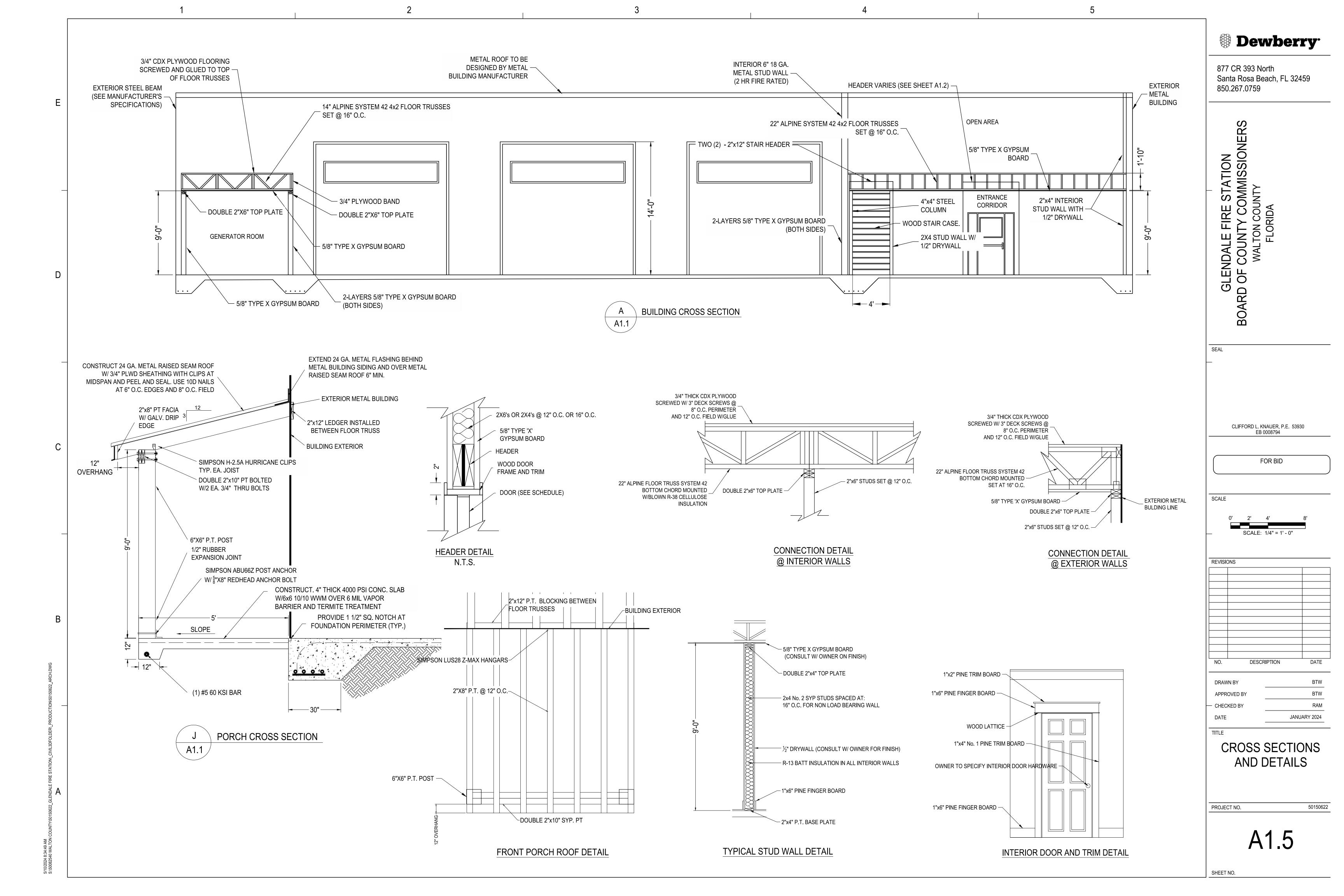
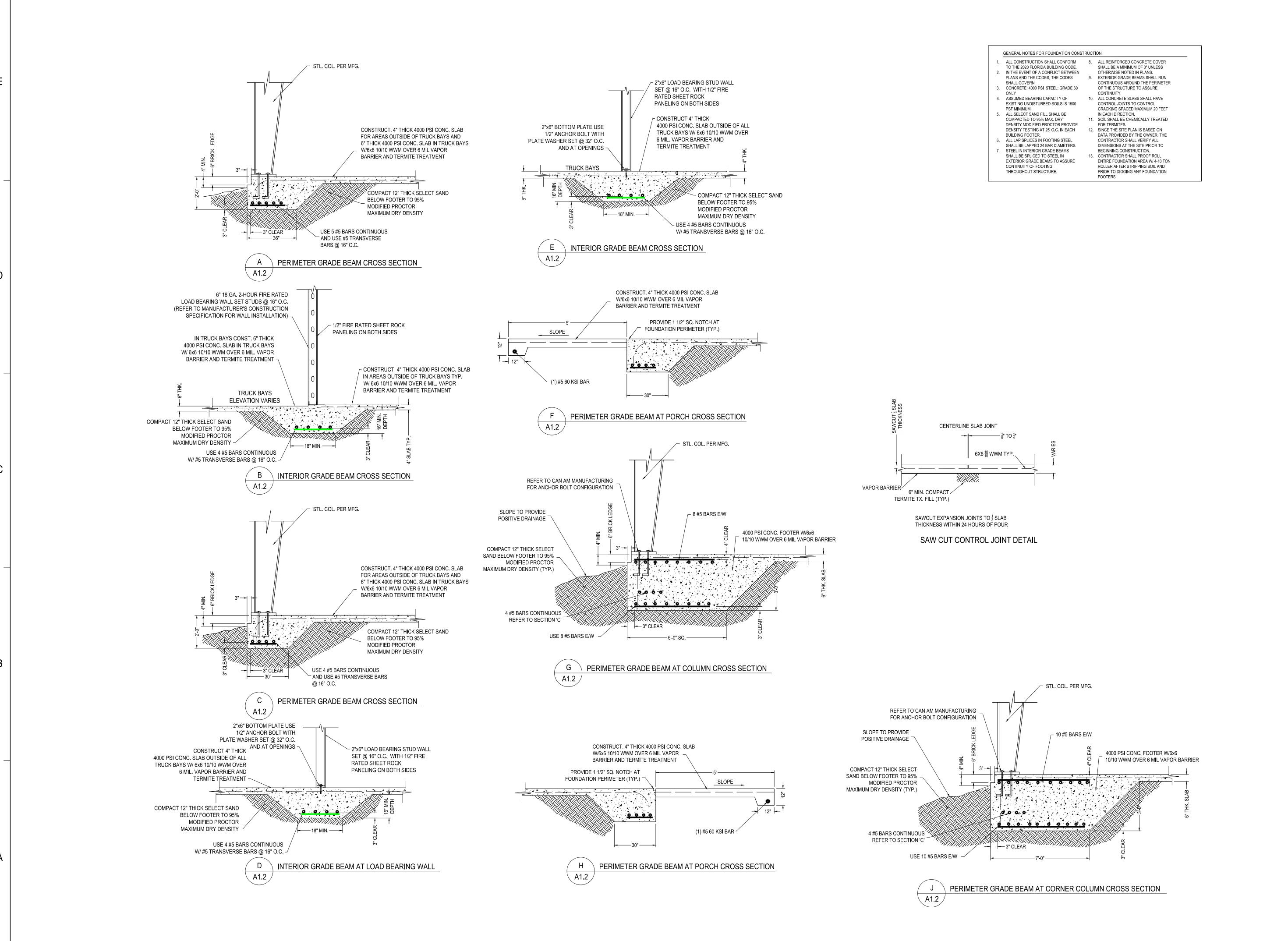


SHEET NO.





**Dewberry** 

877 CR 393 North Santa Rosa Beach, FL 32459 850.267.0759

GLENDALE FIRE STATION
RD OF COUNTY COMMISSIONERS
WALTON COUNTY
FLORIDA

SEAL CLIFFORD L. KNAUER, P.E. 53930 EB 0008794 FOR BID SCALE REVISIONS DESCRIPTION BTW DRAWN BY BTW APPROVED BY RAM CHECKED BY JANUARY 2024 DATE TITLE

A16

FOUNDATION

**DETAILS** 

50150622

SHEET NO.

PROJECT NO.

1. THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND IT'S FASTENINGS" AND FLORIDA BUILDING CODE 2010 EDITION, AND THE NATIONAL DESIGN STANDARD FOR METAL PLATE, CONNECTED WOOD TRUSS CONSTRUCTION.

2. IN ACCORDANCE WITH RULE 61G15-31.003 OF THE FLORIDA ADMINISTRATIVE CODE THE TRUSS SYSTEM ENGINEER, A DELEGATED ENGINEER, SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW TO ARCHITECT / ENGINEER FOR THE ASSEMBLAGE OF PREFABRICATED, ENGINEERED WOOD TRUSSES AND TRUSS GIRDERS, TOGETHER WITH ALL BRACING, CONNECTIONS AND OTHER STRUCTURAL ELEMENTS AND ALL SPACING AND LOCATION CRITERIA (TRUSS PLACEMENT PLAN), THAT, IN COMBINATION, FUNCTION TO SUPPORT THE DEAD, LIVE AND WIND LOADS APPLICABLE TO THE ROOF TRUSS SYSTEM, THE TRUSS SYSTEM DOES NOT INCLUDE WALLS, OR ANY OTHER STRUCTURAL SUPPORT SYSTEMS. THESE SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY THE TRUSS SYSTEM

3. IN ACCORDANCE WITH RULE 61G15-31.003 OF THE FLORIDA ADMINISTRATIVE CODE, THE TRUSS DESIGN ENGINEER, A DELEGATED ENGINEER, SHALL DESIGN THE INDIVIDUAL TRUSSES OF THE TRUSS SYSTEM, BUT DOES NOT DESIGN THE TRUSS SYSTEM. THE TRUSS DESIGN ENGINEER SHALL SUBMIT SHOP (PIECE) DRAWINGS AND CALCULATIONS FOR EACH DIFFERENT TRUSS AND TRUSS GIRDER THAT TOGETHER COMPRISE THE TRUSS SYSTEM. THESE SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY THE TRUSS DESIGN ENGINEER.

4. THE TRUSS SYSTEM ENGINEER AND THE TRUSS DESIGN ENGINEER SHALL EACH BE RESPONSIBLE FOR THEIR OWN WORK. HOWEVER, THEY MAY BE THE SAME INDIVIDUAL PROVIDING TWO SEPARATE SERVICES.

5. THE LOADS, LAYOUTS AND CONNECTIONS PROVIDED ON THE STRUCTURAL CONSTRUCTION DOCUMENTS ARE THE MINIMUMS TO BE FOLLOWED BY THE TRUSS SYSTEM ENGINEER AND THE TRUSS DESIGN ENGINEER.

6. USE STRESS-RATED TIMBER FOR ALL WOOD STRUCTURAL MEMBERS. USE WOOD STRUCTURAL MEMBERS WITH A MINIMUM BENDING STRESS OF 1200 PSI & MODULUS OF ELASTICITY NOT LESS THAN 1,400,000 PSI WHEN USED AT 19% MAXIMUM MOISTURE CONTENT.

7. PRESSURE TREATMENT OF ALL STRUCTURAL LUMBER SHALL BE IN ACCORDANCE WITH AWPA STANDARDS C1 AND C2, LATEST EDITIONS WITH A WATERBORNE PRESERVATIVE IN ACCORDANCE WITH STANDARD P5, LATEST EDITION. ALL LUMBER TO BE KILN-DRIED AFTER TREATMENT TO A MOISTURE CONTENT NOT TO EXCEED 19% OVEN-DRY BASIS, PER STANDARD C2. ALL LUMBER LESS THAN 4"x4" (NOMINAL SIZE) TO BE TREATED TO THE ABOVE GROUND REQUIREMENTS OF C2.

8. ANCHOR ROOF TRUSSES TO TOP PLATE W/ H-10 TRUSS CLIPS TYP.

9. SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW TO ARCHITECT/ENGINEER FOR PRE-FABRICATED, ENGINEERED WOOD TRUSSES SHOWING ALL MATERIALS, CONNECTIONS, ERECTION PROCEDURE, BRACING, BRIDGING, ATTACHMENT TO THE STRUCTURE, DESIGN LOADS (INCLUDING CONCENTRATED LOADS DUE TO EQUIPMENT, ETC.). SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED, BY OR UNDER THE SUPERVISION OF AND SIGNED AND SEALED BY A DELEGATED ENGINEER, SEE NOTES "SHOP DRAWINGS AND OTHER SUBMITTALS".

10. THE DESIGN AND ERECTION OF WOOD TRUSSES, INCLUDING PERMANENT BRACING, SHALL CONFORM TO THE COMMENTARY AND RECOMMENDATIONS OF THE TRUSS PLATE INSTITUTE. THESE CALCULATIONS ARE TO BE PROVIDED BY THE TRUSS MANUFACTURER AND SUBMITTED AS STATED PREVIOUSLY ON THIS DRAWING. IN ADDITION TO CONTINUOUS LATERAL BRACING OF TOP AND BOTTOM CHORDS (DESIGNED BY DELEGATED ENGINEER BUT SPACED NOT MORE THAN 10'-0" O.C.). PROVIDE DIAGONAL BRACING (MIN. 2" THICK NOMINAL LUMBER) AS

A. IN THE PLANE OF THE TOP CHORD — LOCATE BETWEEN LATERAL BRACING. SET AT 45° ANGLES.REPEAT AT MAX 20'-0" INTERVALS.

B. IN THE PLANE OF THE WEB MEMBERS (PERPENDICULAR TO TRUSSES) — AT EACH WEB MEMBER REQUIRING CONTINUOUS LATERAL BRACING BUT NOT MORE THAN 16'-0" INTERVALS, SPACING BETWEEN SETS OF DIAGONALS SHALL NOT EXCEED 20'-0" OR TWICE THE HORIZONTAL RUN AT THE DIAGONAL.

C. IN THE PLANE OF THE BOTTOM CHORD — PLACE BETWEEN CONTINUOUS LATERAL BRACING AT 45° ANGLES AT EACH END OF BUILDING.

D. ANCHOR ALL DIAGONAL BRACING TO REINFORCED MASONRY WALLS OR REINFORCED CONCRETE MEMBERS WITH PRE-FABRICATED (MIN. 12 ga) GALVANIZED STEEL STRAPS OR FRAMING CONNECTORS. FASTEN STRAPS TO MASONRY WITH 2 1/2" MASONRY ANCHORS OR (4) .17" Øx1 1/2" POWDER DRIVEN PINS IF INTO CONCRETE AND TO WOOD MEMBERS WITH NOT LESS THAN (6) 16d NAILS.

11. MINIMUM DESIGN LOADS FOR ROOF TRUSSES:
50 PSF LIVE LOAD TOP CHORD
10 PSF DEAD LOAD TOP CHORD
5 PSF DEAD LOAD BOTTOM CHORD
SEE ROOF FRAMING PLAN FOR NET WIND UPLIFT LOAD & WIND PRESSURES BASED ON ASCE7.
THE WEIGHT OF ROOFING & CEILING MAY NOT BE USED TO REDUCE WIND UPLIFT LOADS.

WOOD FRAMING & SHEATHING

1. WOOD CONSTRUCTION SHALL COMPLY WITH AF & PA NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION AND CHAPTER 23 OF FBC.

2. MANUFACTURED LUMBER, S4S AND GRADESTAMPED, TO COMPLY WITH PS20 AND APPLICABLE GRADING RULES OF INSPECTION AGENCIES CERTIFIED BY ALSC'S BOARD OF REVIEW.

3. PROVIDE SEASONED LUMBER WITH 19% MOISTURE CONTENT AT THE TIME OF DRESSING AND SHIPMENT, FOR SIZES 2" OR LESS IN THICKNESS.

4. COMPLY WITH "DESIGN SPECIFICATION FOR METAL PLATE CONNECTED ROOF TRUSSES".

5. ALL STRUCTURAL LUMBER TO BE MIN SOUTHERN YELLOW PINE NO. 2 S4S, GRADESTAMPED. ALL OTHER LUMBER PER ARCHITECTURAL SPECIFICATIONS.

6. USE LUMBER WITH MINIMUM Fb 1050 / 1200 PSI FOR SINGLE/REPETITIVE USE AND A MOULES OF ELASTICITY, "E" OF 1,200,000 PSI (FOR WALLS AND BEAMS). PARALLEL STRAND LUMBER SHALL HAVE MINIMUM Fb 2900 PSI, Ex2,000,000 PSI & Fv x 290 PSI. EXTERIOR STUD WALLS TO BE CONSTRUCTED USING TIMBERSTRAND LSL.

7. AT ALL BUILT-UP WOOD AND BEAM BEARING LOCATIONS A MINIMUM OF (4) 2x6 STUD MEMBERS TO BE USED U.O.N.

8 MICRO-LAM LVL BEAMS USED AS MULTIPLE ASSEMBLY BEAMS TO BE CONNECTED WITH 3 ROWS OF 16d NAILS @ 12" O.C.

9. ONE PIECE OF 3 1/2" THICK MICRO-LAM LVL MAY BE SUBSTITUTED FOR TWO PIECES OF 1 3/4".

10. THE INDIVIDUAL STUDS OF BUILT-UP COLUMNS TO BE ADEQUATELY FASTENED TO DEVELOP COMPOSITE ACTION OF THE ASSEMBLY.

11. DO NOT SPLICE STRUCTURAL MEMBERS BETWEEN SUPPORTS, U.O.N.

12. AT ALL BUILT-UP WOOD COLUMNS USE (2) HUGHES AS 5B ANCHOR TIE DOWNS OR EQUIV. AT BASE AND AT TOP OF BUILT-UP COLUMN. BEAMS OR TRUSS GIRDERS BEARING ON BUILT-UP COLUMNS TO BE ANCHORED W/WWUC WOOD TO WOOD UPLIFT CONNECTOR.

13. USE MANUFACTURERS REQUIRED SIZE AND NUMBER OF NAILS OR BOLTS FOR ANCHOR TIE DOWNS, HURRICANE CLIPS AND ALL CONNECTORS.

14. SECURELY ATTACH CARPENTRY WORK TO SUBSTRATES AND SUPPORTING MEMBERS USING FASTENERS OF SIZE THAT WILL NOT PENETRATE MEMBERS WHERE THE OPPOSITE SIDE WILL BE EXPOSED TO VIEW OR RECEIVE FINISH MATERIALS.

15. 3/4" PLYWOOD ROOF SHEATHING TO BE APA RATED 32/16 EXPOSURE I NAILED TO SUPPORTING MEMBERS WITH 10d NAILS @ 8" O.C. AND 4" O.C. ALONG THE EDGES. PROVIDE 1/16"SPACE AT END JOIST AND 1/8" AT EDGE JOINTS. PROVIDE PLY CLIPS ALONG EDGE JOINT AT MID SPAN BETWEEN SUPPORTS

16. 3/4" PLYWOOD WALL SHEATHING TO BE NAILED TO WALL STUDS WITH 10d NAILS @ 8" O.C. AND 4" O.C. ALONG THE EDGES WITH CLIPS.

17. GYPSUM WALL SHEATHING AT EXTERIOR WALLS, LOAD BEARING WALLS AND INTERIOR WALLS INTERSECTING WALLS TO BE BLOCKED AND SECURED TO STUDS WITH DRYWALL NAILS AT 7" O.C. AND 4" O.C. ALONG THE EDGES.

18. PROVIDE CONT. LATERAL BRACING AND/OR BLOCKING BETWEEN CHORDS OF TRUSSES AS REQUIRED OR AS NOTED BY TRUSS MANUFACTURER TO ADEQUATELY TRANSFER LOADS TO

19. CONTRACTOR TO VERIFY THAT HURRICANE CLIPS/TIE DOWNS SHOWN WILL RESIST WIND UPLIFT FROM ROOF TRUSSES. IF UPLIFT VALUE FROM ROOF TRUSS MANUFACTURER EXCEEDS CAPACITY OF THE HURRICANE CLIPS/TIE DOWN, THE CONTRACTOR SHALL PROVIDE A CONNECTOR TO SAFELY RESIST THE UPLIFT LOADS.

20. PLACE FLAT STRAPPING BETWEEN STUDS AT ENDS OF ALL BEAM BEARING LOCATIONS, W/
1 1/4"x30"x16 GA FLAT STRAP W/ (24) 10d NAILS TO BE PLACED AT ENDS OF BUILT-UP WOOD
BEAMS

21. ANCHOR BOLTS TO BE 1/2"x10" LONG W/ 2" SQ. WASHERS & SPACED 24" O.C. U.O.N. FIRST ANCHOR BOLT IS TO BE PLACED MAX 4" FROM EACH CORNER.

22. EXTERIOR END WALLS AT VAULTED OR CATHEDRAL CEILING LOCATIONS TO BE BALLOON FRAMED FROM SILL PLATE TO ROOF DIAPHRAGM.

23. PROVIDE THREADED ROD TIE-DOWN SYSTEM AT LOCATIONS NOTED ON PLANS.

24. ALL PRESSURE TREATED LUMBER TO BE ALKALINE COPPER QUAT (ACQ) TREATED, KILN DRIED AND CONNECTED WITH STAINLESS STEEL FASTENERS.

25. ALL CONNECTORS AND PLATES SHALL BE STAINLESS STEEL U.O.N. TRUSS PLATES AT INTERIOR LOCATIONS MAY BE GALVANIZED.

26. AT ENDS OF ALL SHEAR WALLS PLACE MIN. (3) BUILT-UP STUD GROUP. INSTALL SIMPSON HD101 HOLDDOWN ANC. AT BUILT-UP STUD GROUP ENDS OF SHEAR WALLS TO PROVIDE CONTINUOUS LOAD PATH FROM FRAMING LEVEL TO PRE-CAST BEAM LEVEL.

27. EXTERIOR WALLS TO BE FRAMED WITH TIMBERSTRAND LSL WALL STUDS AT ALL LOCATIONS. PROVIDE 1 ROW OF TIMBERSTRAND LSL BLKG. FOR WALLS UP TO 14' HIGH AND 2 ROWS FOR WALLS GREATER THAN 14' HIGH, NAIL STUDS TO PLATES W/ (3) 16d (3 1/2") END NAILS. 2x6 MEMBERS @ 12" O.C. TO BE USED FOR WALLS UP TO 14'-0". FOR END WALLS AND GABELS PROVIDE 3x6 MEMBERS @ 12" O.C.

28. THE ENGINEER SHOULD BE NOTIFIED OF ANY DEVIATIONS FROM THE PLANS OR TRUSS SHOP

29. PERMANENT TRUSS BOTTOM CHORD LATERAL BRACING, CONSISTING OF 2x4 GRADE MARKED LUMBER, NAILED W/ MIN. (2) 16d NAILS PER TRUSS AND LAPPED AT LEAST TWO TRUSSES, SHALL BE SPACED NO GREATER THAN 15'-0".

30. PRE-ENGINEERED TRUSS BRACING PER TRUSS MANUFACTURER TO BE A MINIMUM OF 2x4 OF THE SAME SPECIES AS TRUSS.

31. CEILING TO BE A MINIMUM OF 1/2" GYPSUM WITH 5d COOLER NAILS OR GWB-54 1 1/2" NAILS INSTALLED AT 10" O.C. AND 7" ALONG EDGES.

32. ALL TIMBER PRODUCTS SHALL BE INSPECTED IN ACCORDANCE WITH SECTION 951 AND SHALL MEET SPECIFICATIONS UNDER SECTION 952, 953, 954, AND 955 OF THE 2013 FDOT STANDARD SPECIFICATIONS.

33. CONTRACTOR MAY USE ALTERNATE STRAP AND ANCHOR MANUFACTURER FOR CONNECTIONS AND SHALL SUBMIT SHOP DRAWINGS TO STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.

CONCRETE MASONRY

1. CONSTRUCT MASONRY IN ACCORDANCE WITH SPECIFICATION SECTIONS 04200 AND 04230, ACI 530/ASCE 5, "BUILDING CODE REQUIREMENTS FOR CONCRETE M530.1 / ASCE 6,"SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD BEARING CONCRETE MASONRY".

2. THE STRUCTURE IS SUPPORTED BY BEARING WALLS, U.O.N. ERECT MASONRY PRIOR TO CASTING CONCRETE COLUMNS WITHIN BEARING WALLS OR CASTING BEAMS AND SLABS SUPPORTED BY BEARING WALLS.

3. USE 50% SOLID, NOMINAL 8X8X16, CONCRETE MASONRY UNITS CONFIRMING TO ASTM C90. LAY UP UNITS IN RUNNING BOND. SAWCUT UNITS WHICH ARE NOT IN MULTIPLES OF 8". UNITS SHALL BE AT LEAST 8" LONG. BAND CORNERS BY LAPPING ENDS 8" IN SUCCESSIVE VERTICAL COURSES. DESIGN OF WALLS IS BASED ON A F'm OF 1500 PSI.

4. USE TYPE S MORTAR IN ACCORDANCE WITH ASTM C270 EXCEPT USE TYPE M MORTAR BELOW GRADE. HEAD AND BED JOINTS SHALL BE 3/8" FOR THE THICKNESS OF THE FACE SHELL. WEBS ARE TO BE FULLY MORTARED IN ALL COURSES OF PIERS, COLUMNS AND PILASTERS, IN THE STARTING COURSE, AND WHERE AN ADJACENT CELL IS TO BE GROUTED. REMOVE MORTAR PROTRUSIONS EXTENDING 1/2" OR MORE INTO CELLS TO BE GROUTED.

5. USE STANDARD (9 GAGE) HORIZONTAL JOINT REINFORCING CONFORMING TO ASTM A-82 IN EVERY OTHER COURSE. OVERLAP DISCONTINUOUS ENDS 6". USE PREFABRICATED CORNERS. USE TRUSS TYPE, EXCEPT USE LADDER TYPE IN WALLS WITH VERTICAL REINFORCING. EXTEND JOINT REINFORCING A MINIMUM OF 4" INTO TIE COLUMNS.

6. USE FINE GROUT CONFORMING TO ASTM C-476, WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI IN 28 DAYS. AGGREGATE TO CONFORM TO ASTM C404 FOR FINE GROUT, WITH SLUMP OF 8" TO 10". GROUT ALL MASONRY CELLS. ALLOW MORTAR TO CURE 24 HOURS PRIOR TO GROUTING. PROVIDE CLEANOUT OPENINGS AT THE BASE OF CELLS CONTAINING REINFORCING STEEL TO CLEAN THE CELL AND TO TIE THE VERTICAL BAR TO THE DOWEL. IN HIGH-LIFT GROUTING, USE 5'-0" (MAXIMUM) LIFTS, WITH 1/2" HOUR TO 1 HOUR BETWEEN LIFTS. VIBRATE EACH LIFT AND RECONSOLIDATE THE PREVIOUS LIFT.

7. USE ASTM A-615 GRADE 60 REINFORCING STEEL. REINFORCE WALLS WHERE INDICATED ON THE DRAWINGS AND AT ALL INTERSECTIONS, EACH SIDE OF OPENINGS AND AT THE ENDS OF WALLS. USE BAR SPACERS AT 10'-0" O.C. WHERE GROUT POUR HEIGHT EXCEEDS 10'-0".

8. AT BOND/TIE BEAM CORNERS AND INTERSECTIONS, PLACE 1 #5 X 5'-0" T & B CORNER BAR,

WITH 30" LEGS EACH WAY, AT THE EXTERIOR FACE.

9. BEAMS NOT SCHEDULED ARE MINIMUM 8" X 12" TIE BEAMS WITH 2 #5 BARS TOP AND BOTTOM AND #3 TIES SPACED AT 48" O.C. TYPICAL AND 4 TIES AT 12" O.C. AT ENDS AND INTERSECTIONS, U.O.N. COLUMNS NOT SCHEDULED ARE MINIMUM 8" X 12" TIE COLUMNS WITH 4 #5 VERTICAL BARS AND #2 TIES AT 12" O.C. USE 30" LAP SPLICES. HOOK ALL BARS AT DISCONTINUOUS ENDS.

10. REINFORCED MASONRY WALL CONSTRUCTION SHALL BE INSPECTED BY AN ENGINEER OR ARCHITECT IN ACCORDANCE WITH ACI 530.1/ASCE 6.

11. WHERE ANCHOR BOLTS, WEDGE ANCHORS OR ANCHORS SET IN EPOXY ARE SET IN A MASONRY WALL, FILL CELLS WITH GROUT FOR BOLTED COURSE, ONE COURSE ABOVE AND TWO COURSES BELOW.

12. PROVIDE LINTELS OR HEADERS WITH MINIMUM 8" BEARING OVER ALL MASONRY OPENINGS.

13. USE PRESSURE TREATED WOOD FOR WOOD IN CONTACT WITH MASONRY.

14. ALL CELLS SHALL BE SOLID FILLED W/ GROUT CONFORMING TO ASTM C-476.

15. ALL P.T. FURRING SHALL BE ANCHORED W/ 2 1/2" TAPCON SCREWS @ 16" O.C. TYP.

SHALLOW FOUNDATIONS:

1. ALL CONSTRUCTION SHALL CONFORM TO THE 2017 FLORIDA BUILDING CODE.

2. IN THE EVENT OF A CONFLICT BETWEEN PLANS AND THE CODES, THE CODES SHALL GOVERN.

3. CONCRETE: 3500 PSI STEEL: GRADE 60 ONLY

4. FOUNDATION DESIGN, SOIL PREPARATION AND COMPACTION ARE ASSUMED 1500 PSI BEARING CAPACITY.

5. ALL SELECT SAND FILL SHALL BE COMPACTED TO 95% MAX. DRY DENSITY MODIFIED PROCTOR PROVIDE DENSITY TESTING AT 25' O.C. IN EACH BUILDING FOOTER.

6. ALL LAP SPLICES IN FOOTING STEEL SHALL BE LAPPED 24 BAR DIAMETERS.

7. STEEL IN INTERIOR GRADE BEAMS SHALL BE SPLICED TO STEEL IN EXTERIOR GRADE BEAMS TO ASSURE CONTINUITY OF FOOTING THROUGHOUT STRUCTURE.

8. ALL REINFORCED CONCRETE COVER SHALL BE A MINIMUM OF 3" UNLESS OTHERWISE NOTED ON PLANS.

9. EXTERIOR GRADE BEAMS RUN CONTINUOUS AROUND THE PERIMETER OF THE STRUCTURE TO ASSURE CONTINUITY.

10. ALL CONCRETE SLABS SHALL HAVE CONTROL JOINTS TO CONTROL CRACKING SPACED MAXIMUM 25 FEET IN EACH DIRECTION.

11. SOIL SHALL BE CHEMICALLY TREATED FOR TERMITES.

12. SINCE SITE PLAN IS BASED ON DATA PROVIDED BY THE OWNER, THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AT THE SITE PRIOR TO BEGINNING CONSTRUCTION.

CHEMICAL ADHESIVE FOR ANCHORING REINFORCING BOLTS, THREADED BARS & ANCHOR BOLTS

1. USE AN EPOXY, ACRYLIC OR POLYESTER RESIN ADHESIVE SYSTEM SUCH AS THE POWERS RAWL POWER-FAST SYSTEM, HILTI HIT HY150, ITW RAMSET/RED HEAD EPCON AT OR C6 INJECTION SYSTEM, ALLIED FASTERNER ALLIED GOLD A-1000, OR ACCEPTED EQUIVALENT. FOLLOW MANUFACTURER'S SPECIFICATIONS FOR USE AND INSTALLATION.

2. CONFIRM THE ABSENCE OF REINFORCING STEEL BY DRILLING A 1/4" Ø PILOT HOLE FOR EACH ANCHOR. DO NOT CUT REINFORCING STEEL WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.

3. DRILL 1/16" LARGER Ø HOLE THAN ANCHOR BOLT AND 1/8" LARGER HOLE THAN REINFORCING BAR. THOROUGHLY CLEAN HOLE INCLUDING REMOVAL OF DUST PRIOR TO FILLING WITH EPOXY.

4. PROVIDE ANCHOR EMBEDMENT, SPACING AND EDGE DISTANCE AS SHOWN ON THE DRAWINGS.

**Dewberry** 

877 CR 393 North Santa Rosa Beach, FL 32459 850.267.0759

GLENDALE FIRE STATION
SOARD OF COUNTY COMMISSIONERS
WALTON COUNTY
FLORIDA

CLIFFORD L. KNAUER, P.E. 53930

EB 0008794

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SCALE

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	REVISIONS				
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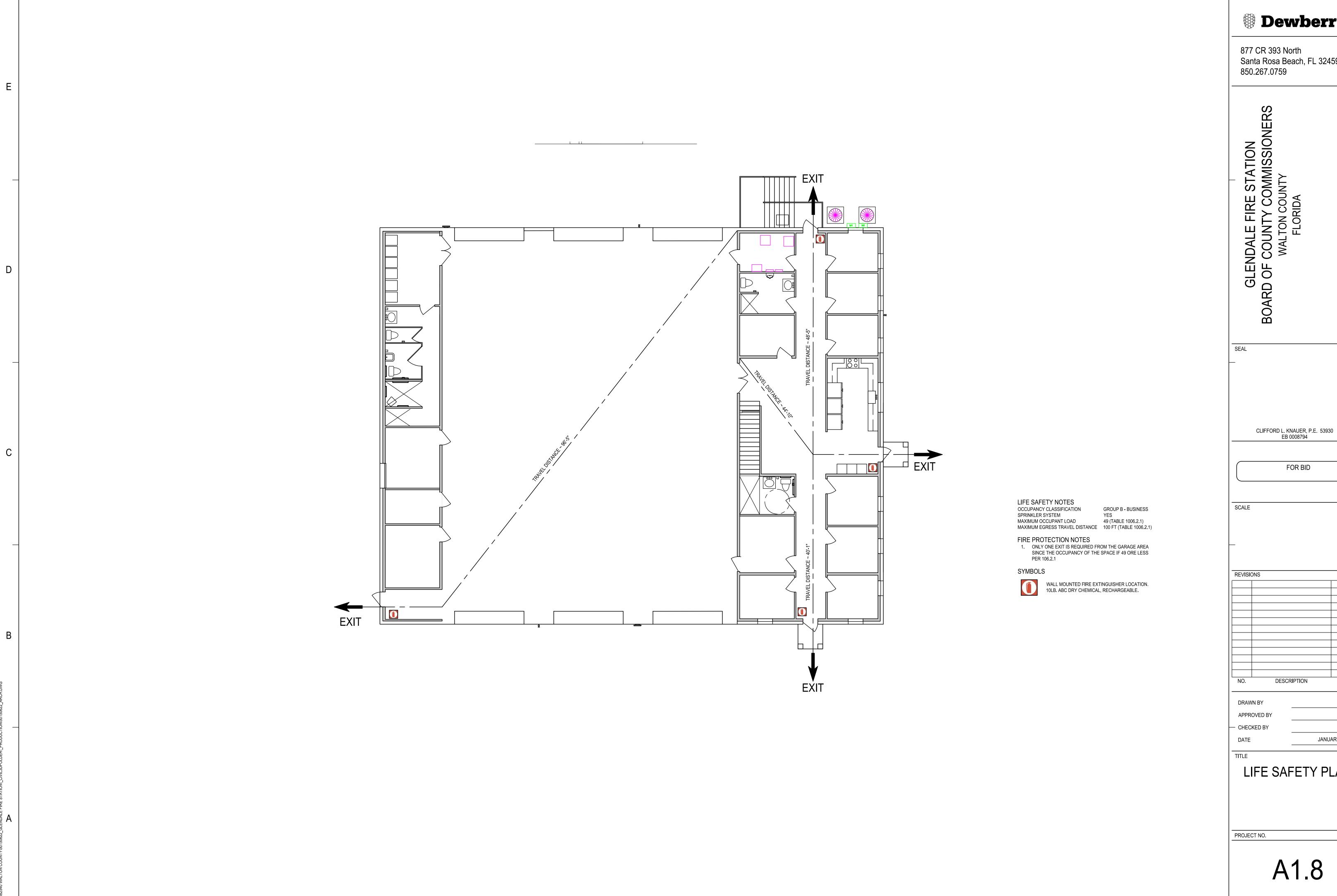
**GENERAL NOTES** 

50150622

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SHEET NO.

PROJECT NO.



**Dewberry** 

877 CR 393 North Santa Rosa Beach, FL 32459

DESCRIPTION

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JANUARY 2024

50150622

LIFE SAFETY PLAN

SHEET NO.