 0.70 -0.80 24.3 -27.1	-	20	0.95	-1.30	31.2	-40.9
500 0.70 -0.80 24.3 -27.1 ZONE 1 - INTERIOR ROOF -0.80 24.3 -27.1	5	50	0.90	-1.15	29.9	-36.8
ZONE 1 - INTERIOR ROOF		100	0.80	-1.05	27.1	-34.0
		500	0.70	-0.80	24.3	-27.1
ZONE 3 - ROOF WITHIN 4' OF (2) EDGES ZONE 4 - INTERIOR WALLS	ZONE 2	100 500 - INTERIOR ROOF - ROOF WITHIN 4' C	0.80 0.70 DF BUILDING EDGE	-1.05	27.1	-34.0





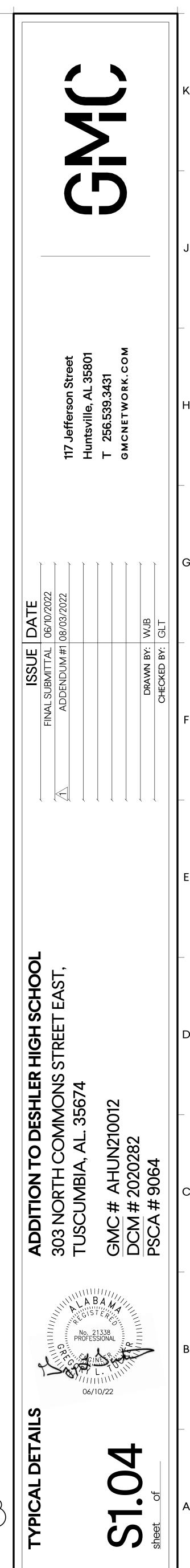


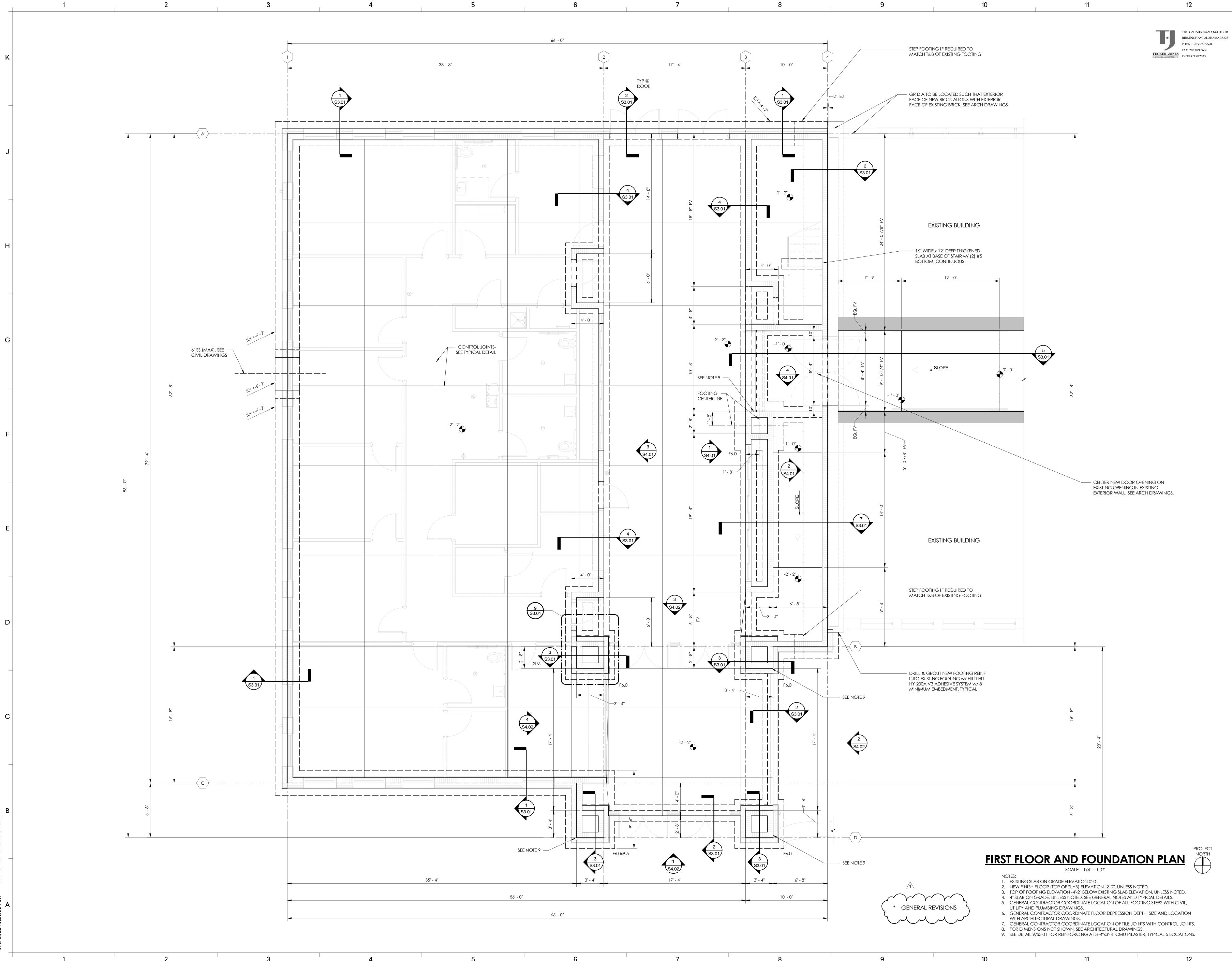


L2x2x1/4 EA SIDE OF JOIST (EXTEND FROM LOAD TO NEAREST PANEL POINT ON OPPOSITE SIDE)	CONCENTRATED LOAD
	<u>CAL JOIST</u> <u>CEMENT DETAIL</u>

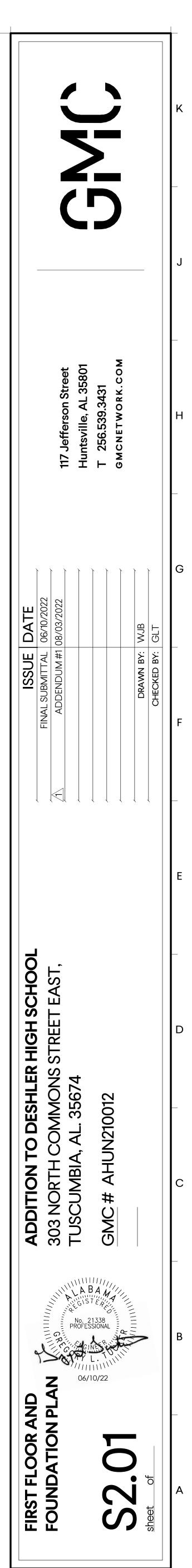
	SONRY W PLICE SCH	
bar Ø	8" WIDE WALLS	12" WIDE WALLS
	f'm = 2000psi	f'm = 2000psi
#4	1 <i>5</i> ''	12"
#5	24"	15"
#6	44"	28"
#7	61"	38"
#8	91"	57"

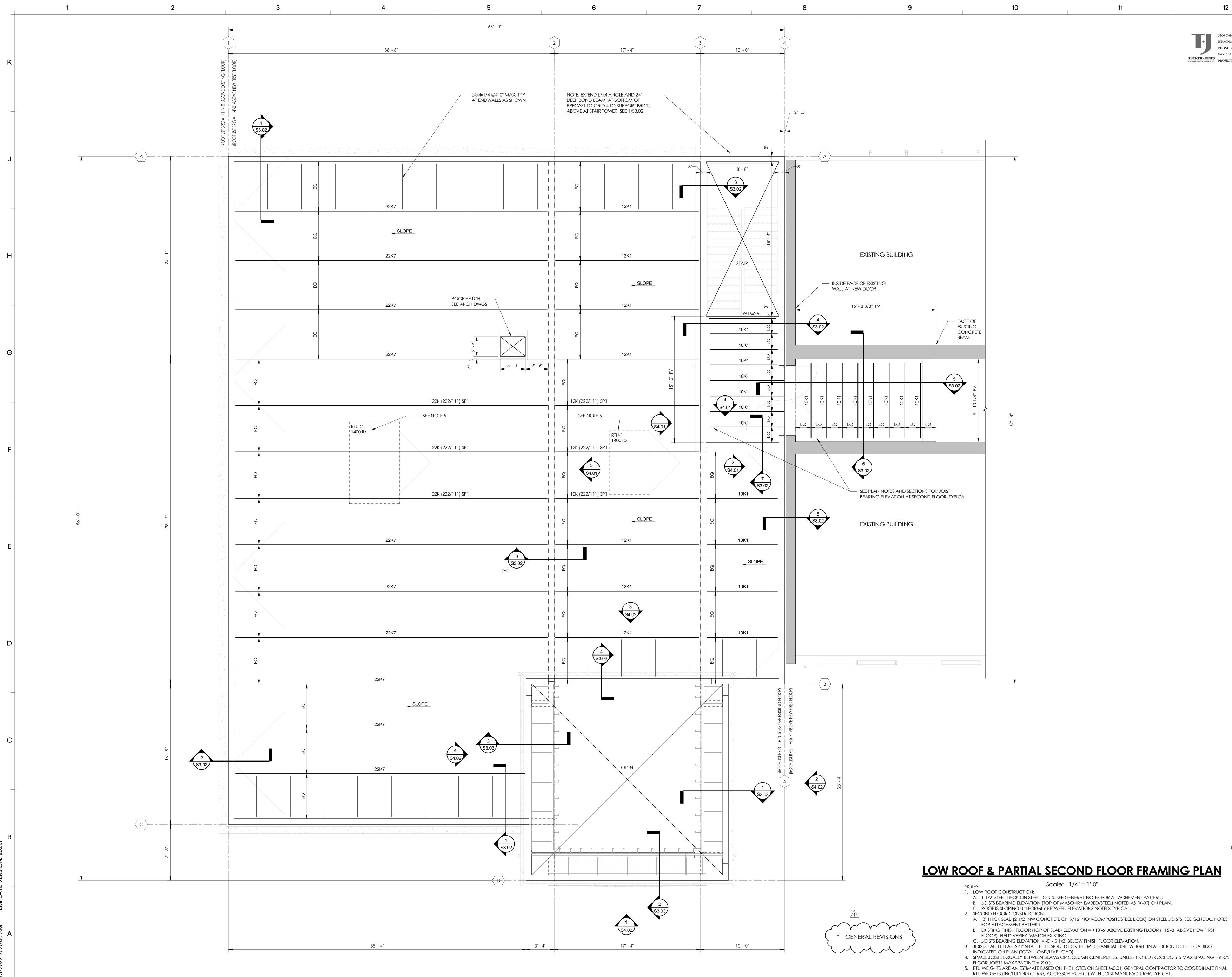
MAXIMUM OPENING	LINTEL	DIMENSIONS & REI	NFORCING
WIDTH	DEPTH	8'' WALL	12" WALL
2'-0''	8	1#4 BOT	1#4 BOT
4'-0''	8	1#4 BOT	2#4 BOT
6'-0''	8	2#5 BOT & 1#4 TOP	2#5 BOT & 2#4 TOP
8'-0''	16	2#5 BOT & 2#4 TOP	2#5 BOT & 2#5 TOP
10'-0''	24	2#7 BOT & 2#5 TOP	2#6 BOT & 2#5 TOP
12'-0''	24	2#7 BOT & 2#5 TOP	2#6 BOT & 2#5 TOP





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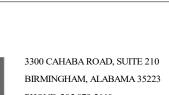




S

10

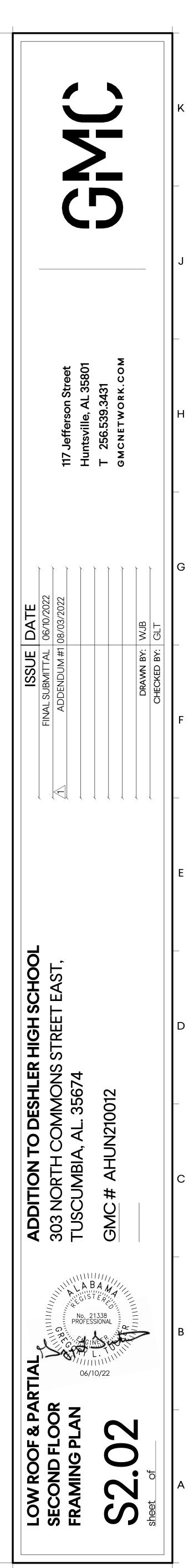




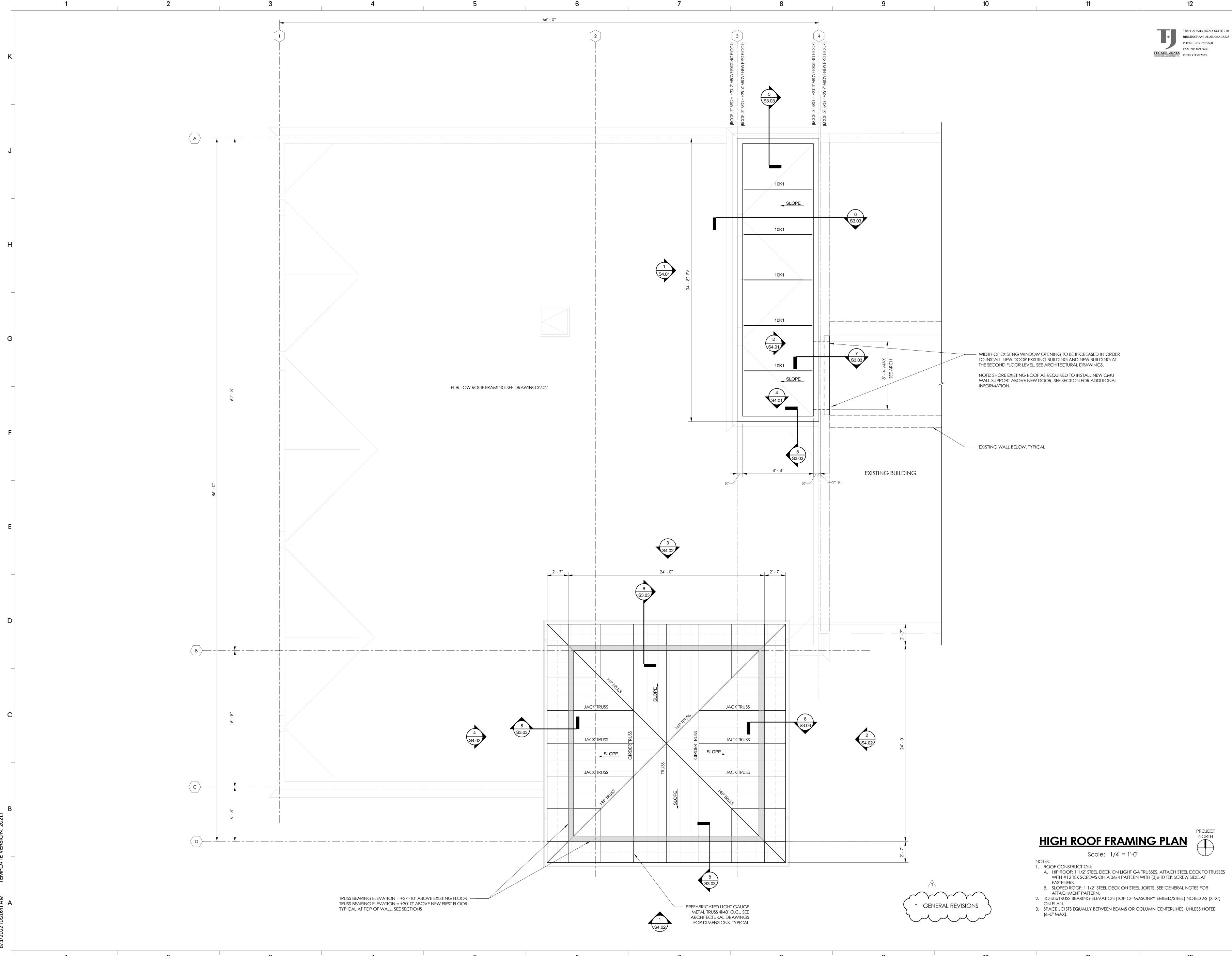
12

PHONE: 205.879.5660 FAX: 205.879.5606 TUCKER · JONES ENGINEERS ASSOCIATED. P.C. PROJECT #22025

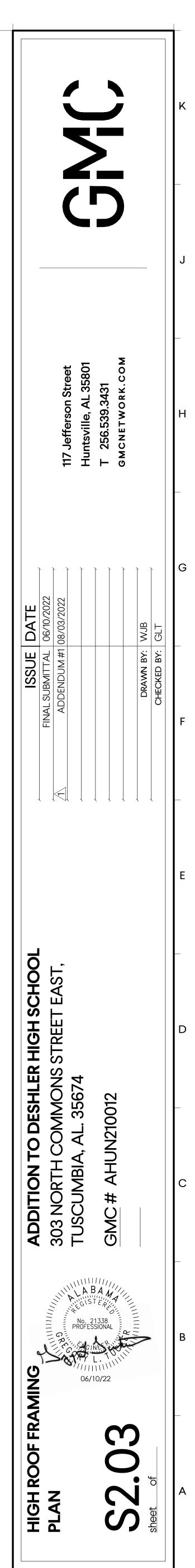
12



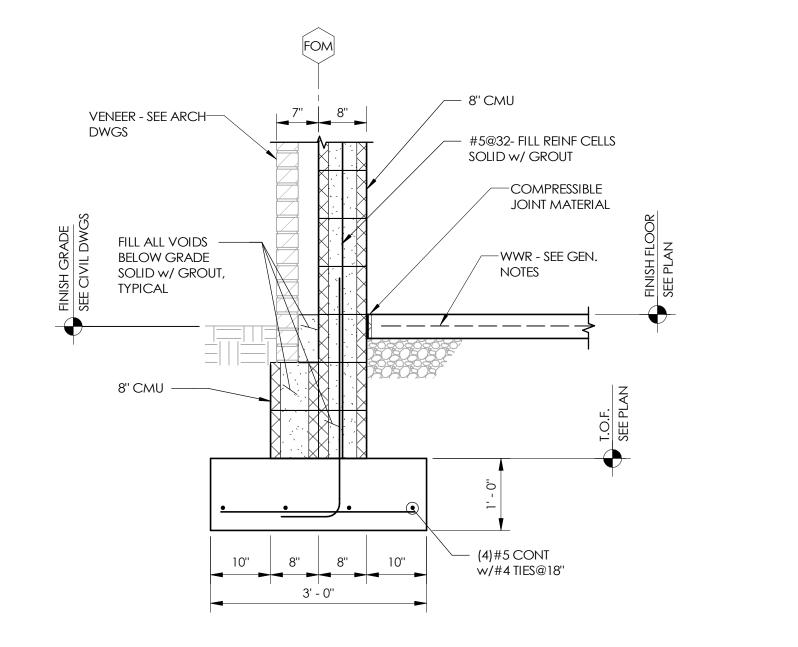
PROJECT NORTH

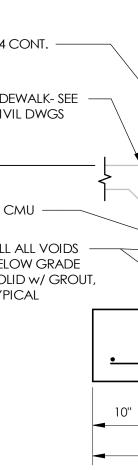


BIRMINGHAM, ALABAMA 35223

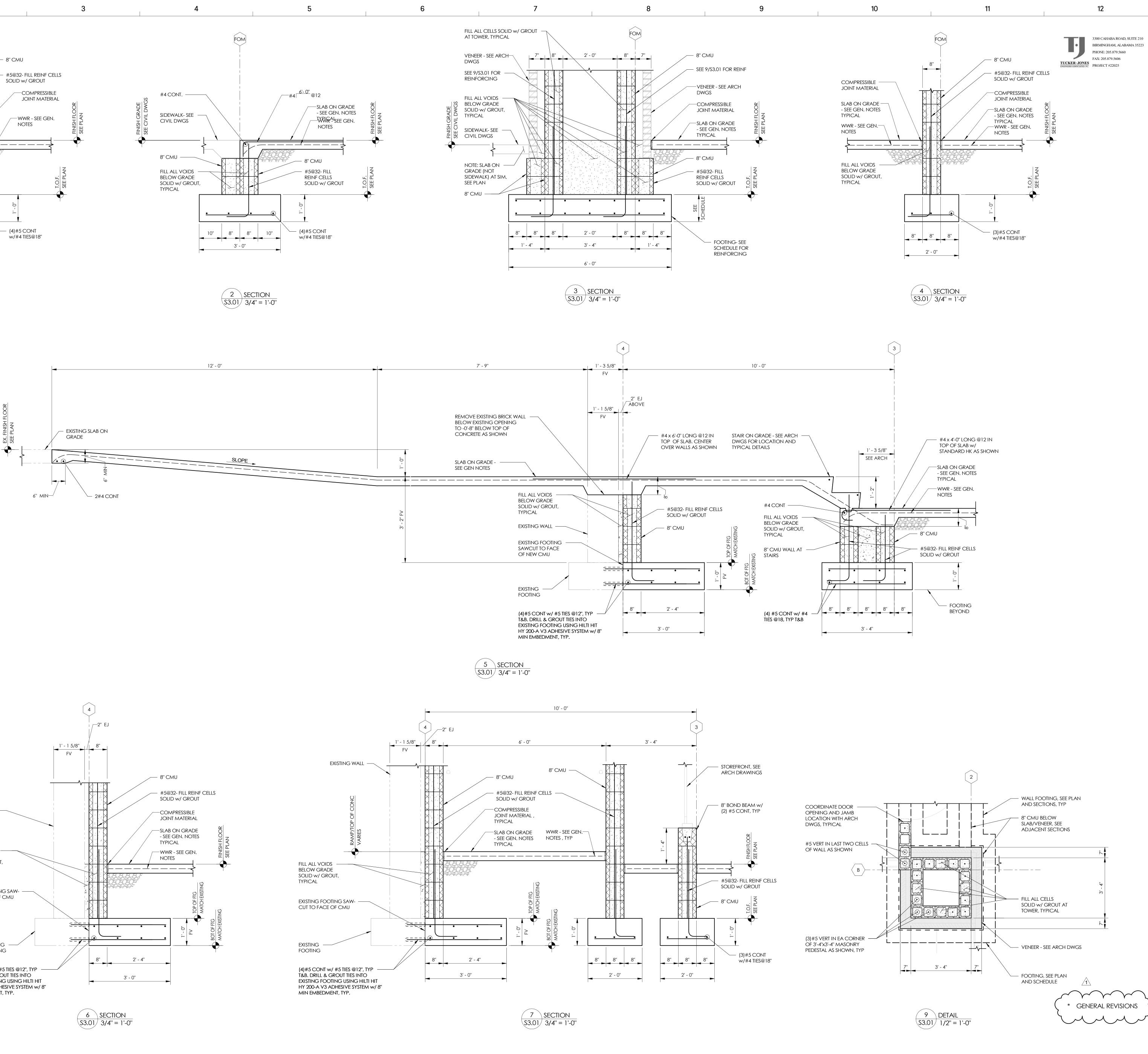


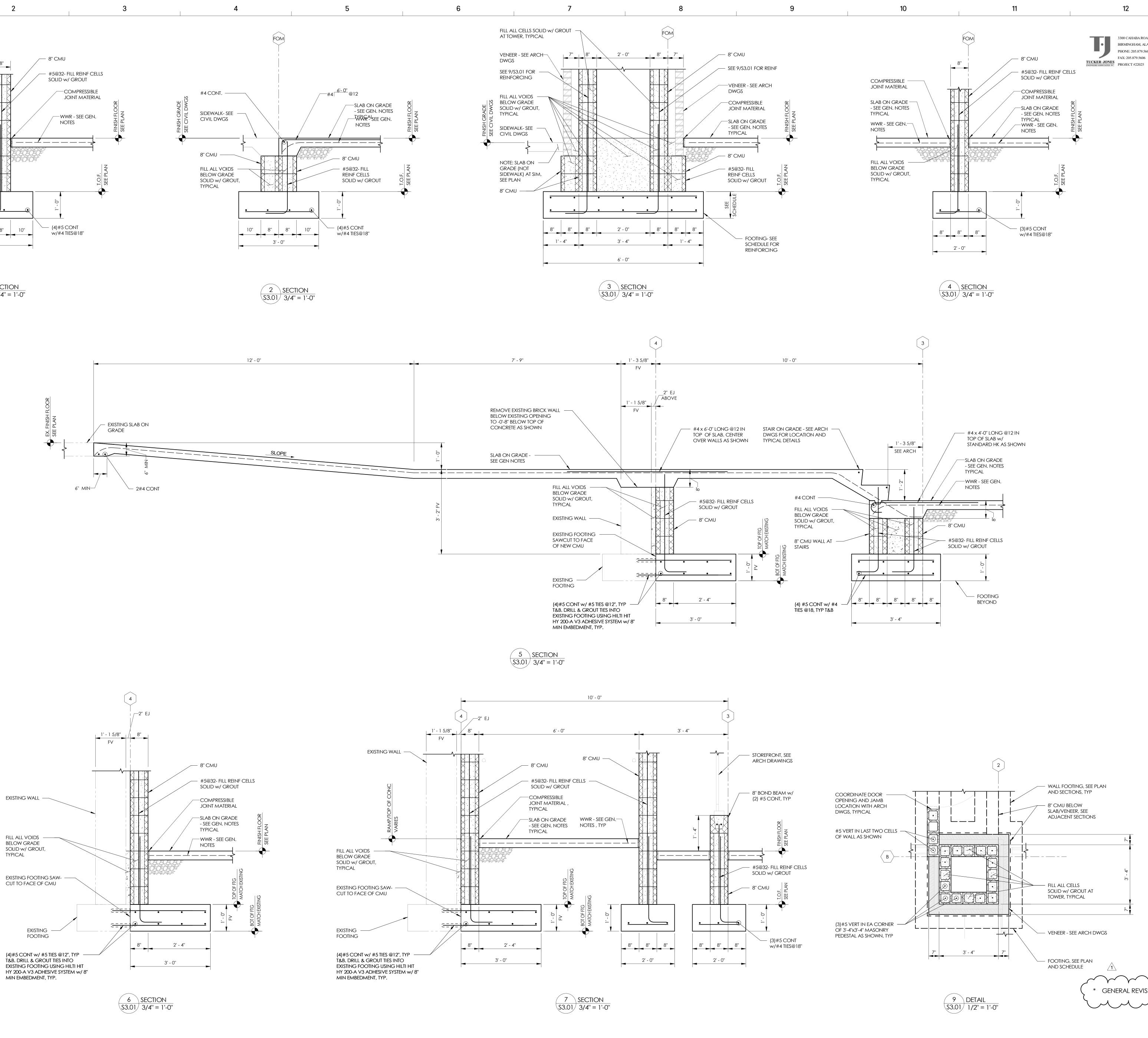
PROJECT NORTH



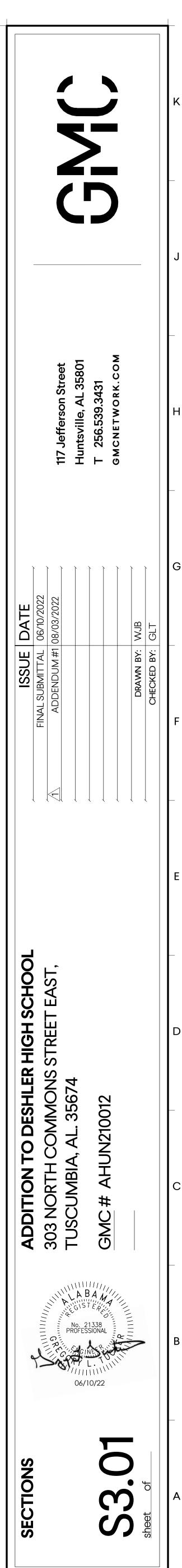


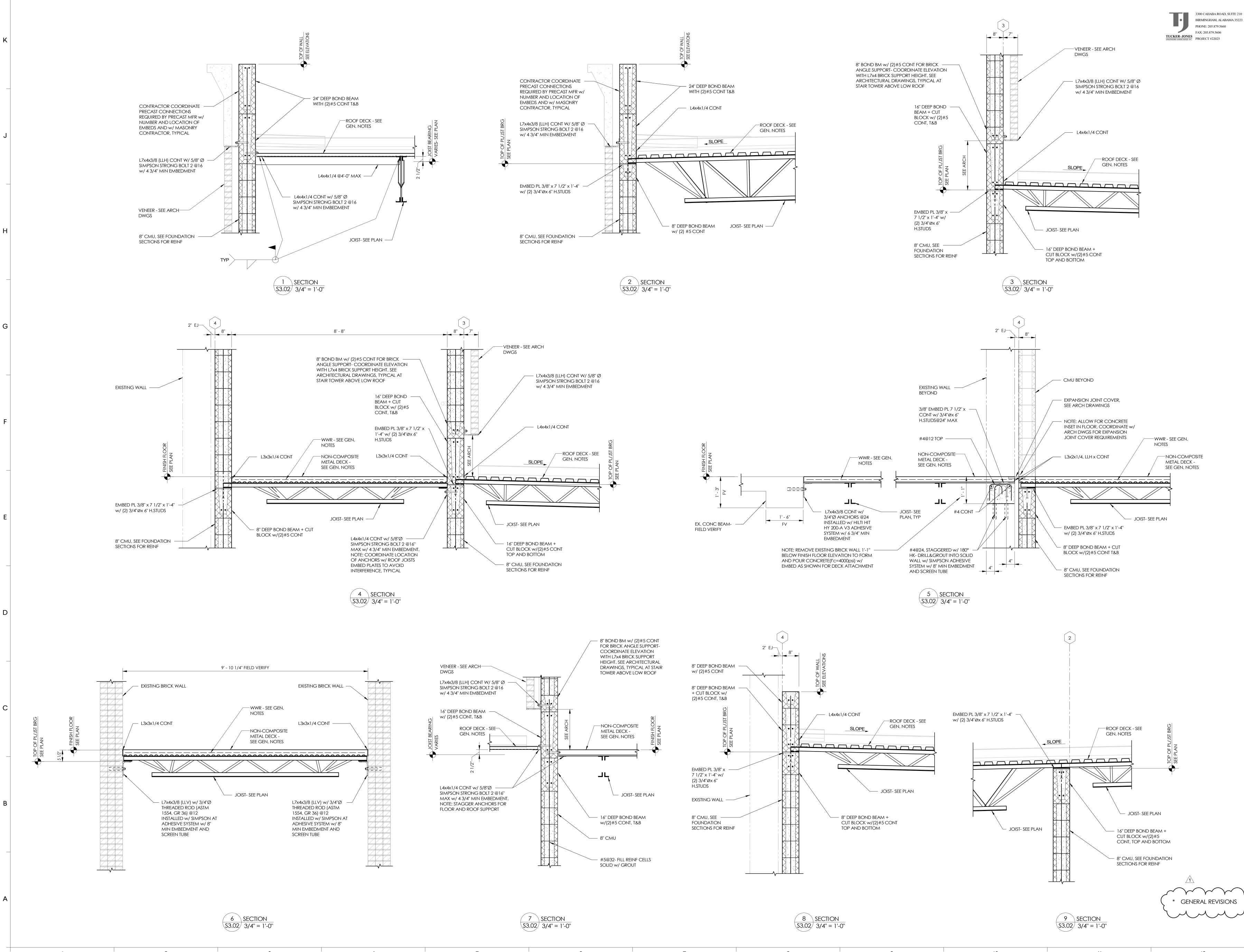
 $\frac{1}{3.01} \frac{\text{SECTION}}{3/4'' = 1'-0''}$



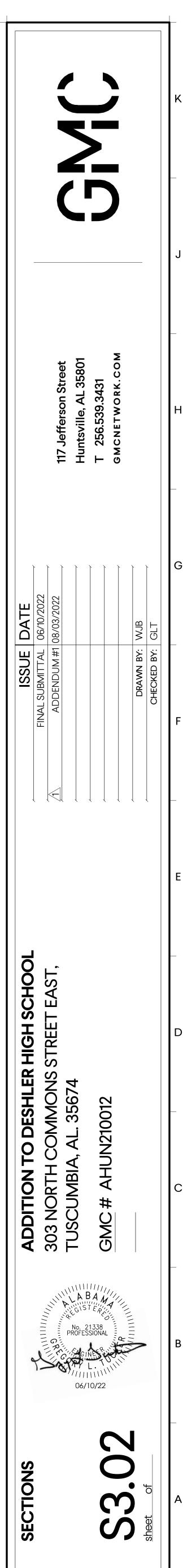


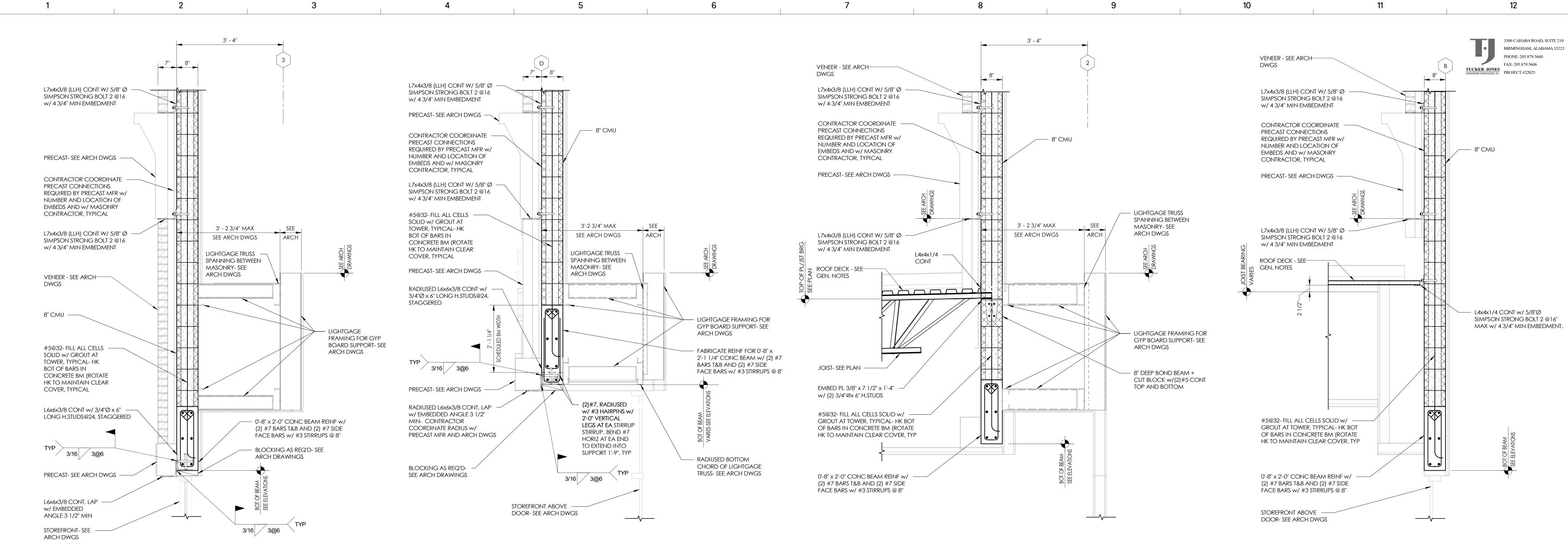




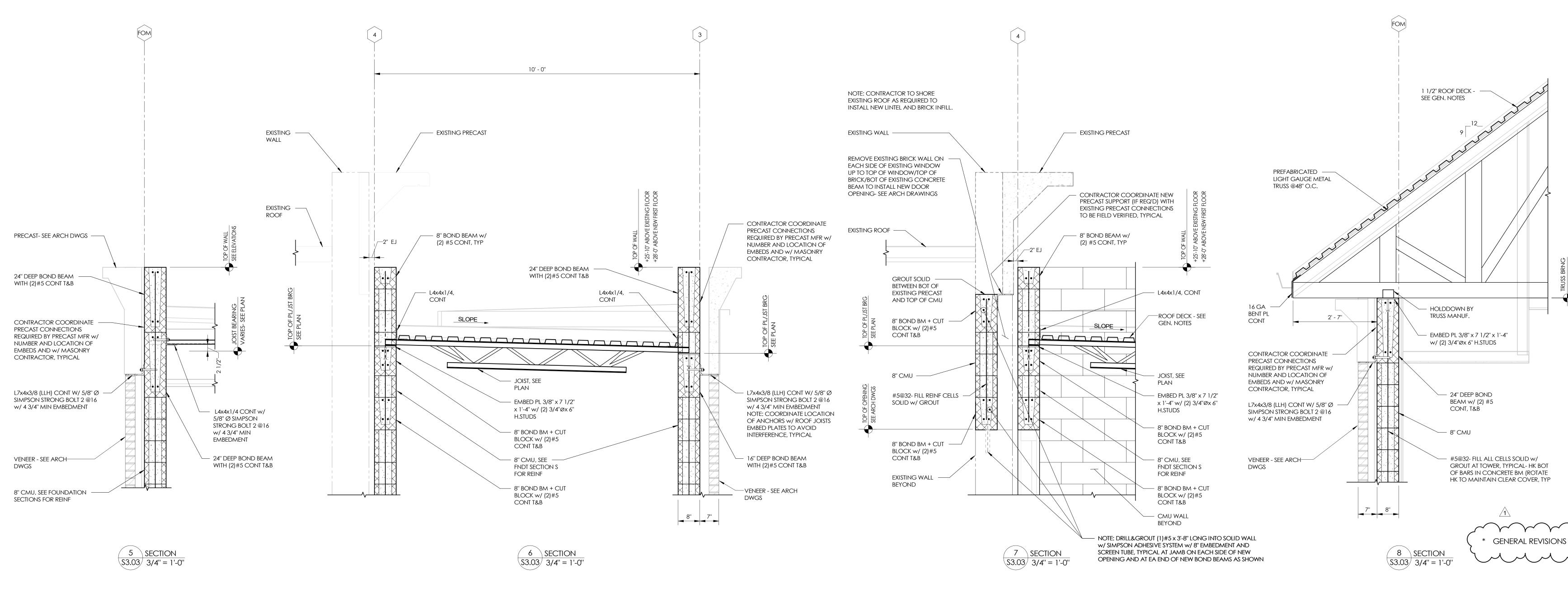


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1 SECTION \$3.03 3/4" = 1'-0"



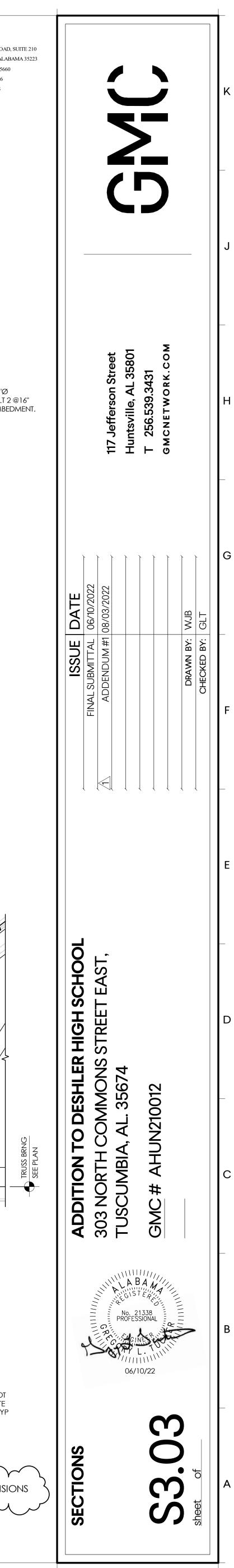
2 SECTION \$3.03 3/4" = 1'-0"

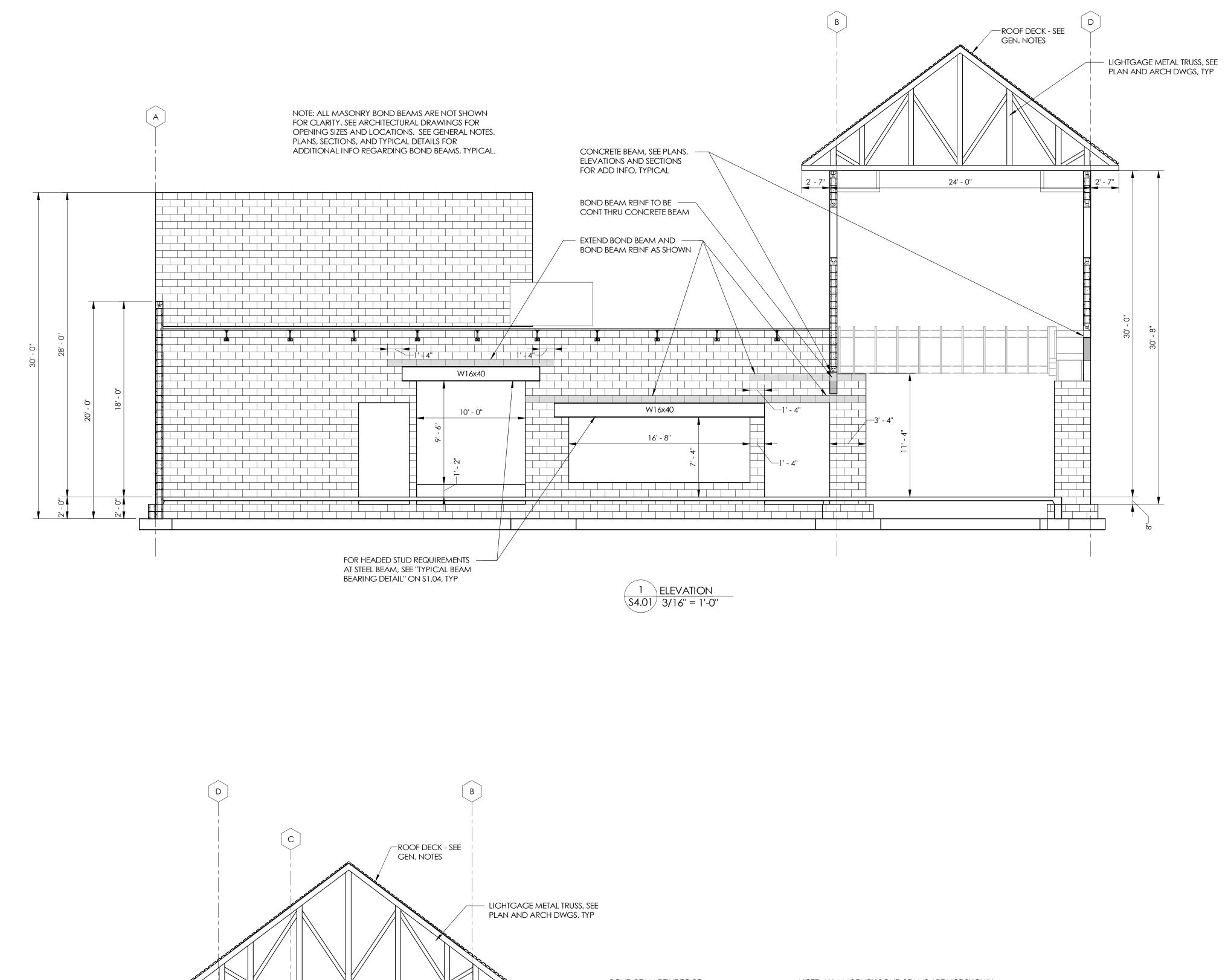
3 SECTION (\$3.03) 3/4" = 1'-0"

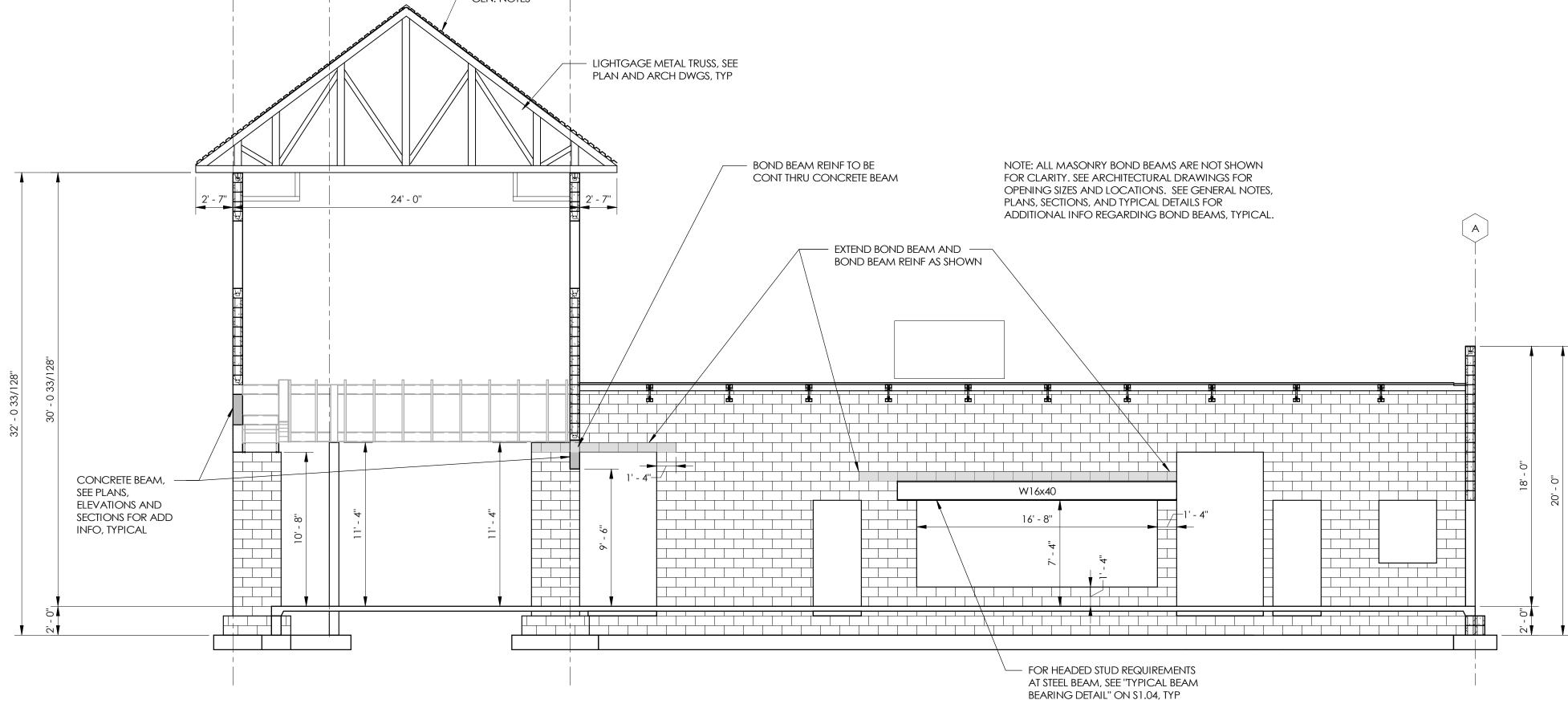
4 SECTION \$3.03 3/4" = 1'-0"

10

11

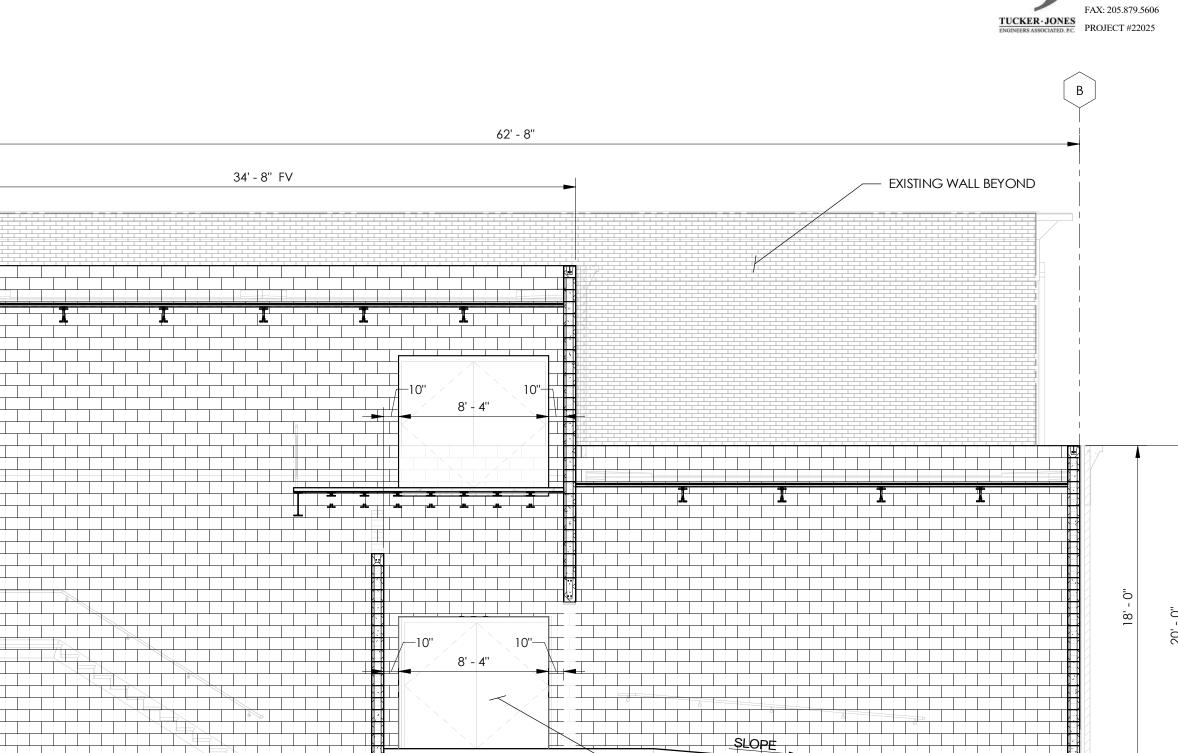






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3 ELEVATION \$4.01 3/16" = 1'-0"



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THICKENED SLAB AT BASE OF STAIRS

I

2 ELEVATION \$4.01 3/16" = 1'-0"

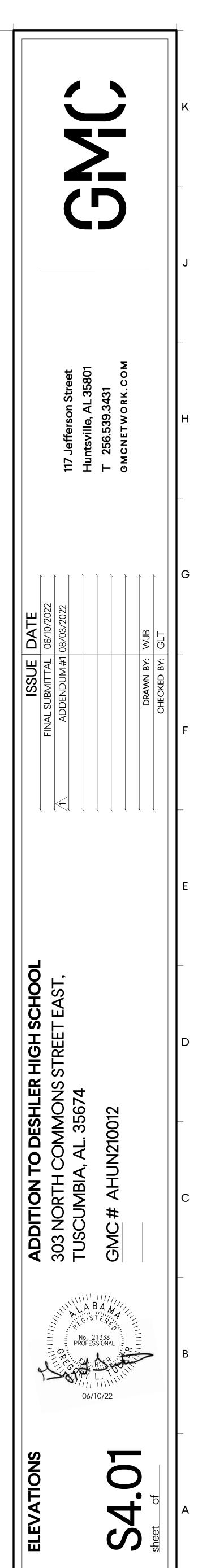
10' - 0'' 2" EJ— S3.03 - ----S3.01

4 ELEVATION \$4.01 3/16" = 1'-0"

* NEW SHEET

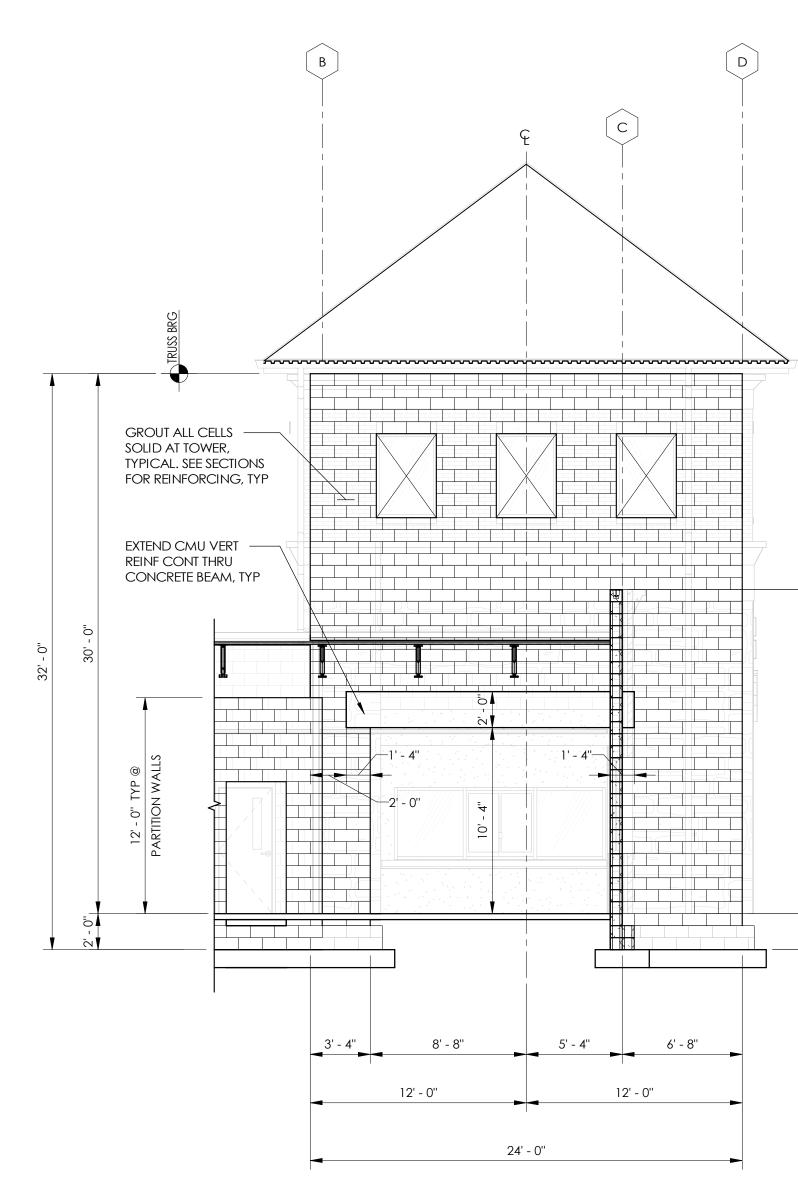
CENTER NEW DOOR OPENING ON EXISTING OPENING IN EXISTING EXTERIOR WALL, SEE ARCH DRAWINGS.





GROUT ALL CELLS SOLID AT TOWER, TYPICAL. SEE SECTIONS FOR REINFORCING, TYP EXTEND CMU VERT REINF CONT THRU CONCRETE BEAM, TYP - --1 5/8" 1 5/8"— 1' - 4''— 2' - 0''— —1'-4" Ō 3' - 4" 8' - 8" 8' - 8" 3' - 4" 12' - 0'' 12' - 0'' 24' - 0''

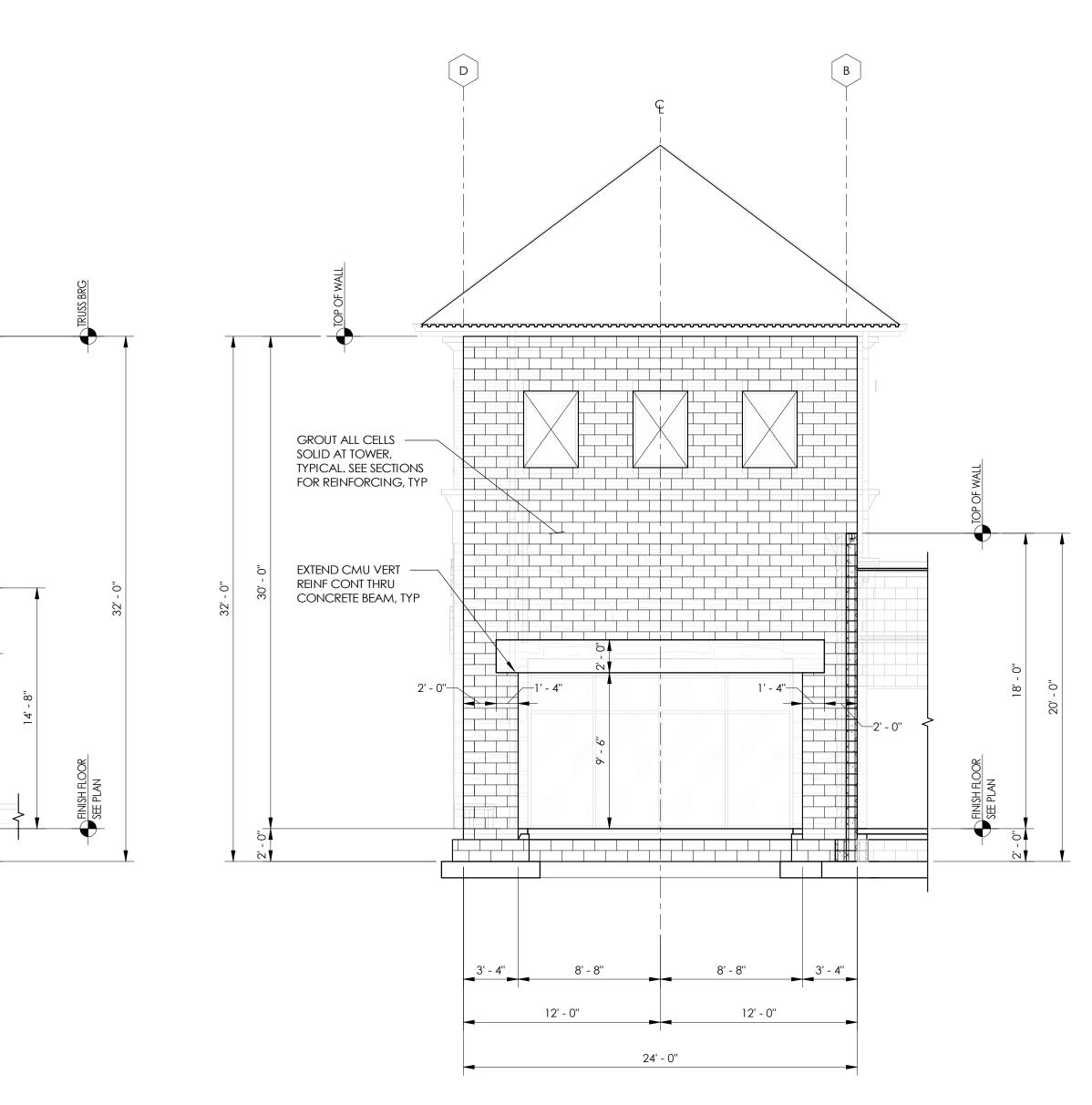
1 ELEVATION \$4.02 3/16" = 1'-0"



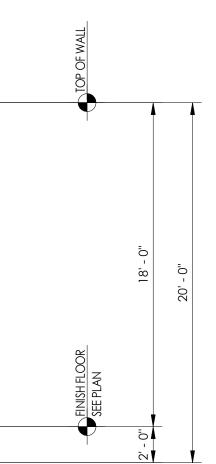
2

4 ELEVATION \$4.02 3/16" = 1'-0"

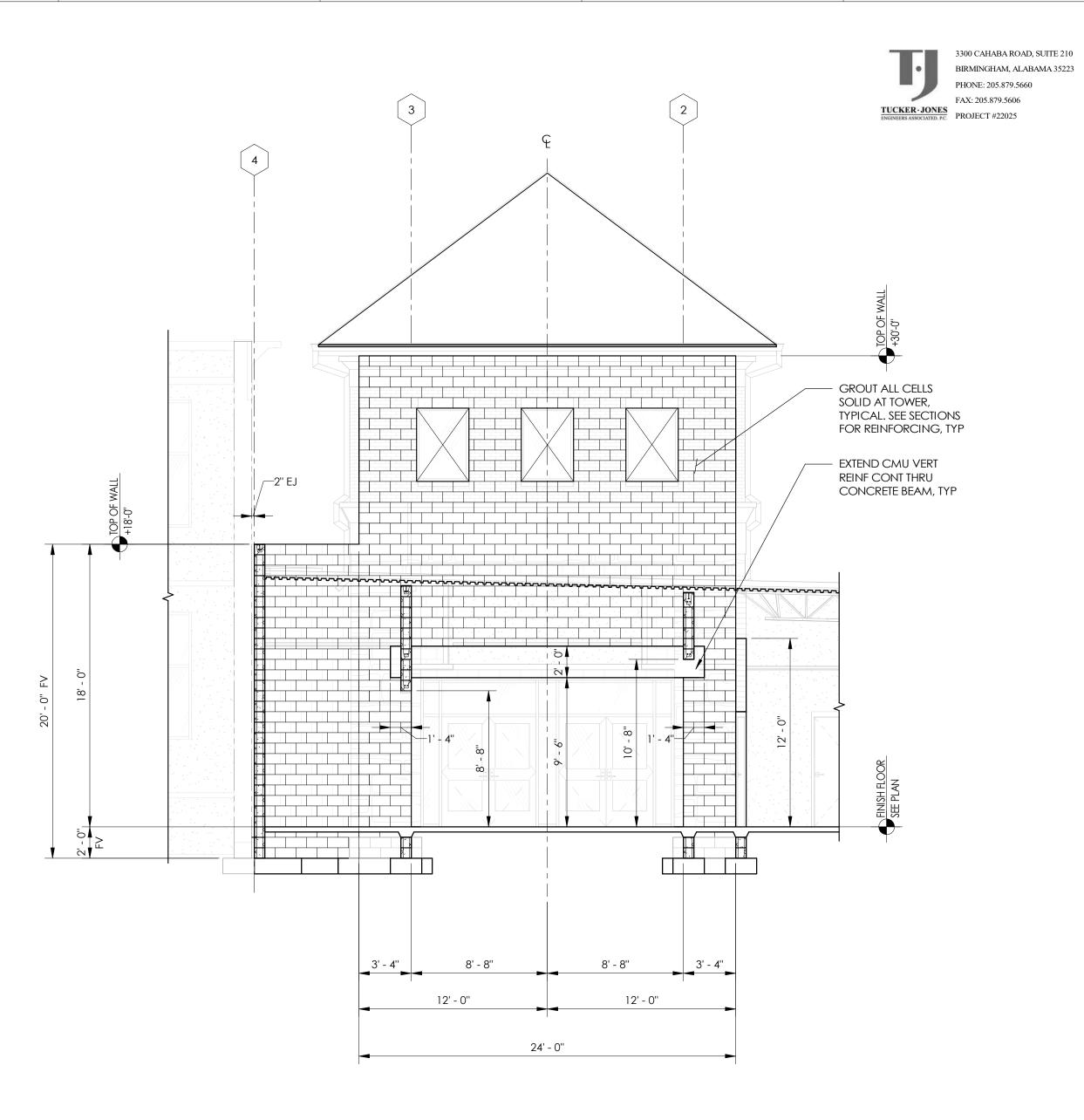
4



2 ELEVATION \$4.02 3/16" = 1'-0"



5



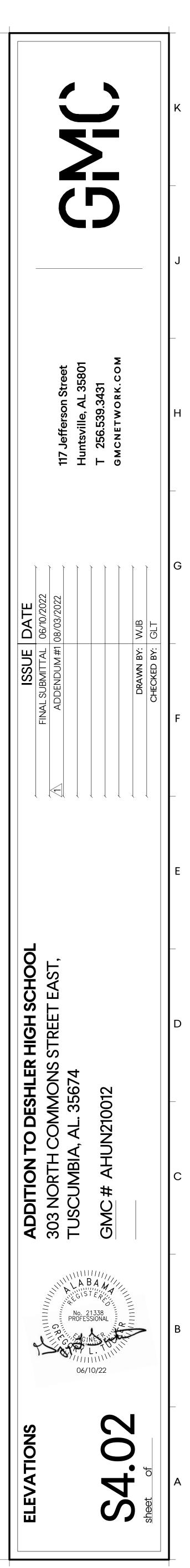
10

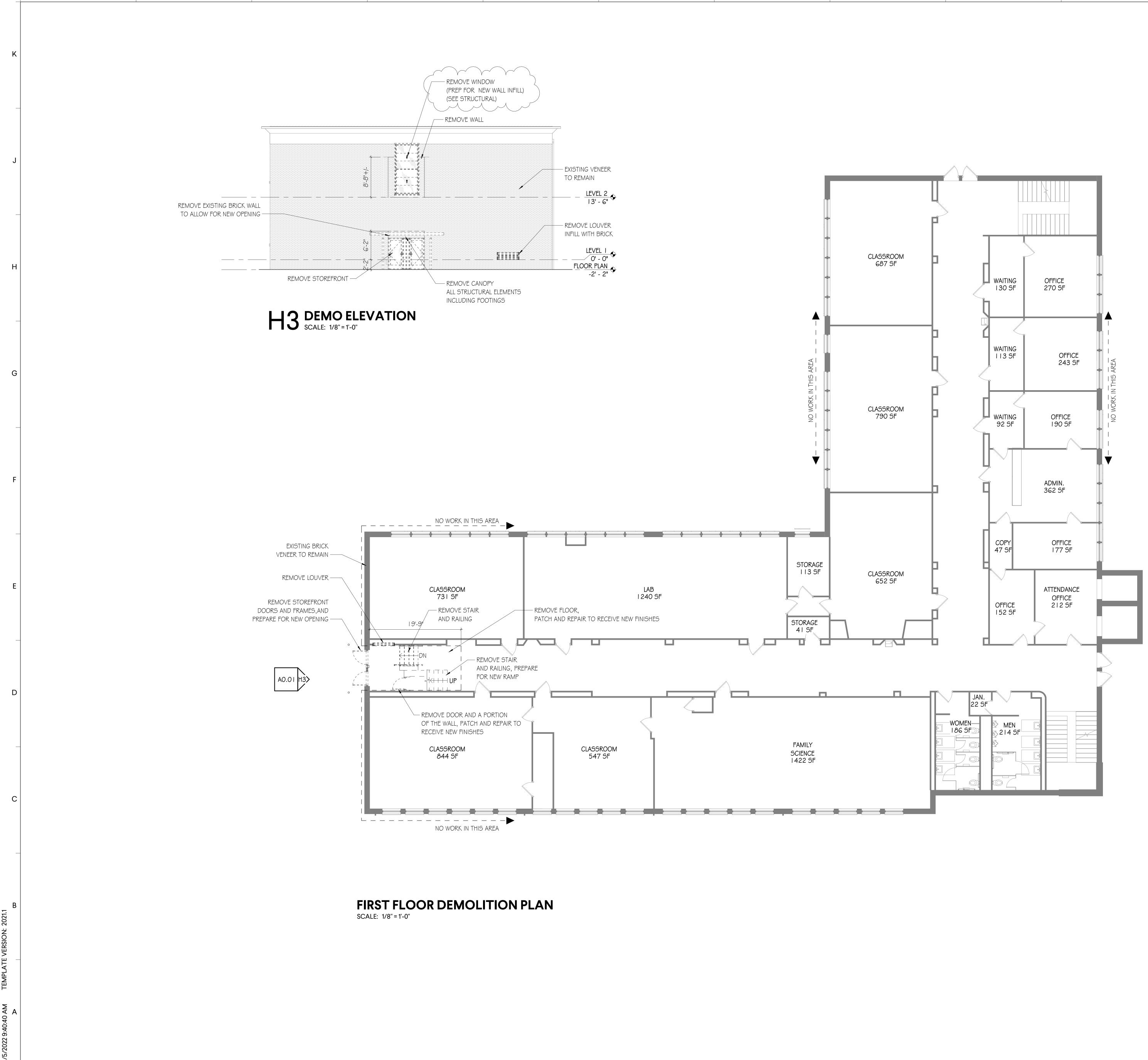
3 ELEVATION \$4.02 3/16" = 1'-0"

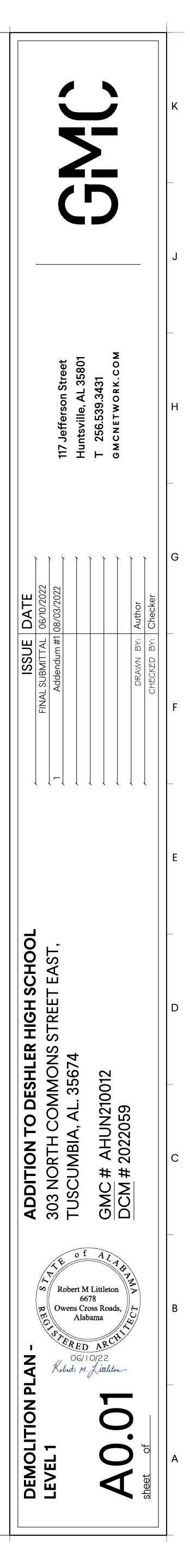
* NEW SHEET

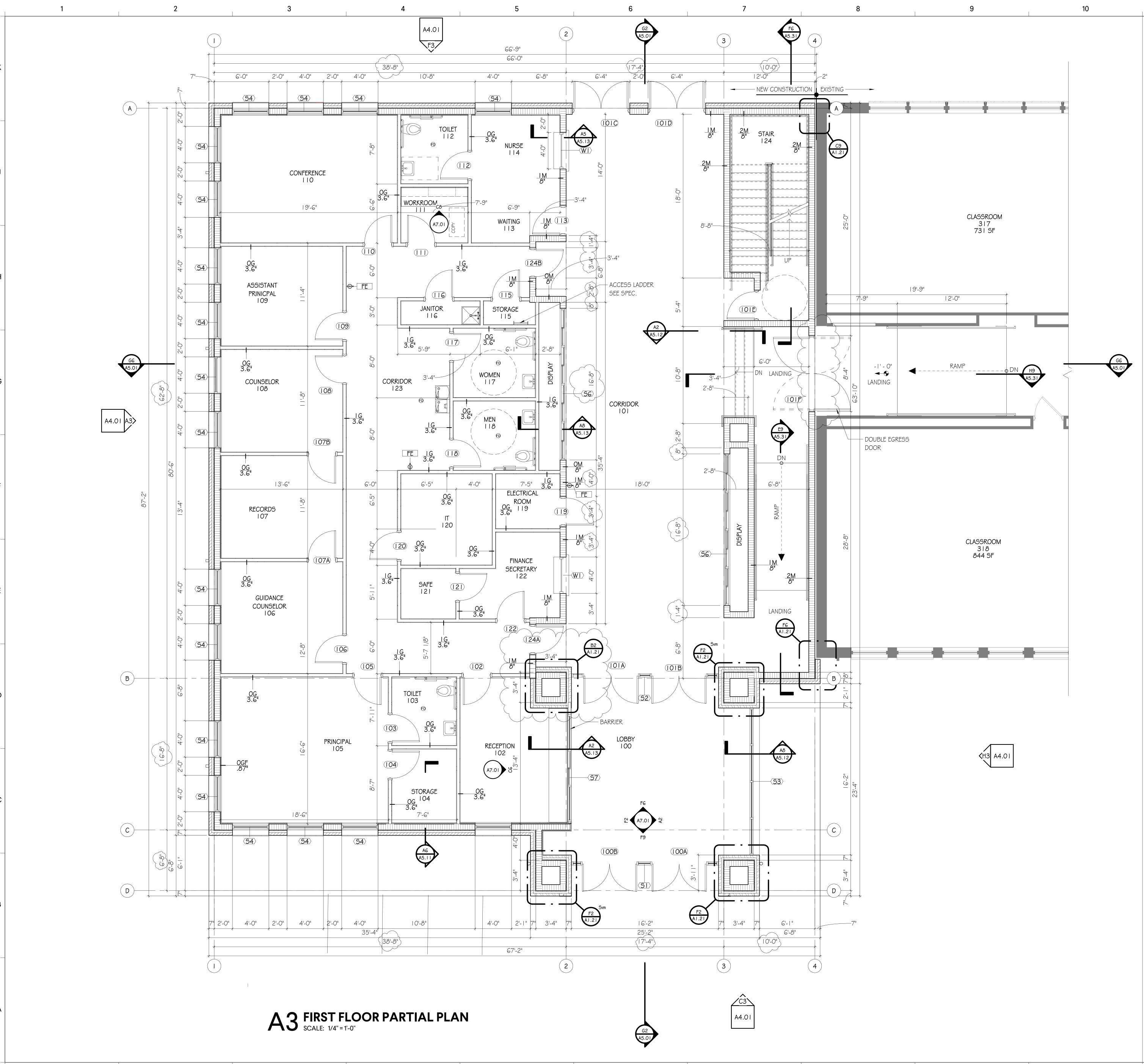
11

12

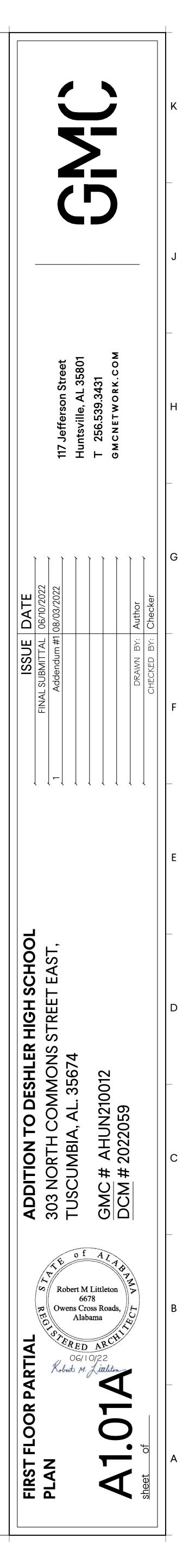


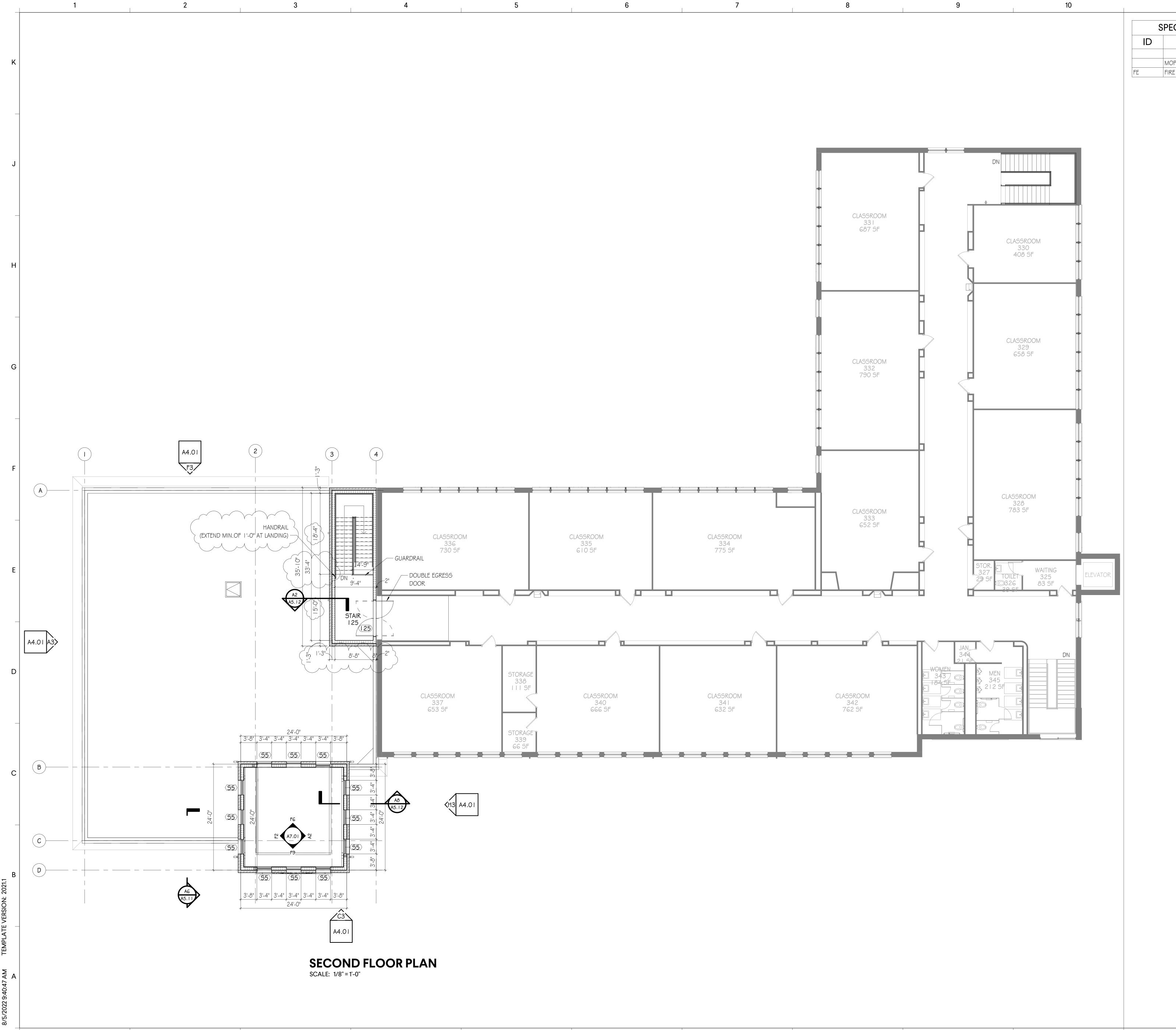






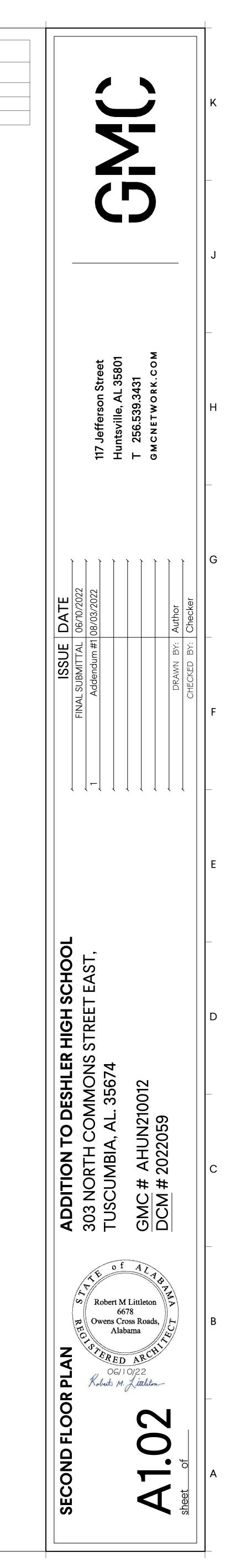
022 9:40:44 AM TEMPLATE VERSION: 2021.1 ይ

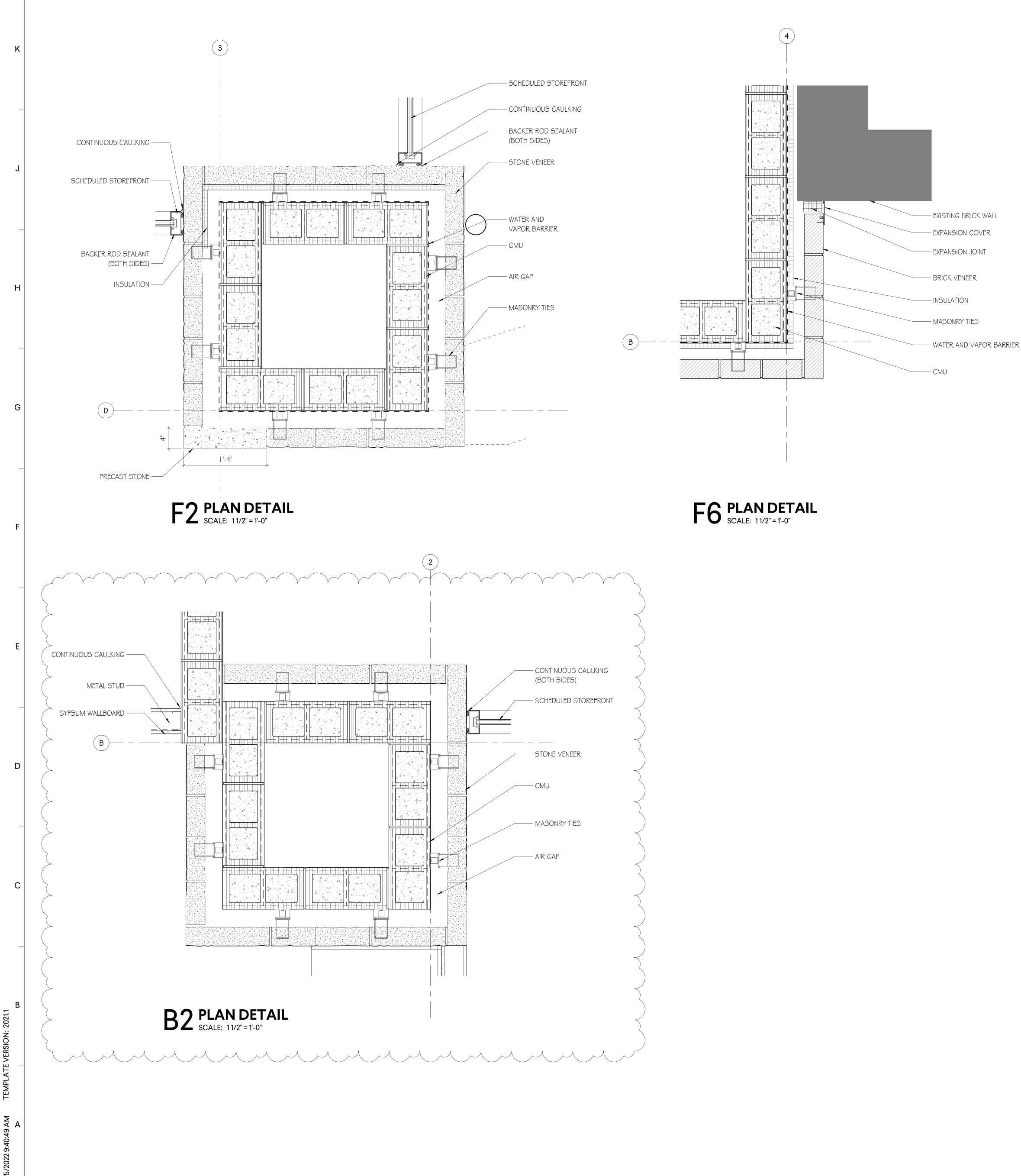


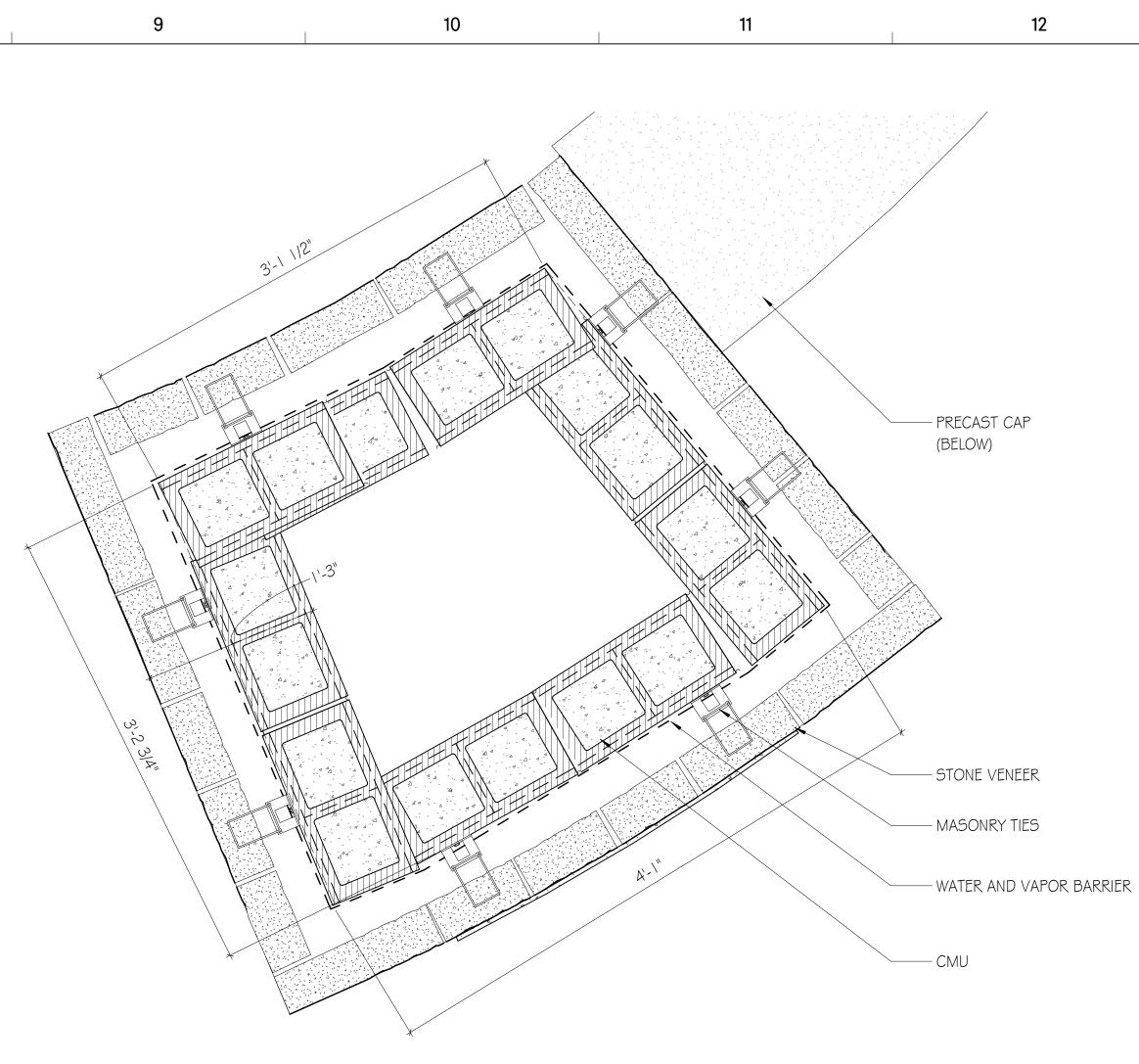


SPECIALTY EQUIPMENT SCHEDULE DESCRIPTION MOP SINK FIRE EXTINGUISHER

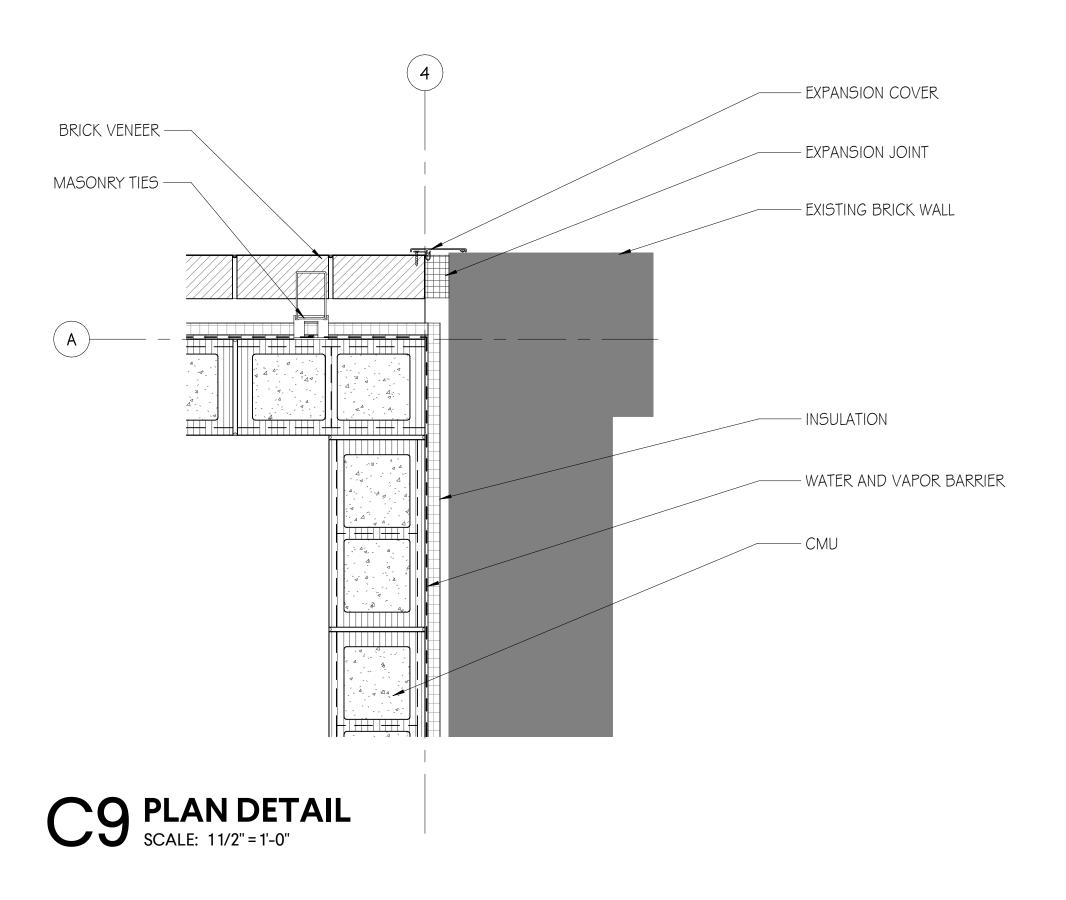
g

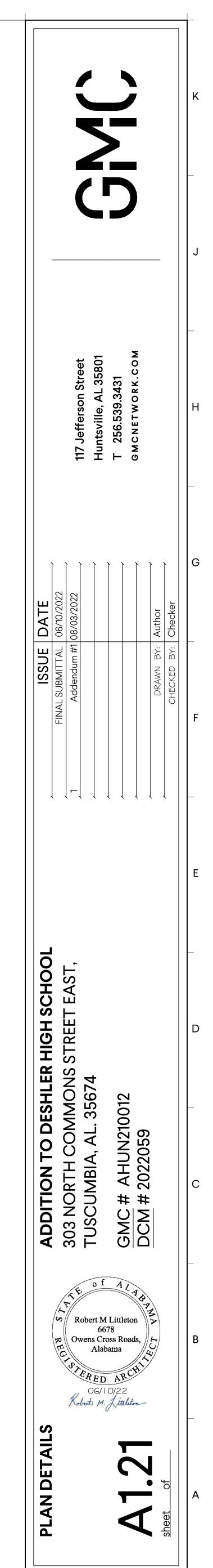










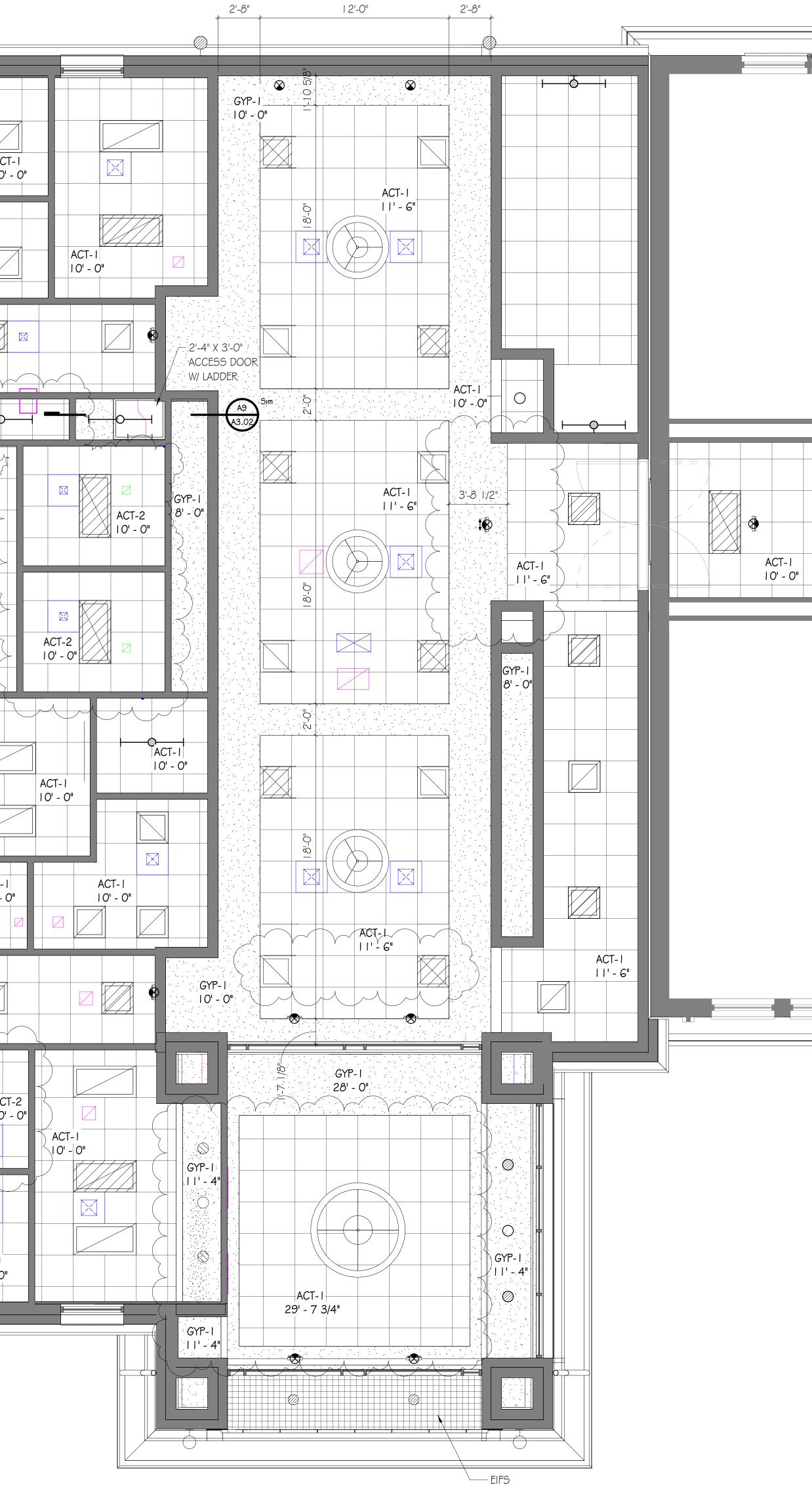


ACT-1 10' - 0"			AC ¹ 10' ACT-1 10' - 0"
ACT-1 10' - 0"		ACT-2 10' - 0"	
ACT-1 10' - 0"			
ACT-1 10' - 0"		ACT-1 10' - 0"	
ACT-1 10' - 0"			ACT- 10'-
	xCT-1 0' - 0"		AC 10 2



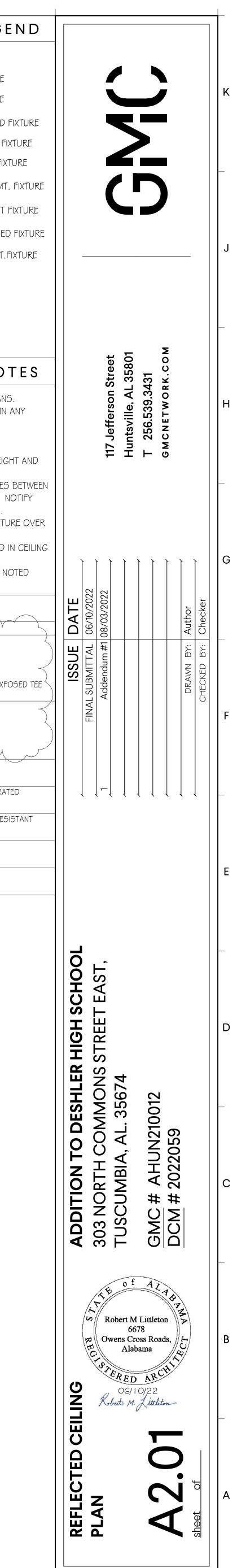


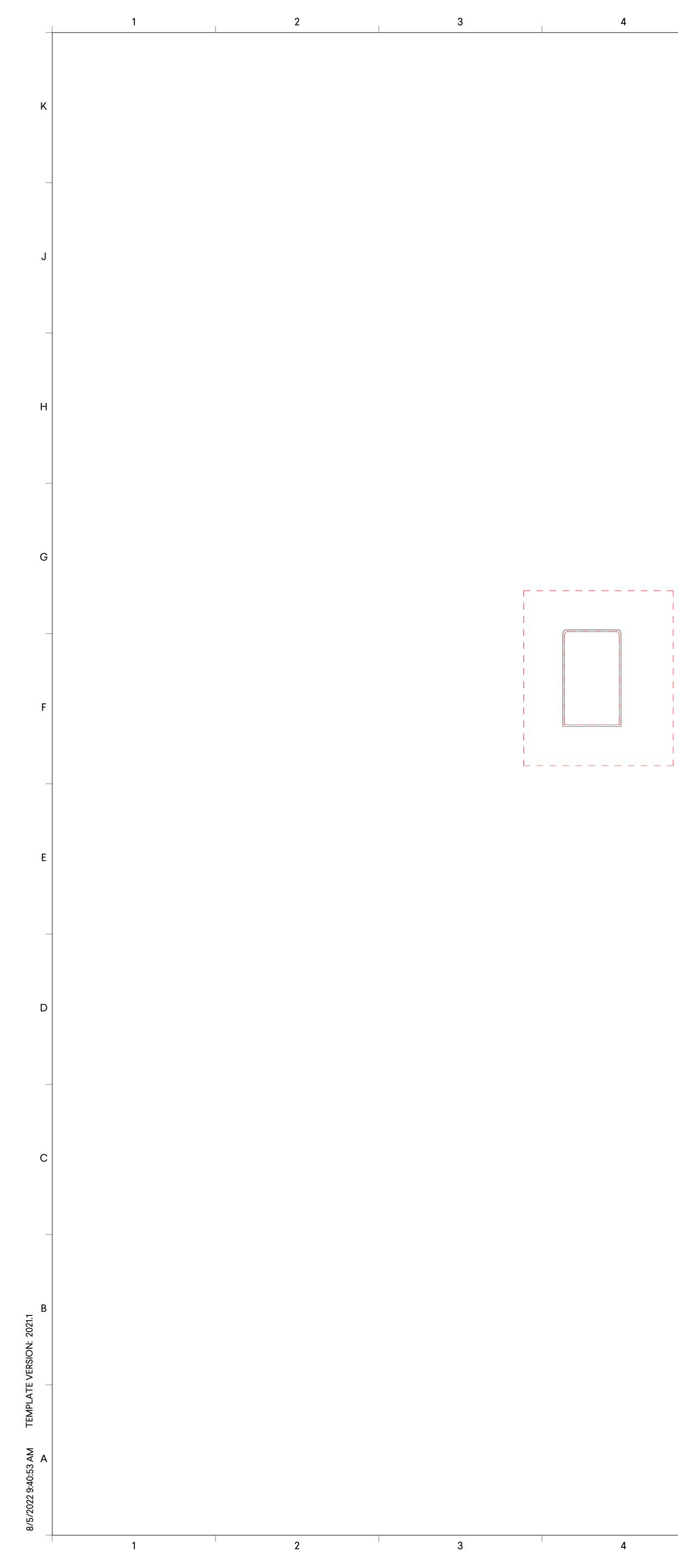
¥ A

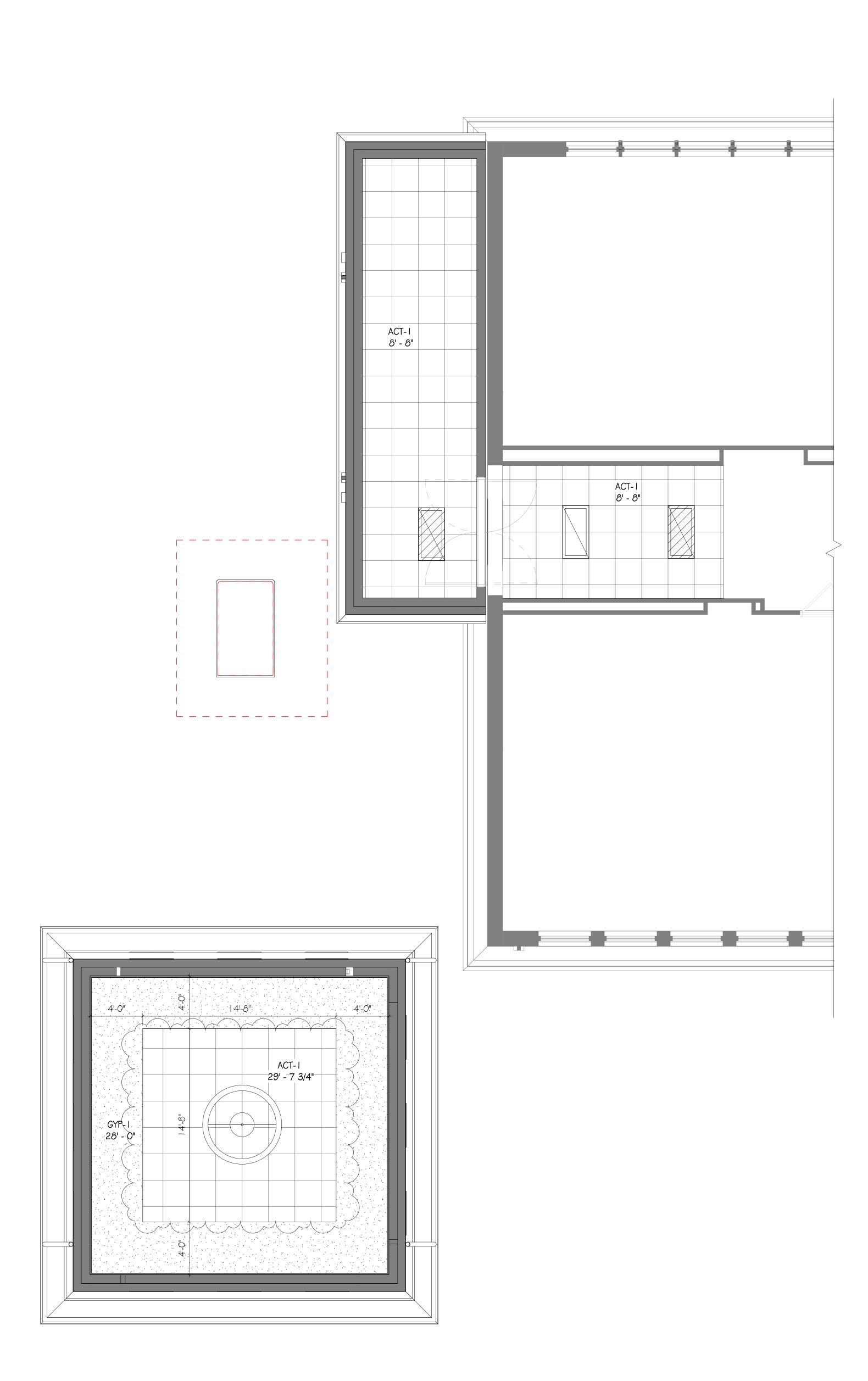


RCP – FIRST FLOOR PARTIAL PLAN SCALE: 1/4" = 1'-0"

9	10		11			12
		REF	LECTED	CEILII	NG F	PLAN LEGEN
		CEILING	FINISHES:		<u>LIGHTING</u>	<u>;</u>
			LAY-IN ACOUSTICAL CEILING SYSTEM - 2 LAY-IN ACOUSTICAL CEILING SYSTEM - 2 GYP BOARD - INTER EIFS SOFFIT - EXTER EXPOSED STRUCTUR	''X2' ''X4' IOR KIOR	 	2X2 LAY-IN FIXTURE 2X4 LAY-IN FIXTURE LINEAR SUSPENDED FIXTURE LINEAR RECESSED FIXTURE LINEAR WALL MT. FIXTURE
			2X2 LAY-IN METAL CEILING SYSTEM 2X2 LAY-IN WOOD CEILING SYSTEM RECESSED LIGHT PC <u>NICAL:</u> SUPPLY DIFFUSER RETURN AIR GRILL EXHAUST FAN	, X		CIRCULAR SURF. MT. FIXTU CIRCULAR PENDANT FIXTUR CIRCULAR RECESSED FIXTU CIRCULAR WALL MT.FIXTUR EXIT LIGHT NURSE CALL LIGHT
		RE	FLECTED	CEIL	ING	PLAN NOTE
		2. WHE DIRI 3. SEE 4. SEE 5. SEE LOC 6. COC 7. WHE DOC 8. ALL TILE 9. ALL	EREVER POSSIBLE N ECTION. ELECTRICAL FOR AL MECHANICAL FOR A INTERIOR ELEVATIO CATIONS. ORDINATE LOCATION ORDINATE LOCATION OR CP AND MECHAN CHITECT OF ANY DISC ERE EXIT SIGNS ARE OR BUT MAINTAIN M SPRINKLER HEADS I	O CEILING TIL L LIGHT FIXTL ALL DIFFUSER NS FOR WALI IS OF ALL LIG ICAL, FIRE PR CREPANCIES LOCATED AB INIMUM OVEI N ACOUSTIC	E SHOULI JRE TYPES TYPES AI MOUNTE HTS, DIFF OTECTION FOUND B OVE DOC RHEAD CL CEILINGS	ND SIZES. ED LIGHT FIXTURE HEIGHT AN EUSERS, AND DEVICES BETV N, AND ELECTRICAL. NOTIFY EFORE PROCEEDING. DRWAYS, CENTER FIXTURE O
			CEILIN	IG FIN	IISH	LEGEND
			TYPE ACOUSTICAL CEILING TILE SYSTEM ACOUSTICAL CEILING TILE SYSTEM	STYLE: FINE I COLOR: WHI SIZE: 24" X 2	RER: ARMS FISSURED 24" X 5/8" N SYSTEM: TE RER: USG TROCK 32 TE 24" X 1/2"	
		GYP-1	GYP BOARD CEILING	PAINTED GYP COLOR: PNT-		
		GYP-2	GYP BOARD CEILING	COLOR: PNT-	-XXX (U.N.(
		MGB-1	MOISTURE RESISTANT GYP BOARD CEILING	PAINTED GYP COLOR: PNT-		CEILING - MOISTURE RESISTAN O. ON RCP)
		EXP-1	EXPOSED TO STRUCTURE			RE - WITH NO FINISH
		EXP-2	EXPOSED TO STRUCTURE			RE – WITH FINISH. EMENTS COLOR: XXX

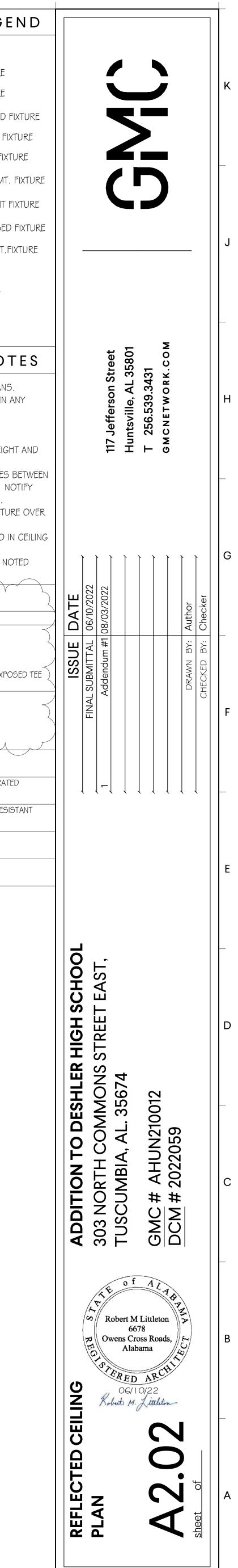


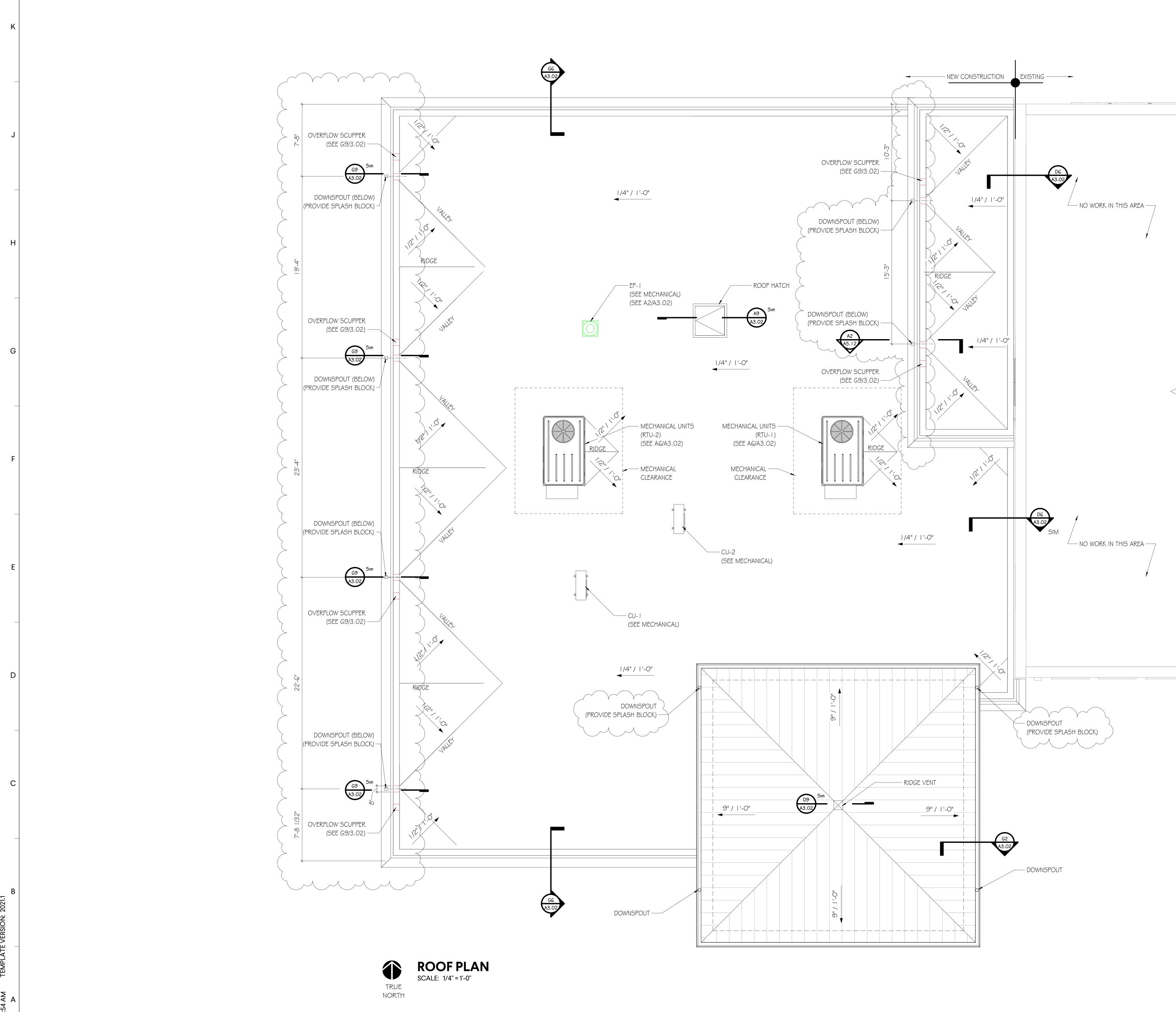


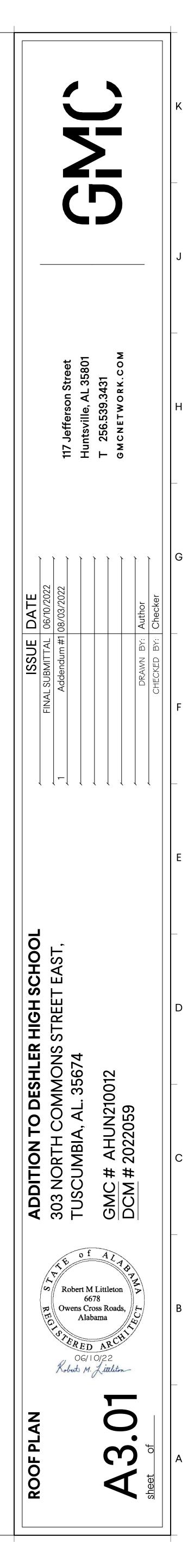


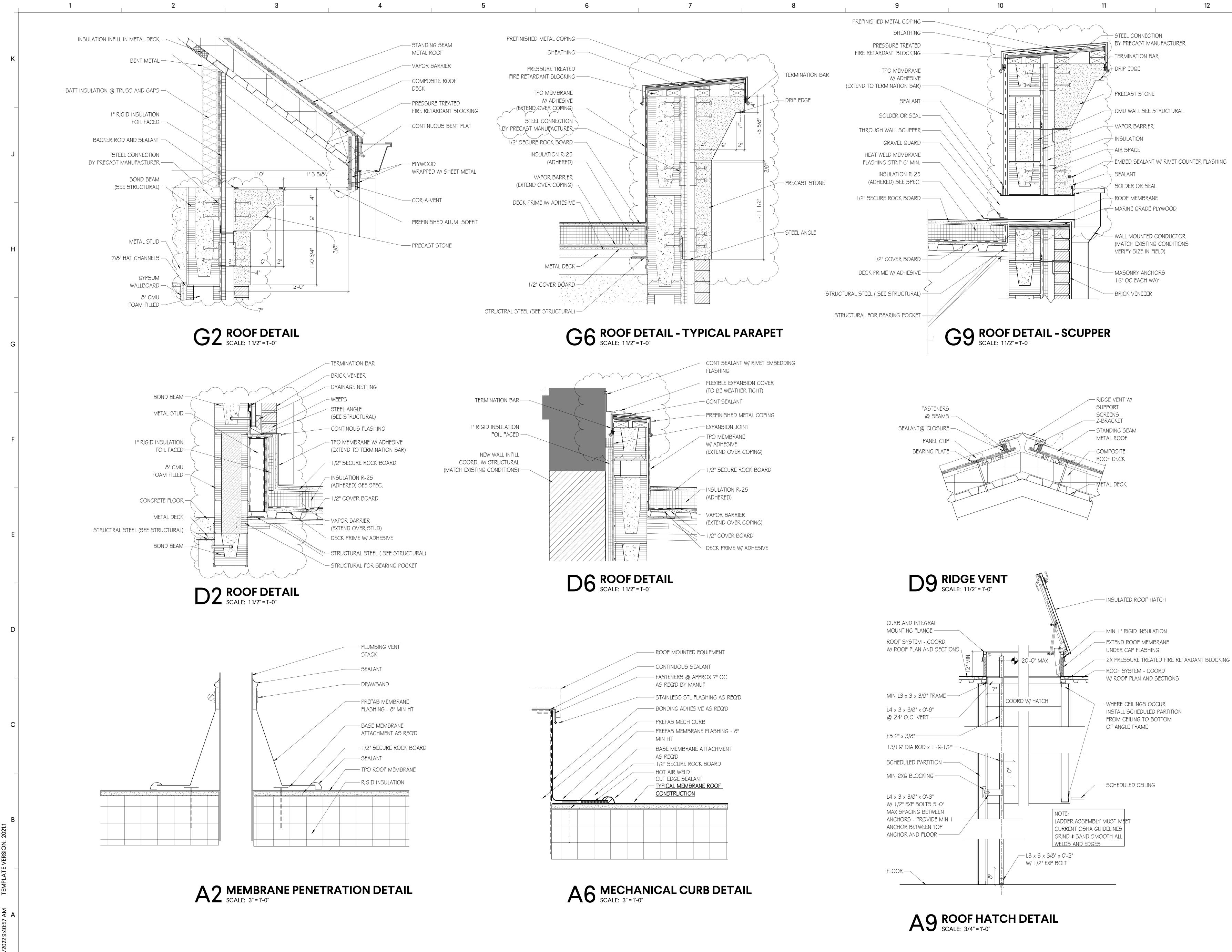
RCP – SECOND FLOOR PARTIAL PLAN SCALE: 1/4" = 1'-0"

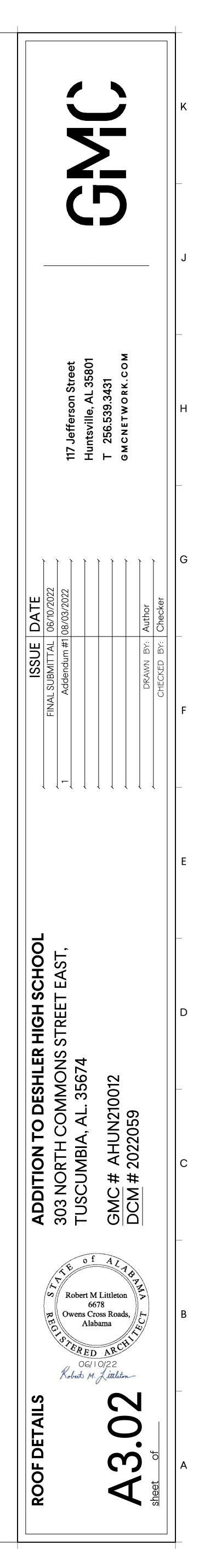
	11			_
REF	LECTED	CEILI	NGF	PLAN LEGE
<u>CEILIN</u>	<u>G FINISHES:</u>		LIGHTING	<u>G:</u>
	LAY-IN ACOUSTICAL			2X2 LAY-IN FIXTURE
				2X4 LAY-IN FIXTURE
	CEILING SYSTEM - 2		0 0	LINEAR SUSPENDED FI.
				LINEAR RECESSED FIXT
	EXPOSED STRUCTU	RE		LINEAR WALL MT. FIXTL
	2X2 LAY-IN METAL CEILING SYSTEM		\bigcirc	CIRCULAR SURF. MT. 1
	2X2 LAY-IN WOOD		٥	CIRCULAR PENDANT FI
	CEILING SYSTEM	OCKET	\oslash	CIRCULAR RECESSED I
			9	CIRCULAR WALL MT.FI
MECHA	ANICAL:		\bigotimes	EXIT LIGHT
	SUPPLY DIFFUSER		NC	NURSE CALL LIGHT
	RETURN AIR GRILL	E		
	EXHAUST FAN			
R E	FLECTED) CEIL	ING	PLAN NOT
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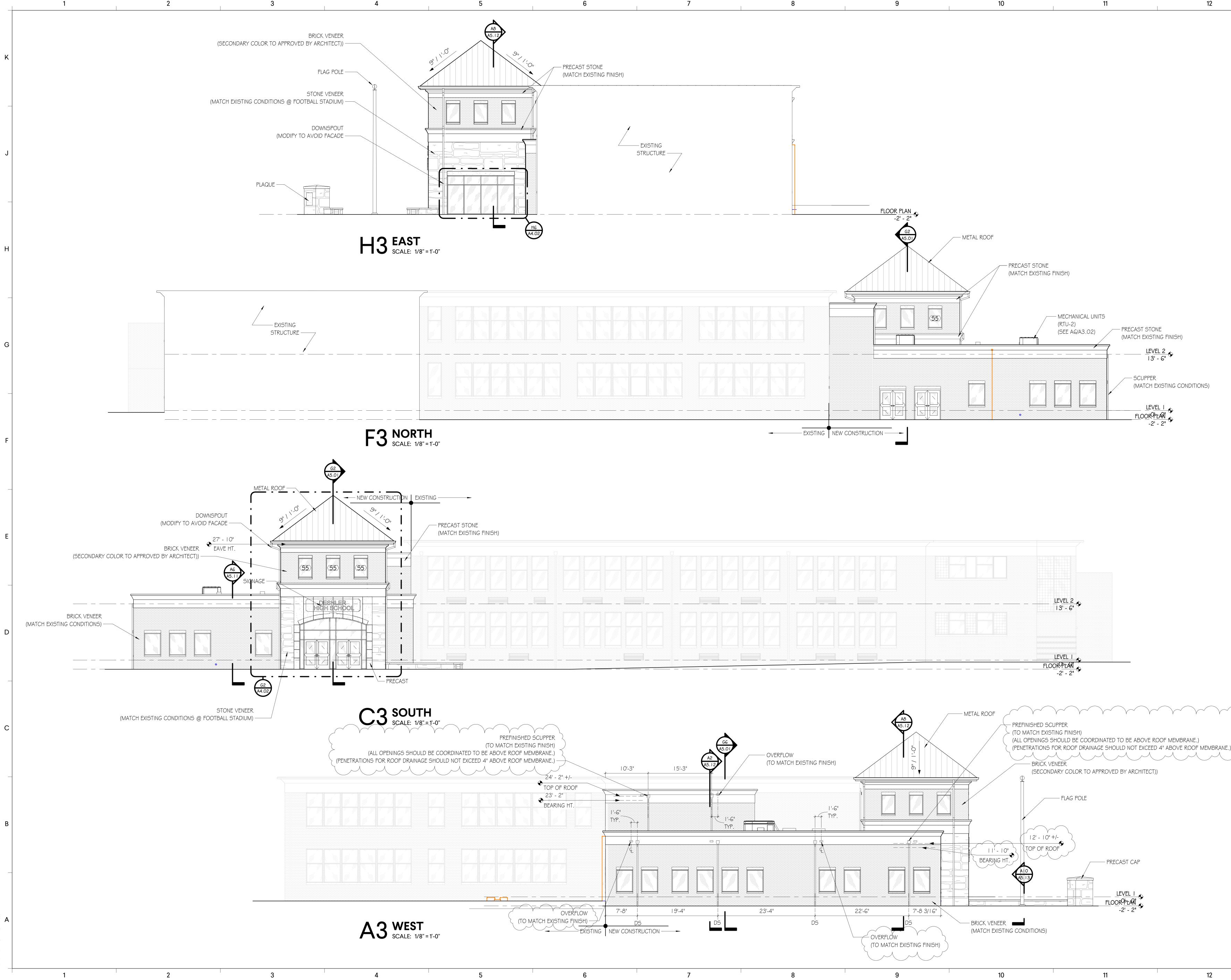




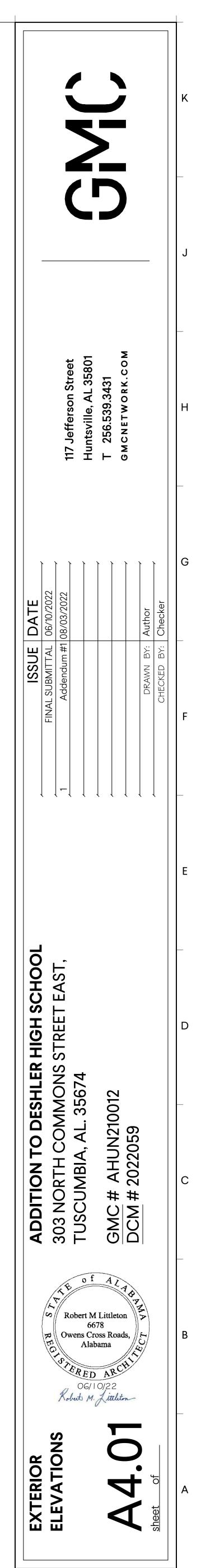


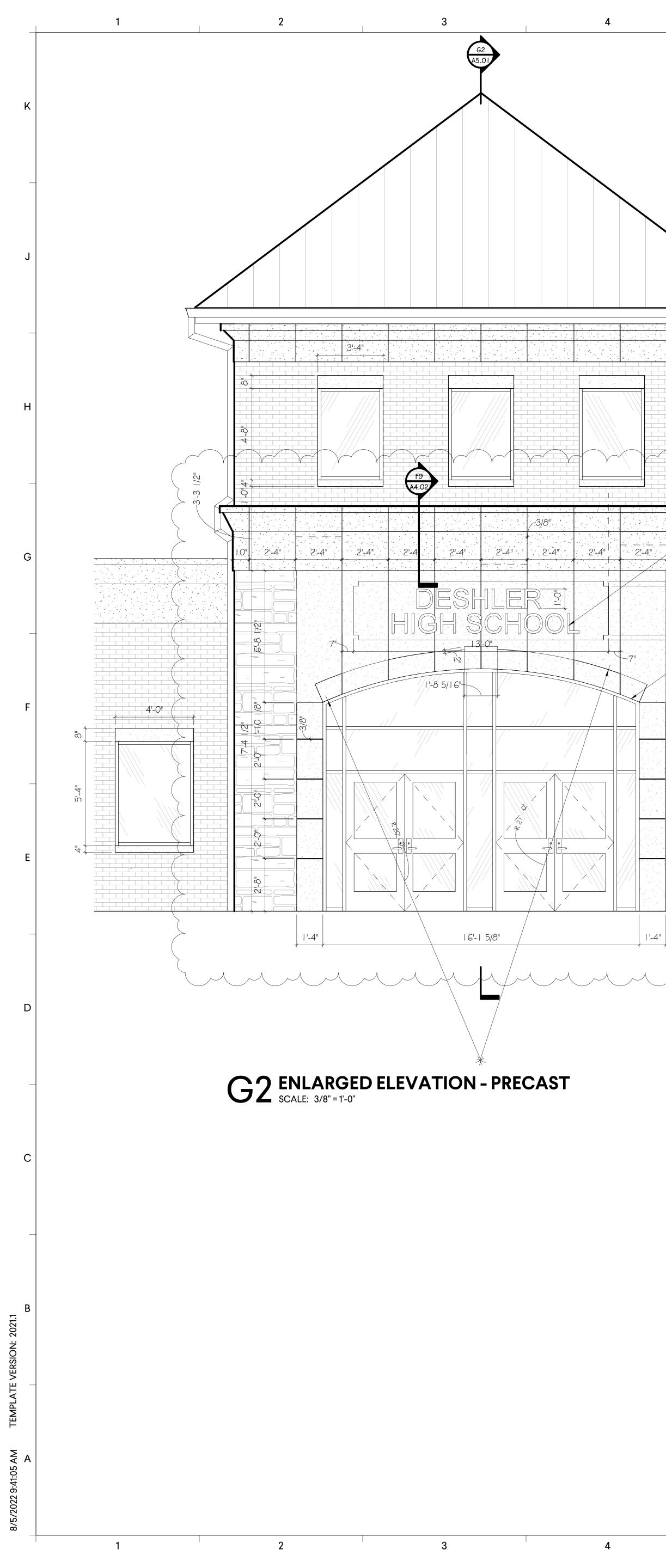


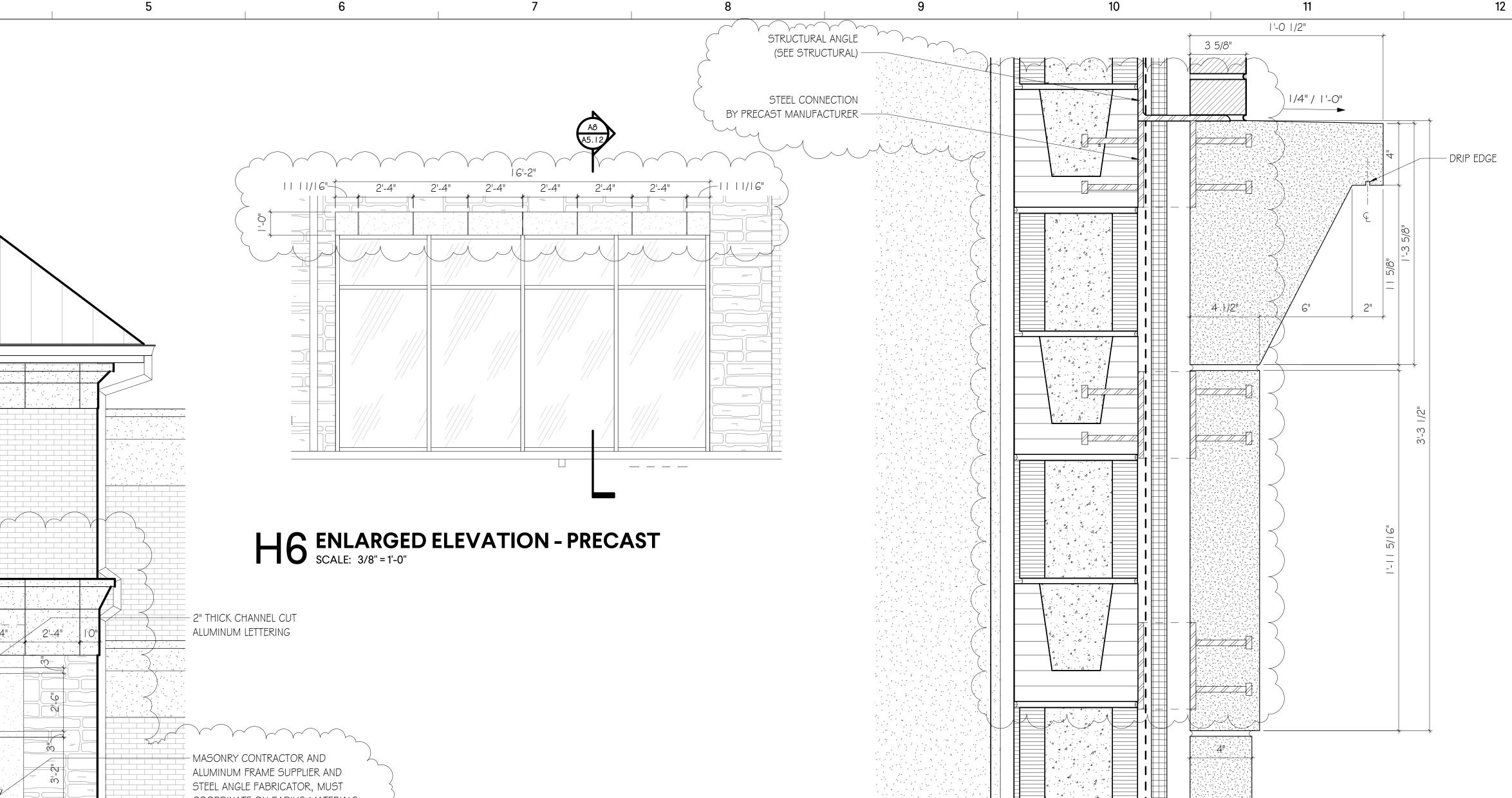




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MASONRY CONTRACTOR AND ALUMINUM FRAME SUPPLIER AND STEEL ANGLE FABRICATOR, MUST COORDINATE ON RADIUS MATERIALS

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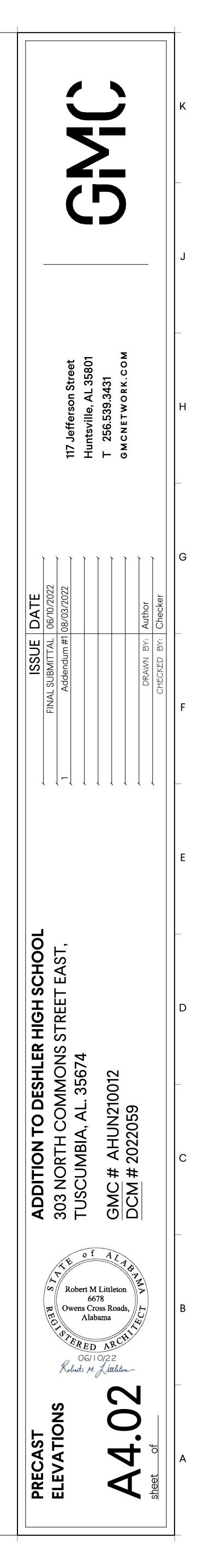
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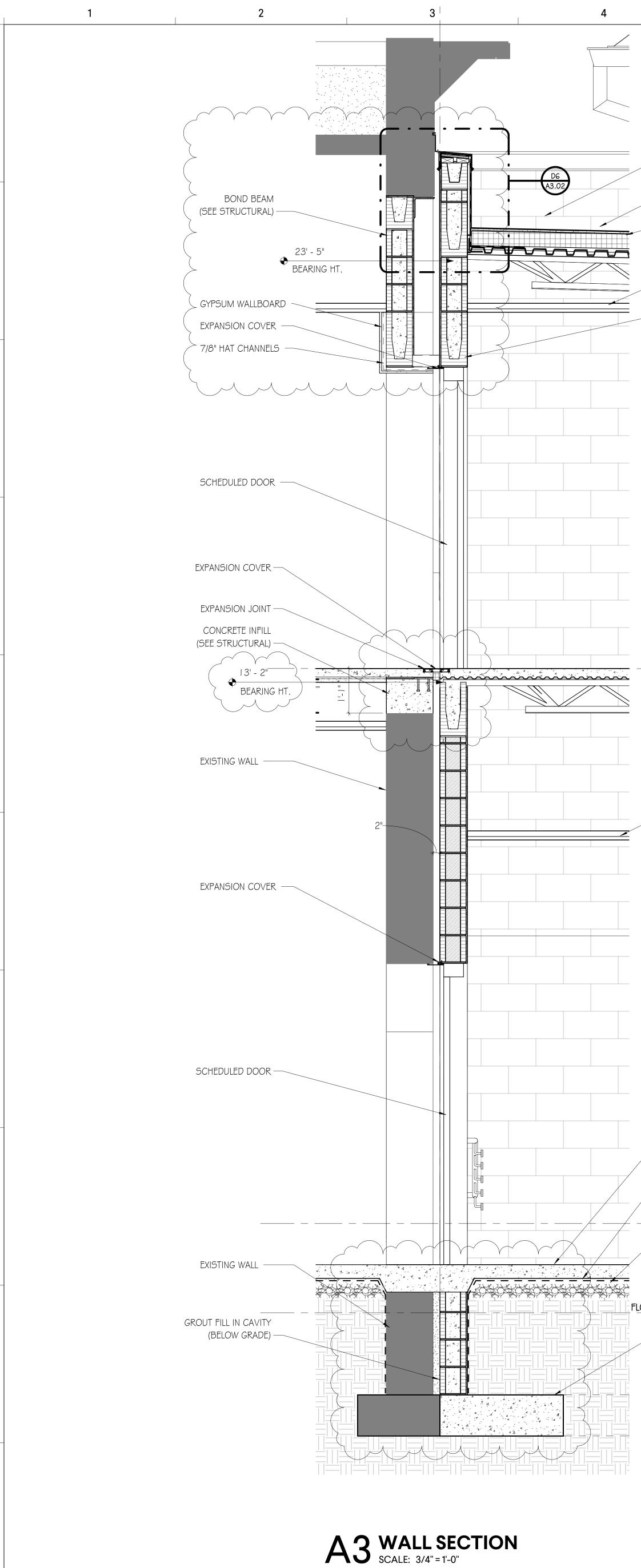


F9 PRECAST DETAIL SCALE: 3" = 1'-0"

10

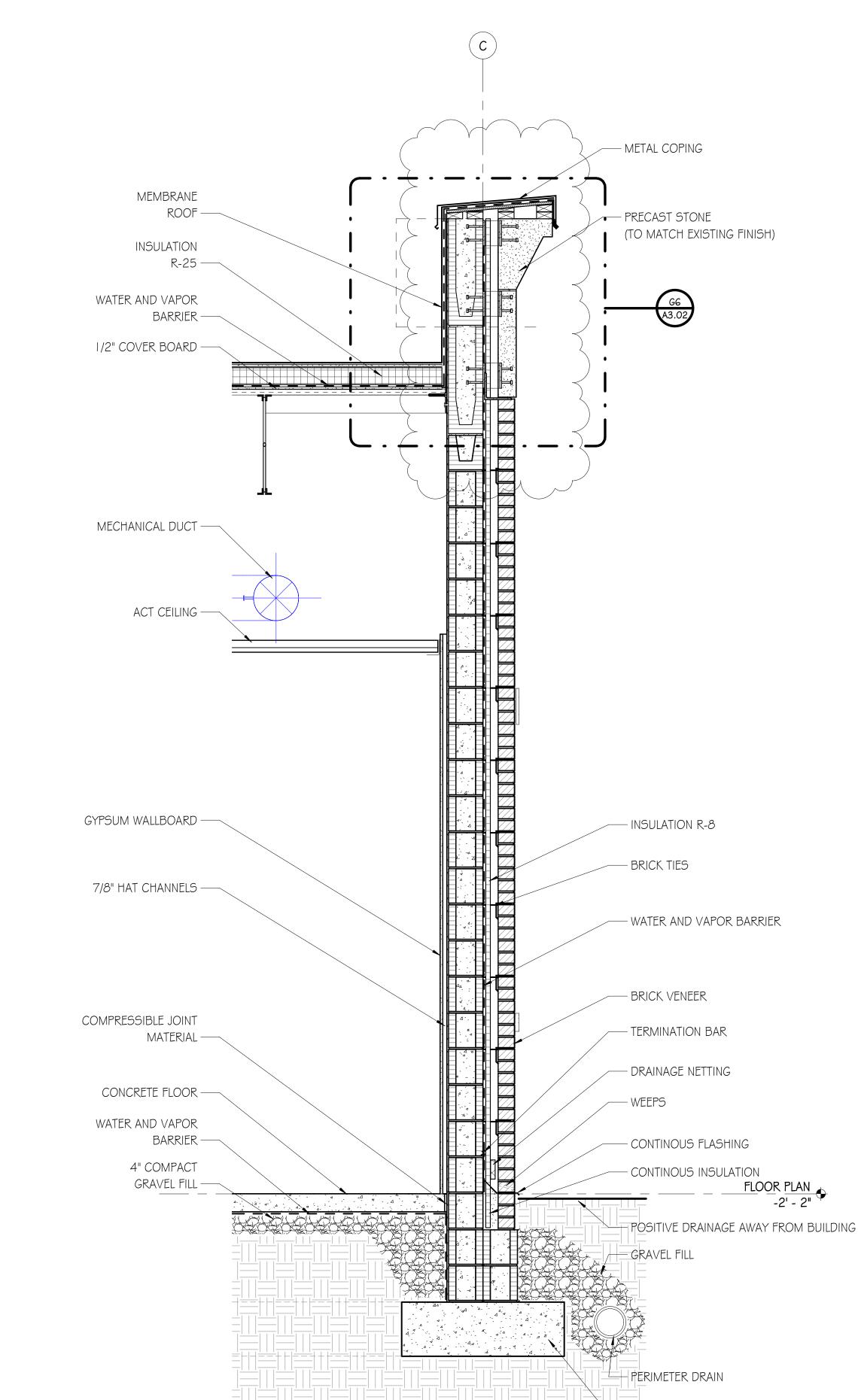
11





- TOP MEMBRANE ROOF
- INSULATION R-25
- WATER AND VAPOR BARRIER
- ACT CEILING
- BOND BEAM

- BOND BEAM (SEE STRUCTURAL)



- CONCRETE FLOOR (SEE STRUCTURAL)

- WATER AND VAPOR BARRIER LEVEL | 0' - 0"

— 4" COMPACT GRAVEL FILL

FLOOR PLAN -2' - 2"

4

LEVEL 2 13' - 6"

- ACT CEILING

- CONCRETE FOOTING (SEE STRUCTURAL)

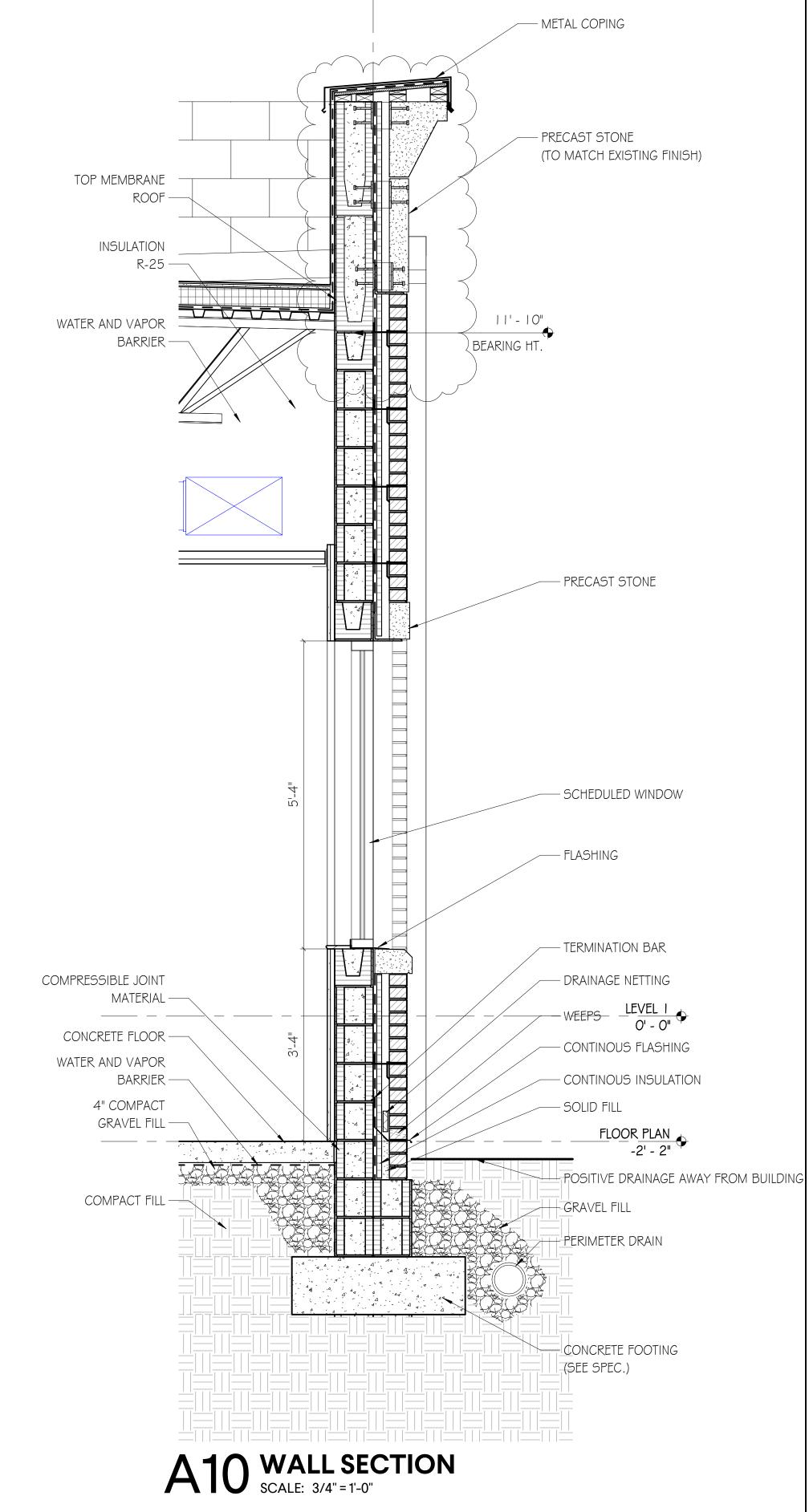
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A6 TYPICAL WALL SECTION SCALE: 3/4" = 1'-0"

CONCRETE FOOTING (SEE SPEC.)

8

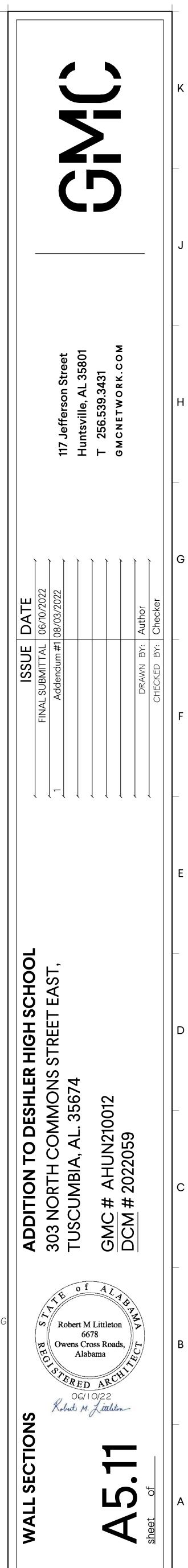
-2' - 2

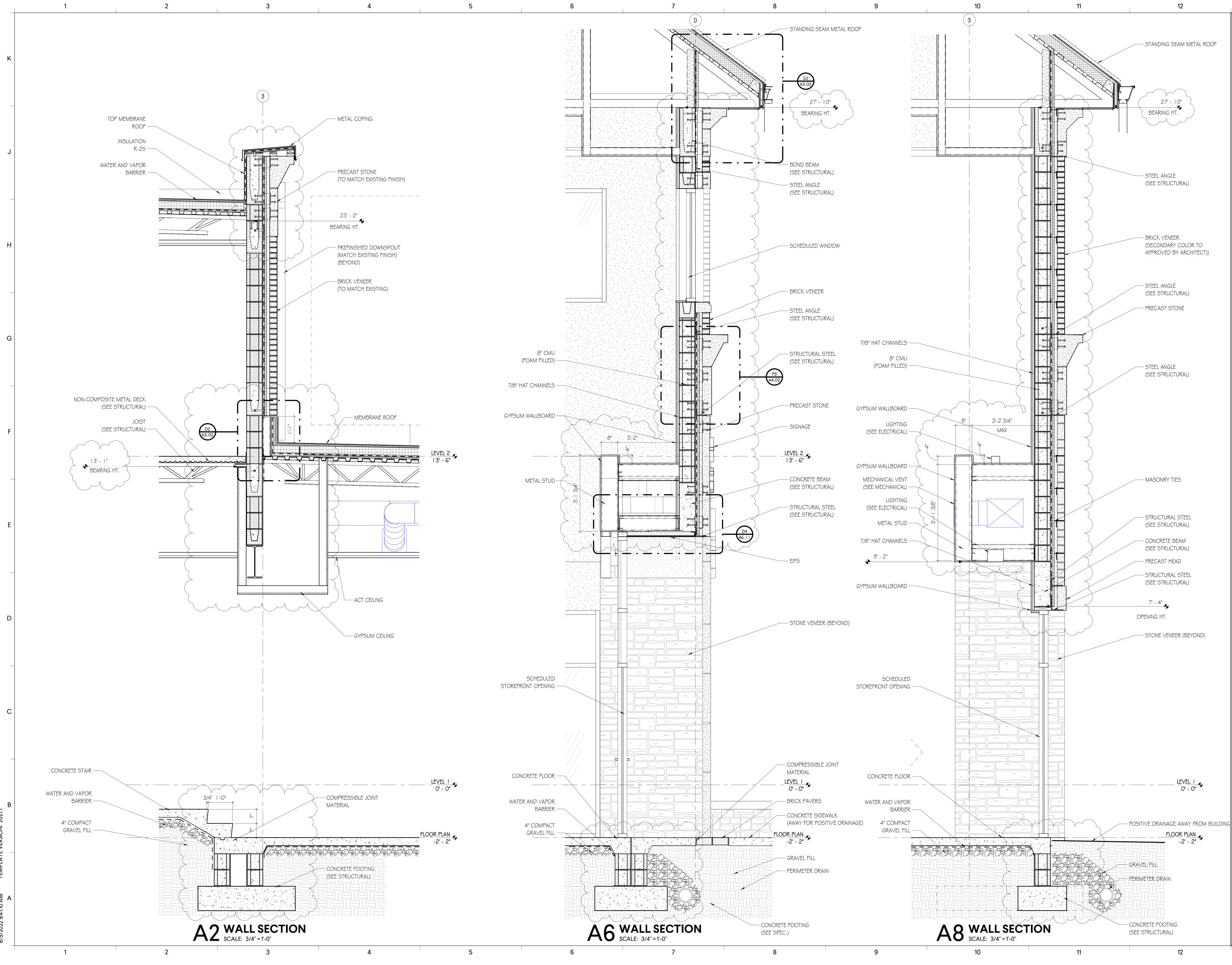


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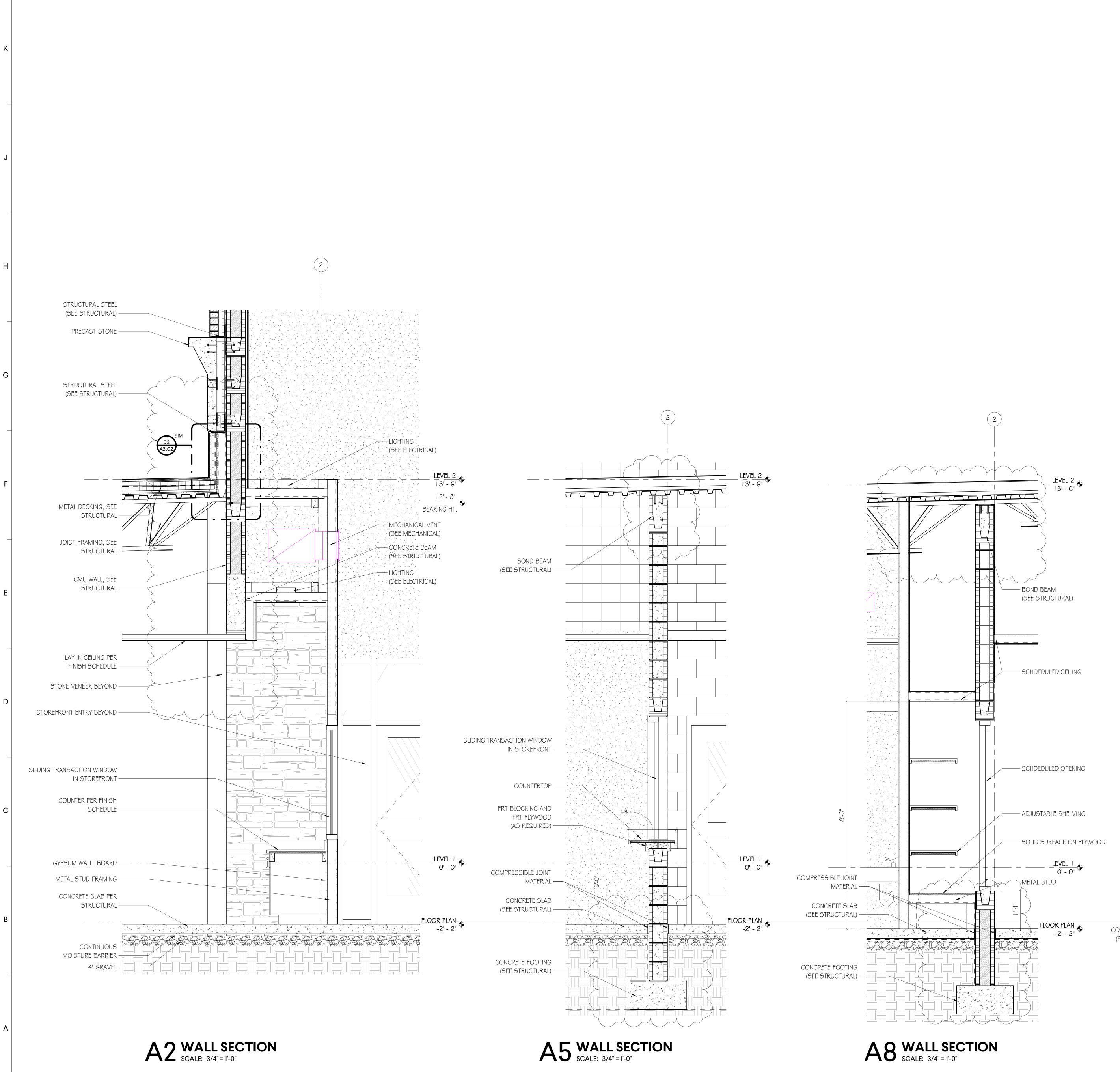
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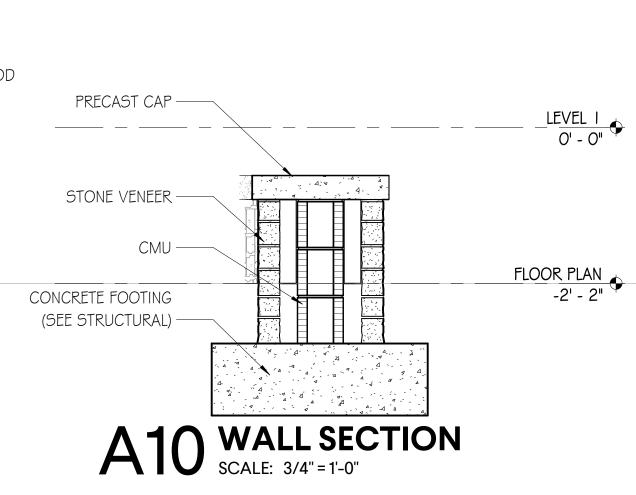


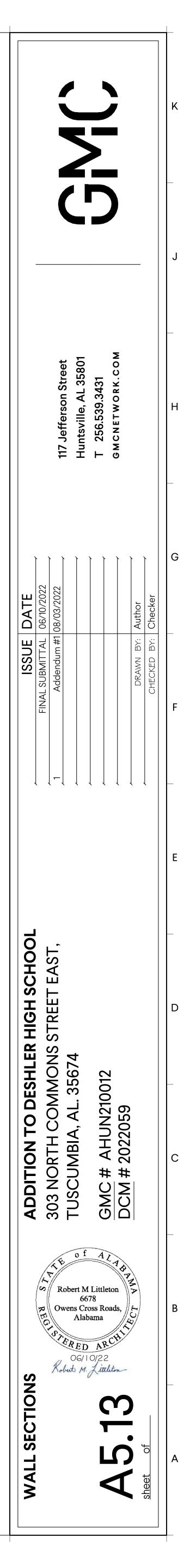
റ്റ S ≥ à 56. N E 0 M 117 Hui T G M **DATE** 06/10/202 H I I I **ISSL** SCHOOL TEAST, **N TO DESHLER HIGH S** H COMMONS STREET 1A, AL. 35674 GMC # AHUN210012 DCM # 2022059 ADDITION T 303 NORTH C TUSCUMBIA, of Ar /∽// Robert M Littleton ` 6678 Owens Cross Roads, Alabama 06/10/22 Roberts M. Littleton S Ζ Ο N Ŭ C S

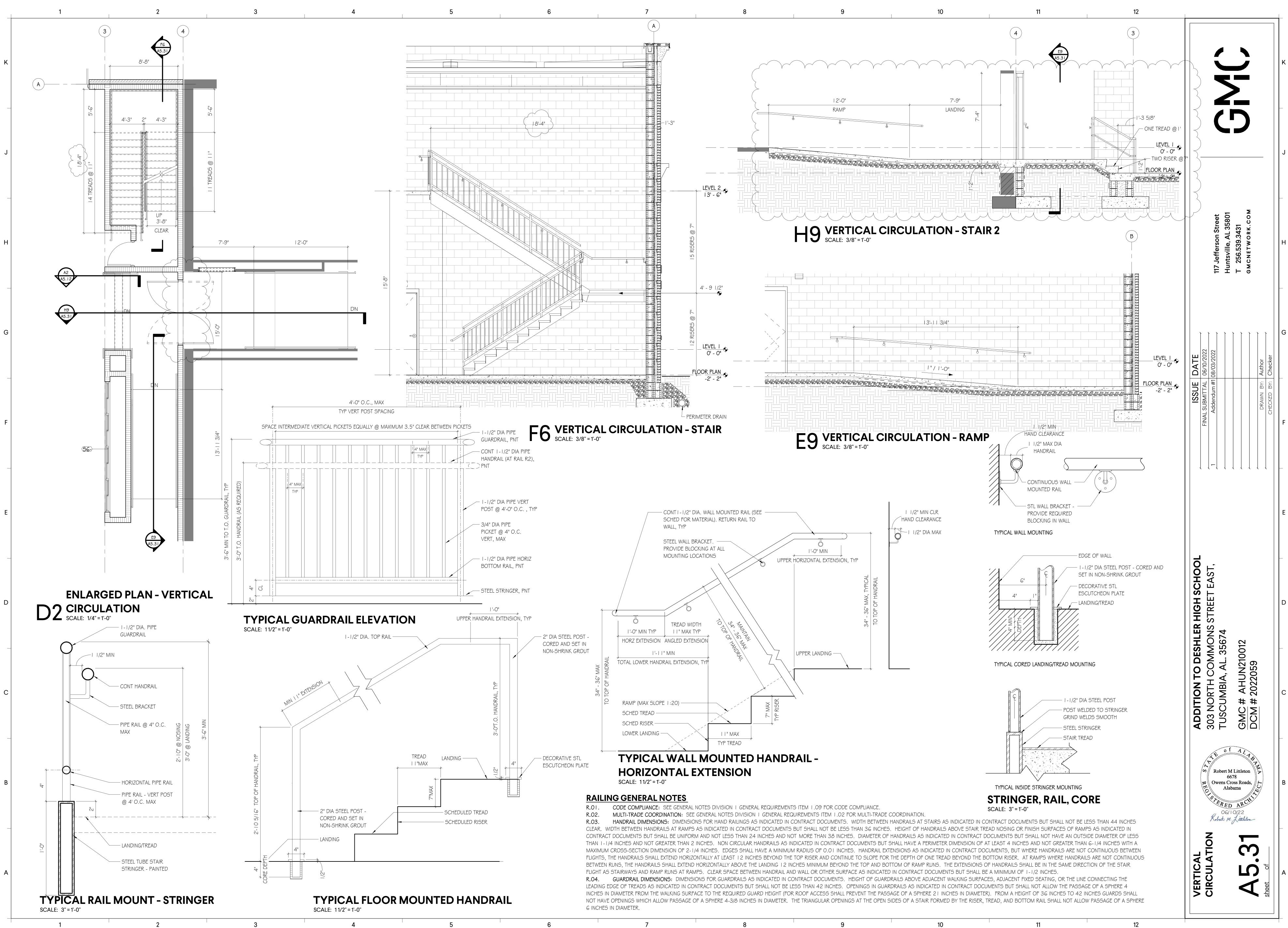


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LOCATION SZ DOOR PAME HARDWARE DTALE Image: Construction III IIII IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII			1			2				3						4				5 DOC	DR S		DU
No. No. <th></th> <th></th> <th></th> <th>L</th> <th>OCATION</th> <th></th> <th>SIZE</th> <th></th> <th></th> <th>DOOR</th> <th>2</th> <th></th> <th>FRAME</th> <th></th> <th>ł</th> <th>HARD</th> <th>WAR</th> <th></th> <th></th> <th></th> <th>-</th> <th></th> <th></th>				L	OCATION		SIZE			DOOR	2		FRAME		ł	HARD	WAR				-		
IDRA LOBAV E-0 7-0 344 FIX X IDRA COBADD E-0 7-0 344 FIX H FIX X H104 X IDRA COBADD E-0 7-0 344 FIX H H H X H104 X H104 X H104 X H104 X H1046 T70468 T204 T2			<	DOOR NUMBER	ROOM NAME	WIDTH	HEIGHT	THICKNESS	DOOR TYPE	MATERIAL	ASS / L	FRAME TYPE	MATERIAL	ASS T	SET N	HOLD OPEN	FIRE ALARM INT.		HEAD	JAMB	SILL	FIRE RATING	NUMBERED NOTES
Integration Construct C / 2 / 0 1 347 File FM X CCARACT Ether of the second s		NB COMPACE 6.0 10		100B 101A	LOBBY CORRIDOR	6' - 0" 6' - 0" 6' - 0"	7' - 0'' 7' - 0''	13/4" 13/4"	FG2 FG2	HM HM		SF SF	HM HM										
NO. TOLLT B-0 7-0 TOK F WO P HM CEAAGE Constant NO. STORET S-0 7-0 TOK WO D H HM CEAAGE COARD				101D 101E	CORRIDOR STAIR	6' - 0" 3' - 0"	7' - 0'' 7' - 0''	13/4" 13/4"	FG2 F	HM			HM HM						H7/A6.01	F7/A6.01			
COUNSELOR SO 7 -0' 134' F V/O F HM CEARABIL CEARABIL CEARABIL SO MN 100 ACCORDED 8 3 -0' 7 -0' 134' F V/O FI HM CEARABIL CEARABIL SO MN 100 CEARABIL SO 7 -0' 134' HS V/O GLARABIL CEARABIL CEARABIL SO MN 100 CEARABIL SO 7 -0' 134' HS V/O GLARABIL CEARABIL CEARABIL SO MN 101 CEARABIL SO 7 -0' 134' HS V/O GLARABIL CEARABIL SO MN 101 CEARABIL SO 7 -0' 134' F V/O FI HM CEARABIL CEARABIL SO MN 112 VAUINS S-0' 7-0' 134' F V/O FI HM CEARABIL CEARABIL SO MN 112 CEARBIL SO 7-0' 134' F V/O FI HM CEARABIL SO MN 113 CECTECALL S-0' 7-0' 134' F				103 104	TOILET STORAGE	3' - 0" 3' - 0"	7' - 0'' 7' - 0''	13/4" 13/4"	F F F N5	WD WD	G1	F1 F1	HM HM						C6/A6.01 C6/A6.01	C9/A6.01 C9/A6.01			
109 ASSITANT 3 -0" 7 -0" 134" NS WD CI P H M C64A8.01 GMARS 90 MIN 110 COMPERINCE 3 -0" 7 -0" 134" NS WD R H H C64A8.01 GMARS 90 MIN 111 WORKCOM 3 -0" 7 -0" 134" F WD F H H C64A8.01 GMARS 90 MIN 111 WORKCOM 3 -0" 7 -0" 134" F WD F H M C64A8.01 GMARS 90 MIN 112 WORKCOM 3 -0" 7 -0" 134" F WD F HM C64A8.01 GMARS 90 MIN 115 SCORDOR 3 -0" 7 -0" 134" F WD F HM C64A8.01 GMARS 90 MIN 116 MARS 5 -0" 7 -0" 134" F WD F HM C64A8.01 GMARS<	Image: Section 1 3 or 7 or 12 are 18 word or 11 min Image: Section 1 Image: Sectio	06 ADDITION 07 2-3 144 N N 0 1 N 0		107A 107B	COUNSELOR RECORDS RECORDS	3' - 0" 3' - 0"	7' - 0'' 7' - 0''	13/4" 13/4"	F F F	WD WD		F1 F1	HM HM						C6/A6.01 C6/A6.01	C9/A6.01 C9/A6.01			
112 TOLLET 8-0° 7-0° SAT F WD P HM C2A801 C2A801 C2A801 C3A801 SMM 116 WORKS 5-0° 7-0° SAT F WD P HM C2A801 C3A801 SMM 116 CORRIDOR 5-0° 7-0° SAT F WD P HM C3A801 C3A801 SMM 116 CORRIDOR 5-0° 7-0° SAT F WD P HM C3A801 SMM SMM 117 WOMEN 5-0° 7-0° SAT F WD P HM C3A801 C3A801 SMM 120 IFMANCE 5-0° 7-0° SAT F WD P HM C3A801 C3A801 SMA01 SMM 122 IFMANCE 5-0° 7-0° SAT NS WD O P HM X C2A801 C3A801 SMA01 <	Image: Volter Marked B, et of 7 - or 1 start F WOW IN F HW Image: Volter Marked B, et of 7 - or 1 start HW Image: Vo			109 110	ASSISTANT PRINICPAL CONFERENCE	3' - 0" 3' - 0"	7' - 0'' 7' - 0''	13/4" 13/4"	N5	WD WD	G1	F1 F1	HM HM						C6/A6.01 C6/A6.01	C9/A6.01 C9/A6.01		90 MIN 90 MIN	
17 WOMEN 3 -0* 7 -0* 134/r F WO FI HM CRABDI CRABDI CRABDI SUBSTITUT 18 BLECTECLU 3 -0* 7 -0* 134/r F WO FI HM CRABDI CRABDI CRABDI SUBSTITUT 19 BLECTECLU 3 -0* 7 -0* 134/r F WO FI HM CRABDI CRABDI SUBSTITUT 100 FI S -0* 7 -0* 134/r F WO FI HM CRABDI CRABDI SUBSTITUT 120 FILMANCE 3 -0* 7 -0* 134/r F WO FI HM CRABDI CRABDI SUBSTITUT 122 FRANCE 3 -0* 7 -0* 134/r NS WD GI FI HM X CRABDI CRABDI SUBSTITUT	Image: Name of the image o			112 113	TOILET WAITING	3' - 0" 3' - 0"	7' - 0'' 7' - 0''	13/4" 13/4"	N5 F F F	WD WD	G1	F1 F1	HM HM						C6/A6.01 C2/A6.01	C9/A6.01 C3/A6.01		90 MIN	
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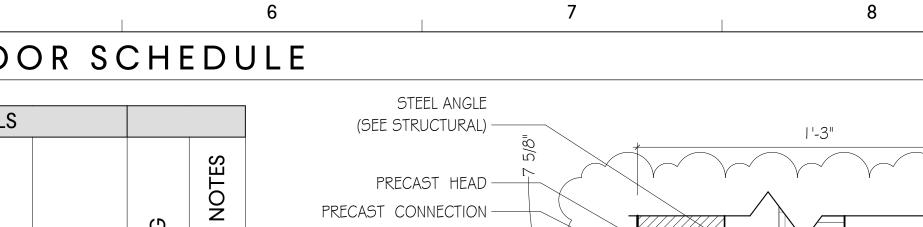
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DOOR

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AIR GAP –

FLASHING -

WEEP -

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I " RIGID INSULATION

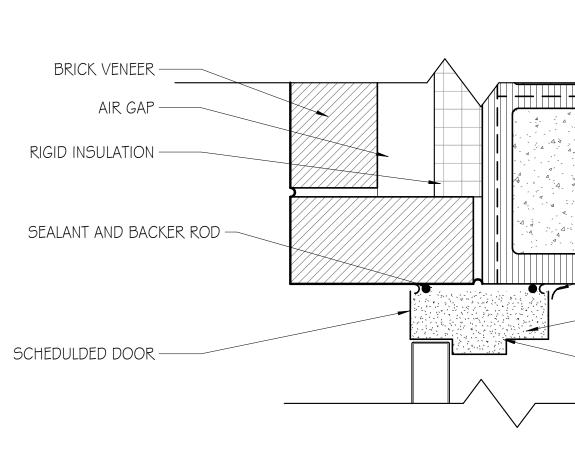
STEEL BRICK LINTEL

BRICK BEYOND -

FRAME BEYOND -

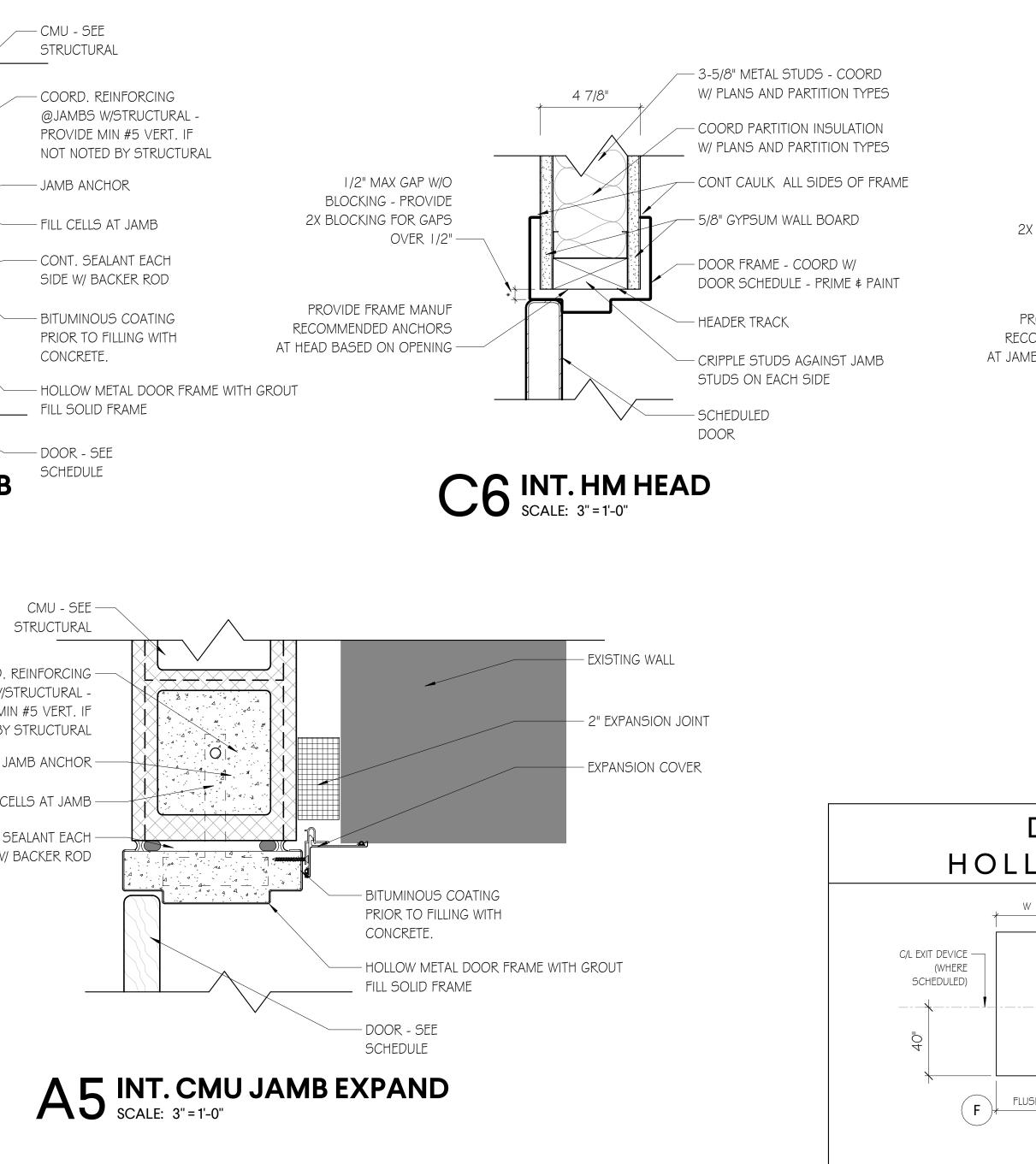
SCHEDULED DOOR -

SEALANT ALONG FRAME -



H7 EXT. CMU HEAD SCALE: 3" = 1'-0"

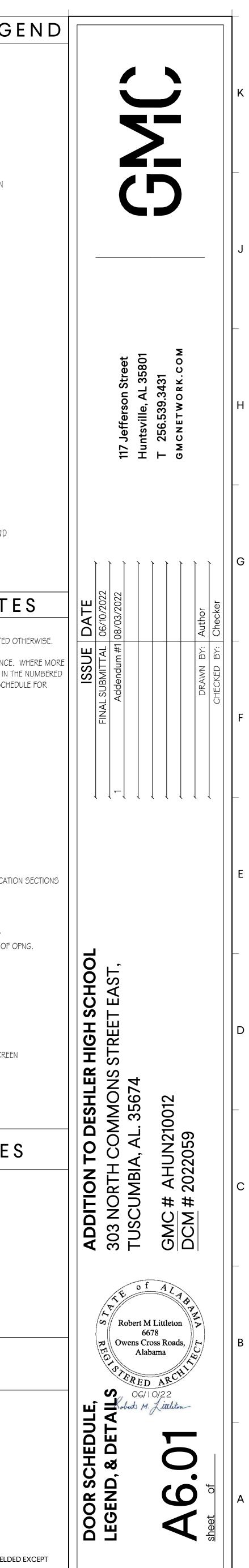


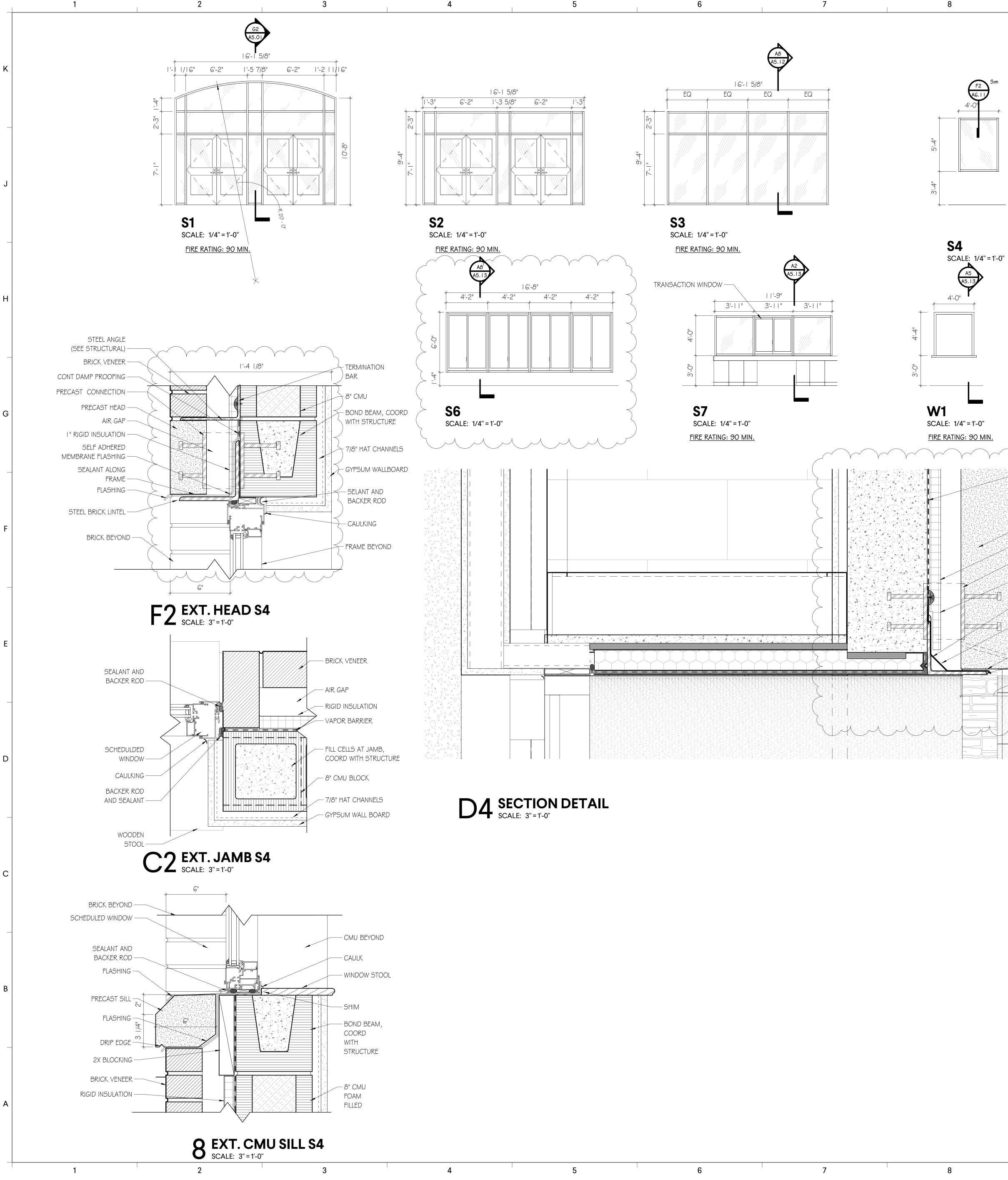


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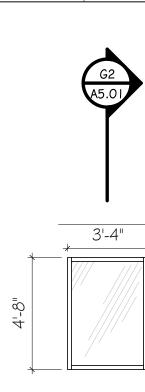
9 10	11 12
	DOOR COMMENTS LEG
	01 MAGNETIC HOLD-OPEN DEVICES
TERMINATION BAR	02 AUTOMATIC DOOR OPERATOR, SEE SPECIFICATIONS
	(03) AUTOMATIC ENTRANCE DOORS, SEE GLAZING SCHEDULE FOR CONFIGURATION
BOND BEAM, COORD WITH	 (04) ALUMINUM-FRAME ENTRANCE DOORS, SEE GLAZING SCHEDULE FOR CONFIG. (05) ALL-GLASS ENTRANCE DOORS, SEE GLAZING SCHEDULE FOR CONFIGURATION
STRUCTURE	 (05) ALL-GLASS ENTRANCE DOORS, SEE GLAZING SCHEDULE FOR CONFIGURATION (06) ALUMINUM-FRAME ICU/CCU DOOR, SEE GLAZING SCHEDULE FOR CONFIGURATION
SEALANT AND BACKER ROD	(07) SLIDING ICU/CCU DOOR, SEE SPECIFICATIONS
	(08) OVERHEAD COILING DOOR, SEE SPECIFICATIONS
GROUT FILL FRAME SOLID.	09 OVERHEAD COILING GRILLE, SEE SPECIFICATIONS
COAT INTERIOR OF FRAME WITH BITUMINOUS COATING	10 SOUND CONTROL DOOR, SEE SPECIFICATIONS
5" DOOR FRAME, PRIME & PAINT	11 POCKET DOOR AND FRAME, SEE SPECIFICATIONS
	12 LEAD-LINED DOOR AND FRAME, SEE SPECIFICATIONS: RADIATION PROTECTION
	13 SOUND RATED ASSEMBLY. MATCH RATING OF PARTITION IN WHICH LOCATED
	14 THERMAL RATED [INSULATED] ASSEMBLY
	15 DOOR EDGE CONSTRUCTION: SEAMLESS
	16 LEVEL 3 AND PHYSICAL PERFORMANCE LEVEL A [EXTRA HEAVY-DUTY]
	EGRESS - EXIT ONLY DOOR
	(18) EXISTING DOOR AND FRAME TO REMAIN
	(19) EXISTING FRAME TO REMAIN
FILL CELLS AT JAMB, COORD WITH	(20) INSTALL NEW DOOR HARDWARE AS SCHED, MODIFY EXIST. DOOR/FRAME AS REQ'D
STRUCTURE FOR REINFORCING	 (21) MECHANICAL KEYPAD LOCK. SEE SPECIFICATIONS (22) STORM SHELTER DOOR
CAULKING	DOOR GENERAL NOTE
GROUT FILL FRAME SOLID. COAT INTERIOR OF FRAME	I) GENERAL:
WITH BITUMINOUS COATING	A. DOOR AND/OR FRAME CONSTRUCTION SHALL BE AS SPECIFIED UNLESS NOTED O
DOOR FRAME, PRIME & PAINT	B. ALL TYPES OF DOORS ARE REPRESENTED IN THIS SCHEDULE FOR CONVENIENCE. DESCRIPTIVE INFORMATION MAY BE LOCATED ELSEWHERE, NOTATION IS MADE IN TH NOTES COLUMN. (E.G. ALUMINUM FRAMED ENTRANCE DOORS. SEE GLAZING SCHED
	CONFIGURATION.)
	 2) MATERIAL AND FINISH: A. MATERIALS AND FINISHES INDICATED ON THE SCHEDULE ARE AS FOLLOWS: HM HOLLOW METAL
	ST STEEL ST/S STEEL / STAINLESS OR STAINLESS CLAD
	WD SOLID CORE WOOD WD/PL WOOD / PLASTIC LAMINATE FACED
	WD/IR WOOD / IMPACT-RESISTANT VINYL-FACED AL ALUMINUM GL GLAZING/GLASS
	PREFIN PREFINISHED (OR FACTORY FINISHED) PNT PAINTED
	STN STAINED 3) GLASS:
4 7/8" 3-5/8" METAL STUDS - COORD	A. GLASS TYPES INDICATED ON THE SCHEDULE ARE AS FOLLOWS (SEE SPECIFICATIO 088000 "GLAZING" & 134900 "RADIATION PROTECTION"):
W/ PLANS AND PARTITION TYPES	MONOLITHIC: GI G.Omm CLEAR, TEMPERED G2 7.5mm CLEAR, LAMINATED, INTERLAYER COLOR: CLEAR
I/2" MAX GAP W/O BLOCKING - PROVIDE W/ PLANS AND PARTITION TYPES 5/8" GYPSUM WALL BOARD	G3 7.5mm CLEAR, LAMINATED, INTERLAYER COLOR: ARCTIC SNOW G4 8.0mm CLEAR, FIRE-RATED CERAMIC GLAZING, MATCH RATING OF O
2X BLOCKING FOR GAPS OVER 1/2"	G5 4.0mm(x2) ULTRACLEAR, TEMPERED, 2 LITES + INTEGRAL BLINDS G6 I 2.5mm SAFETY, LEAD BORATE GLASS, SEE SPECIFICATIONS
DOOR FRAME - COORD W/ DOOR SCHEDULE - PRIME & PAINT	G7 I 2.5mm SAFETY, LASER RATED, SEE SPECIFICATIONS INSULATING: IGI I INCH INSULATING, VISION LITE
PROVIDE FRAME MANUF	IG2 I INCH INSULATING, SPANDREL LITE
MB BASED ON OPENING	 4) LOUVERS: A. DOOR LOUVER TYPES INDICATION ON THE SCHEDULE ARE AS FOLLOWS: LI A" W X B" H, SIGHT PROOF, WEATHER RESISTANT, WITH INSECT SCREEN
SCHEDULED DOOR	L2 A" W X B" H, LIGHT PROOF
	 5) DOOR HARDWARE: A. "HARDWARE SET NUMBER" REFERS TO HARDWARE SETS SPECIFIED IN SPECIFICATION SECTION 087 I 00 "DOOR HARDWARE".
SCALE: 3" = 1'-0"	FRAME PROFILES
	NOTE: $X = THROAT DIMENSION / PARTITION THICKNESS$
	VARIES $\frac{1}{\sqrt{4''}}$
	$(\mathbf{A}) \stackrel{\mathbb{N}}{\cong} (\mathbf{H})$
DOOR TYPES - WOOD +	FRAME TYPES
LOW METAL SWING DOORS	-HOLLOW METAL
W 7" VAR. 7" 5", 7" 5", 7" 7"	
LUSH FG2 STILE STILL STILL STILL STILE STILE STILE STILE STILE STILL STI	(F1) + (F13) +
	NOTE: ALL FRAMES TO BE FACE-WELDE WHERE NOTED "KD"
9 10	11 12

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SCALE: 1/4" = 1'-0" FIRE RATING: 90 MIN. SEE G2/A4.02 FOR SILL HEIGHT

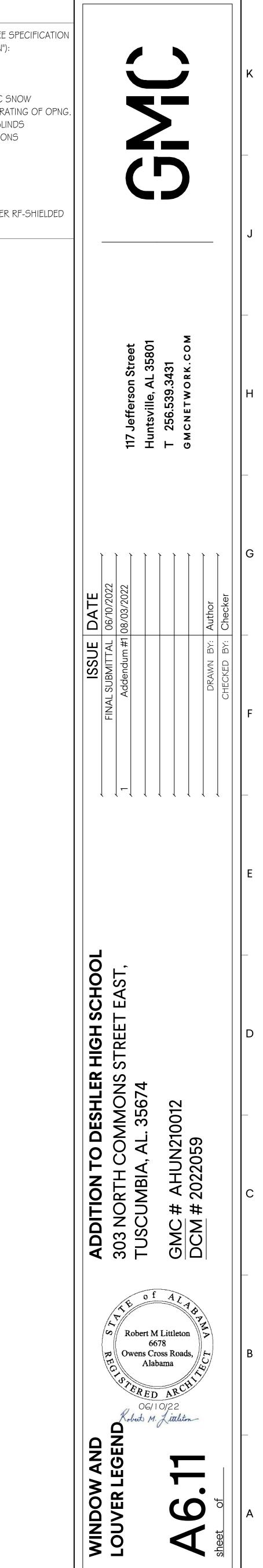
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GL	ASS SC	HEDULE	
SECTIO		ICATED ON THE SCHEDULE ARE AS FOLLO DO "GLAZING" & I 34900 "RADIATION PRO	,
GI	6.0mm	CLEAR, TEMPERED	
G2	7.5mm	CLEAR, LAMINATED, INTERLAYER COLOR	CLEAR
G3	7.5mm	CLEAR, LAMINATED, INTERLAYER COLOR	ARCTIC SI
G4	8.0mm	CLEAR, FIRE-RATED CERAMIC GLAZING, I	MATCH RAT
G5	4.0mm(x2)	ULTRACLEAR, TEMPERED, 2 LITES + INTE	EGRAL BLIN
G6	12.5mm	SAFETY, LEAD BORATE GLASS, SEE SPEC	CIFICATION
G7	12.5mm	SAFETY, LASER RATED, SEE SPECIFICATI	ONS
INSU	LATING:		
IG I	I INCH	INSULATING, VISION LITE	
100		INICI ILATING CRANDREL LITE	

IG2 I INCH INSULATING, SPANDREL LITE

SPECIALTY GLASS: RG1 RF-SHIELDED GLASS AT MRI ROOM TO BE PROVIDED UNDER RF-SHIELDED ENCLOSURE PACKAGE

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		- PRECAST HEAD	2
		- AIR GAP	
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		- PRECAST CONNECTION (BY PRECAST MANUFACTURER)	\prec
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		TERMINATION BAR	
		SELF ADHERED	\leq
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		- SEALANT ALONG	
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		- STEEL ANGLE	2
		(SEE STRUCTURAL)	\checkmark
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	-6 1/2"		
	<u>.</u>	- PRECAST	\leq
		(BEYOND)	4
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SECTION 05 3100

STEEL DECK

PART 1 - GENERAL

1.1 <u>SUMMARY:</u>

- A. Section Includes:
 - 1. Steel roof deck.
 - 2. Non-composite steel form deck.
- B. Related Documents: Drawings and general provisions of Contract, including General and Supplementary Conditions and Division 1 Specification Sections, apply to this section.
- C. Related Sections: The following Sections contain requirements that relate to this Section:
 - 1. Section 03 3000, "Concrete":
 - 2. Section 05 1200, "Structural Steel": Shop-welded shear connectors.
 - 3. Section 05 5000, "Metal Fabrications": Framing openings with miscellaneous steel shapes.
 - 4. Section 05 2100, "Steel Joists".
 - 5. Section 07 8150, "Sprayed-On Fireproofing".
 - 6. Section 09 9000, "Painting":
 - a. Touch-up and repair painting of deck.
 - b. Touch-up and repair of special deck coatings.

1.2 <u>SUBMITTALS:</u>

- A. General: Submit each item in this Article according to the Conditions of the Contract and Division 1 Specification Sections.
- B. Submit all shop drawings on one reproducible print (sepia) and one blue line print only. The reproducible print will be returned. All blue line prints required by the Contractor will be the responsibility of the Contractor and shall be made after reproducible is returned.
- C. Product data for each type of deck, accessory, and product specified.
 - 1. Provide test data for mechanical fasteners used in lieu of welding for fastening deck to supporting structures.
- D. Shop drawings showing layout and types of deck panels, anchorage details, reinforcing channels, pans, cut openings, closure strips, deck openings, special jointing, accessories, and attachment to other construction.
- E. Welder certificates signed by Contractor certifying that welders comply with requirements specified under the "Quality Assurance" Article.

- F. Product test reports from qualified independent testing agencies evidencing compliance with requirements of the following based on comprehensive testing:
 - 1. Mechanical fasteners.
- G. Research reports or evaluation reports of the model code organization acceptable to authorities having jurisdiction that evidence steel deck's compliance with the building code in effect for the Project.

1.3 **QUALITY ASSURANCE:**

- A. Codes and Standards: Comply with provisions of the following codes and standards, except as otherwise indicated:
 - 1. American Iron and Steel Institute (AISC), "Specification for the Design of Cold-Formed Steel Structural Members".
 - 2. American Welding Society (AWS), D1.3 "Structural Welding Code Sheet Steel".
 - 3. Steel Deck Institute (SDI), "Design Manual for Composite Decks, Form Decks and Roof Decks".
- B. Installer Qualifications: Engage an experienced Installer who has completed steel deck similar in material, design, and extent to that indicated for this Project and with a record of successful in-service performance.
- C. Welding Standards: Comply with applicable provisions of AWS D1.1 "Structural Welding Code-Steel" and AWS D1.3 "Structural Welding Code-Sheet Steel."
 - 1. Certify that each welder has satisfactorily passed AWS qualification tests for welding processes involved and, if pertinent, has undergone recertification.
- D. Welded decking in place is subject to inspection and testing. Owner will bear expense of removing and replacing portions of decking for testing purposes if welds are found to be satisfactory. Remove work found to be defective and replace with new acceptable work.
- E. Fire-Test-Response Characteristics: Where indicated, provide steel deck panels identical to those tested as part of an assembly for fire resistance per ASTM E 119 by a testing and inspection agency performing testing and follow-up services, that is acceptable to authorities having jurisdiction.
- F. Fire-Resistance Ratings: As indicated by design designations listed in UL "Fire Resistance Directory," or by Warnock Hersey or another testing and inspecting agency.
- G. Labeling: Identify steel deck with appropriate markings of applicable testing and inspecting agency.
- H. Installation Tolerances: Conform to the installation tolerances specified in Part 3.

1.4 DELIVERY, STORAGE, AND HANDLING:

- A. Protect steel deck from corrosion, deformation, and other damage during delivery, storage, and handling.
- B. Stack steel deck on platforms or pallets and slope to provide drainage. Protect with a waterproof covering and ventilate to avoid condensation.

1.5 <u>COORDINATION:</u>

A. Coordinate installation of sound-absorbing insulation strips in acoustic deck ribs with related units of Work specified in other Sections to ensure that the insulation is protected against damage from effects of the weather and other causes.

PART 2 - PRODUCTS

2.1 <u>MANUFACTURERS:</u>

- A. Manufacturers: Subject to compliance with requirements, provide products by one of the following:
 - 1. Steel Roof Deck:
 - a. Bowman Metal Deck Armco, Inc.
 - b. Epic Metals Corp.
 - c. Centria/Robertson.
 - d. Vulcraft Div. of Nucor Corp.
 - e. Wiremold/Walker.
 - f. Wheeling Corrugating Co., Div. of Wheeling-Pittsburgh Steel Corp.
 - 2. Non-Composite Steel Form Units:
 - a. Epic Metals Corp.
 - b. Vulcraft Div. of Nucor Corp.
 - c. New Millennium

2.2 ROOF DECK:

- A. Steel Roof Deck: Fabricate panels without top-flange stiffening grooves conforming to SDI Publication No. 28 "Specifications and Commentary for Steel Roof Deck" and the following:
 - 1. Galvanized-Steel Sheet: ASTM A 446, Grade A, G 60 (ASTM A 446M, Grade A, Z 180) zinc coated according to ASTM A 525 (ASTM A 525M).
 - 2. Deck Profile:
 - a. Type WR, wide rib.
 - 3. Profile Depth:
 - a. 1-1/2 inches.
 - 4. Design Uncoated-Steel Thickness:
 - a. 0.0295 inch.
 - 5. Span Condition:
 - a. Triple span or more.

- 6. Side Joints:
 - a. Overlapped or interlocking seam at Contractor's option.

2.3 FLOOR DECK:

- A. Non-Composite Steel Form Deck: Fabricate ribbed-steel sheet non-composite form deck panels conforming to SDI Publication No. 28 "Specifications and Commentary for Non-Composite Steel Form Deck," the minimum section properties indicated, and the following:
 - 1. Galvanized-Steel Sheet: ASTM A 653/A 653M, Structural Steel (SS), Grade 60 minimum, G 90 (Z275) zinc coated.
 - 2. Profile Depth:
 - a. 9/16 inch.
 - 3. Design Uncoated-Steel Thickness:
 - a. 0.0179 inch (26 Gauge).
 - 4. Span Condition:
 - a. Triple span or more.
 - Side Joints:
 - a. Overlapped.

2.4 FABRICATION AND ACCESSORIES:

5.

- A. General: Form deck units in lengths of three or more spans, with flush, telescoped, or nested 2inch laps at ends and interlocking or nested side laps, unless noted. End laps shall occur over a support.
- B. Roof Deck Units: Provide deck configurations that comply with SDI "Specifications and Commentary for Steel Roof Deck".
- C. Cant Strips: Fabricate cant strips of not less than 20 gage galvanized sheet steel of same quality as the deck units. Bend cant strips to form a 45 degree cant not less than 5 inches wide with top and bottom flanges not less than 2 inches wide, unless noted. Provide cant strips in 10 foot lengths where possible.
- D. Ridge and Valley Plates: Fabricate ridge and valley plates of not less than 20 gage galvanized sheet steel of the same quality as deck units. Bend to provide tight-fitting closure with deck units. Each leg of bend shall not be less than 3 inches. Provide plates in 10 foot lengths where possible.
- E. Accessories: Provide accessory materials for steel deck that comply with requirements indicated and recommendations of the steel deck manufacturer.
- F. Mechanical Fasteners: Manufacturer's standard, corrosion-resistant, low-velocity, powder-actuated or pneumatically driven carbon steel fasteners; or self-drilling, self-threading screws.
- G. Side Lap Fasteners: Manufacturer's standard, corrosion-resistant, hexagonal washer head; self-drilling, carbon steel screws, No. 10 (4.8 mm) minimum diameter.

- H. Rib Closure Strips: Manufacturer's standard vulcanized, closed-cell, synthetic rubber.
- I. Miscellaneous Roof Deck Accessories: Steel sheet, 0.0359-inch (0.91-mm) thick minimum ridge and valley plates, finish strips, and reinforcing channels, of same material as roof deck.
- J. Column Closures, End Closures, Z-Closures, and Cover Plates: Steel sheet, of same material and thickness as deck panels, unless otherwise indicated.
- K. Weld Washers: Manufacturer's standard uncoated-steel sheet weld washers, shaped to fit deck rib, 0.0598 inch (1.5 mm) thick with 3/8-inch (9.5-mm) minimum diameter prepunched hole.
- L. Recessed Sump Pans: Manufacturer's standard size, single piece steel sheet 0.071-inch-(1.8-mm-) thick minimum, of same material as deck panels, with 1-1/2-inch- (38-mm-) minimum deep level recessed pans and 3-inch- (76-mm-) wide flanges. Cut holes for drains in the field.
- M. Flat Receiver Pan: Manufacturer's standard size, single-piece steel sheet, 0.071-inch- (1.8-mm-) thick minimum units, of same material as deck panels. Cut holes for drains in the field.
- N. Shear Connectors: ASTM A 108, Grade 1010 through 1020 headed stud type, cold-finished carbon steel, AWS D1.1, Type B.
- O. Steel Sheet Accessories: ASTM A 446, G 60 (ASTM A 446M, Z 180) coating class, galvanized according to ASTM A 525 (ASTM A 525M).
- P. Galvanizing Repair Paint: SSPC-Paint 20 or DOD-P-21035, with dry film containing a minimum of 94 percent zinc dust by weight.

PART 3 - EXECUTION

3.1 **EXAMINATION:**

A. Examine supporting framing and field conditions for compliance with requirements for installation tolerances and other conditions affecting performance of steel deck.

3.2 **PREPARATION:**

- A. Do not place deck panels on concrete supporting structure until concrete has cured and is dry.
- B. Locate decking bundles to prevent overloading of supporting members.

3.3 INSTALLATION, GENERAL:

- A. Install deck panels and accessories according to applicable specifications and commentary of SDI Publication No. 28, manufacturer's recommendations, and requirements of this Section.
- B. Install temporary shoring before placing deck panels when required to meet deflection

limitations.

- C. Place deck panels on supporting framing and adjust to final position with ends accurately aligned and bearing on supporting framing before being permanently fastened. Do not stretch or contract side lap interlocks.
 - 1. Do not place deck units on concrete supporting structure until concrete has cured and is dry.
 - 2. Coordinate and cooperate with structural steel erector in locating decking bundles to prevent overloading of structural members.
 - 3. Do not use deck units for storage or working platforms until permanently secured.
 - 4. Place deck panels flat and square and fasten to supporting framing without warp or deflection.
 - 5. Cut and neatly fit deck panels and accessories around openings and other work projecting through or adjacent to the decking.
 - 6. Provide additional reinforcement and closure pieces at openings as required for strength, continuity of decking, and support of other work.
 - 7. Comply with AWS requirements and procedures for manual shielded metal arc welding, appearance and quality of welds, and methods used in correcting welding work.
 - 8. Mechanical fasteners may be used in lieu of welding to fasten deck. Locate mechanical fasteners and install according to deck manufacturer's instructions.
- D. Deck Edge Tolerances: Perimeter deck edges shall be within =/-1/2 inch of the indicated lines.

3.4 <u>ROOF DECK INSTALLATION:</u>

- A. Fasten roof deck panels to steel supporting members by arc spot (puddle) welds of the surface diameter indicated or arc seam welds with an equal perimeter, but not less than 1-1/2 inches (38 mm) long, and as follows:
 - 1. Comply with AWS requirements and procedures for manual shielded metal arc welding, appearance and quality of welds, and methods used in correcting welding work.
 - 2. Weld Diameter:
 - a. 5/8 inch (16 mm), nominal.
 - 3. Weld Spacing: Space and locate welds as indicated.
 - 4. Weld Washers: Install weld washers at each weld location.
 - 5. Side Lap and Perimeter Edge Fastening: Fasten side laps and perimeter edges of panels between supports, at intervals not exceeding 36 inches (910 mm), using one of the following methods:
 - a. Mechanically fasten with self-drilling No. 10- (4.8-mm-) diameter or larger carbon steel screws.
- B. End Bearing: Install deck ends over supporting framing with a minimum end bearing of 1-1/2 inches (38 mm), with end joints as follows:
 - 1. End Joints:
 - a. Lapped 2 inches (51 mm) minimum.

- C. Uplift Loading: Install and anchor roof deck units to resist gross uplift loading of 45 psf at eave overhang and 30 psf for other roof areas.
- D. Reinforcement at Openings: Provide additional metal reinforcement and closure pieces as required for strength, continuity of decking, and support of other work shown.
- E. Roof Sump Pans and Sump Plates: Install over openings provided in roof decking, and weld flanges to top of deck. Space welds not more than 12 inches (305 mm) apart with at least one weld at each corner.
- F. Miscellaneous Roof Deck Accessories: Install ridge and valley plates, finish strips, cover plates, end closures, and reinforcing channels according to deck manufacturer's recommendations. Weld to substrate to provide a complete deck installation.

3.5 FLOOR DECK INSTALLATION:

- A. Fasten floor deck panels to steel supporting members by arc spot (puddle) welds of the surface diameter indicated and as follows:
 - 1. Weld Diameter: 5/8 inch (16 mm), nominal.
 - 2. Weld Spacing: Weld edge ribs of panels at each support. Space additional welds an average of 12 inches (305 mm) apart, but not more than 18 inches (457 mm) apart.
 - 3. Weld Washers: Install weld washers at each weld location.
 - 4. Side Lap and Perimeter Edge Fastening: Fasten side laps and perimeter edges of panels between supports, or at intervals not exceeding 36 inches (910 mm), using one of the following methods:
 - a. Mechanically fasten with self-drilling No. 10- (4.8-mm-) diameter or larger carbon steel screws.
- B. End Bearing: Install deck ends over supporting framing with a minimum end bearing of 1-1/2 inches (38 mm), with end joints as follows:
 - 1. End Joints:
 - a. Lapped.
- C. Pour Stops and Girder Fillers: Weld steel sheet pour stops and girder fillers to supporting structure according to SDI recommendations, unless otherwise indicated.
- D. Floor Deck Closures: Weld steel sheet column closures, cell closures, and Z-closures to deck according to SDI recommendations to provide tight-fitting closures at open ends of ribs and sides of decking. Weld cover plates at changes in direction of floor deck panels, unless otherwise indicated.

3.6 FIELD QUALITY CONTROL:

- A. Quantity of Required Inspections and Tests: Testing Laboratory shall perform inspections or tests in accordance with AISC specification:
 - 1. Field welds and screwed deck attachment will be subject to Special Inspection

requirements per 2012 IBC. Refer to Special Inspection Schedule in Construction Documents.

- B. Testing Agency will report test results promptly and in writing to Contractor and Architect.
- C. Remove and replace work that does not comply with specified requirements.
- D. Additional testing will be performed to determine compliance of corrected work with specified requirements.

3.7 <u>REPAIRS AND PROTECTION:</u>

A. Galvanizing Repairs: Prepare and repair damaged galvanized coatings on both surfaces with galvanized repair paint according to ASTM A 780 and the manufacturer's instructions.

END OF STEEL DECK