NEW ELEVATOR FOR LOCUST FORK ELEMENTARY SCHOOL

BLOUNT COUNTY BOARD OF EDUCATION

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STRUCTURAL ENGINEER

OWNER

- STRUCTURAL DESIGN GROUP, INC. 700 CENTURY PARK SOUTH, SUITE 114 BIRMINGHAM, AL 35226 205-824-5200

ARCHITECT

- LATHAN ASSOCIATES ARCHITECTS, P.C. 300 CHASE PARK SOUTH, SUITE 200 HOOVER, ALABAMA 35244 205-988-9112 CONTACT: RYAN VERNON

PLUMBING ENGINEER WHORTON ENGINEERING, INC.

P.O. BOX 5190 ANNISTON, ALABAMA 36205 256-820-9897

ELECTRICAL ENGINEER

- STEWART ENGINEERING P.O. BOX 2233 ANNISTON, ALABAMA 36202 256-237-0891

DRAWING INDEX - ELEVATOR PROJECT

(2 SHEETS)

(1 SHEET)

(2 SHEETS)

GENERAL DRAWINGS

T1 - TITLE AND INDEX LS1.0 - LIFE SAFETY PLAN

ARCHITECTURAL DRAWINGS (6 SHEETS)

A1.1 - ARCHITECTURAL SITE PLAN - DEMOLITION PLANS, ELEVATIONS AND PHOTOS

A2.1 - FLOOR PLANS

- BUILDING ELEVATIONS AND BUILDING SECTIONS

- ELEVATOR SECTIONS A3.3 - ENLARGED DETAILS

STRUCTURAL DRAWINGS (6 SHEETS)

S1.0 - GENERAL NOTES

S1.1 - GENERAL NOTES CONTINUED

S1.2 - TYPICAL DETAILS S1.3 - TYPICAL DETAILS

- FOUNDATION UPPER FRAMING AND ROOF FRAMING PLAN

S3.1 - SECTIONS AND DETAILS

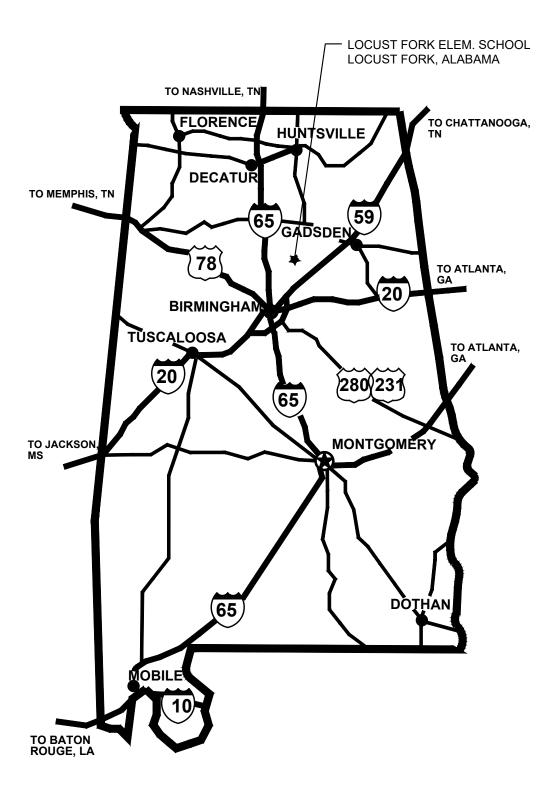
PLUMBING DRAWINGS

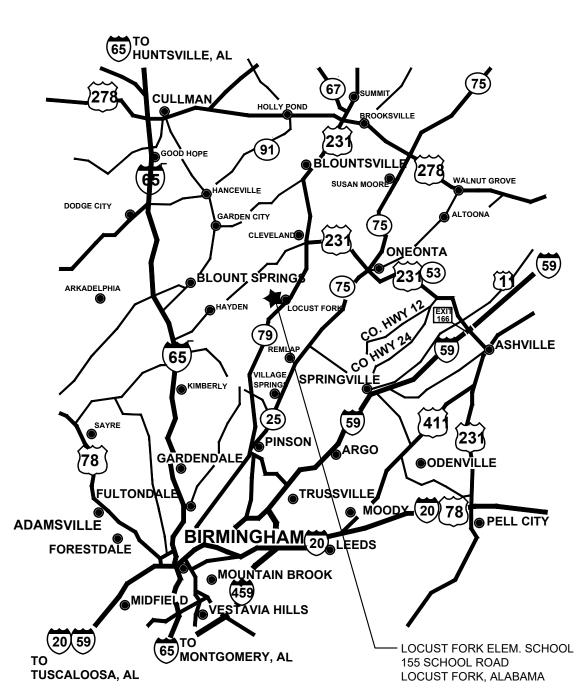
P1.1 - MAIN FLOOR PLUMBING PLAN

ELECTRICAL DRAWINGS

E1.1 - SCHEDULES, SYMBOLS, AND NOTES

E2.1 - ELECTRICAL PLAN

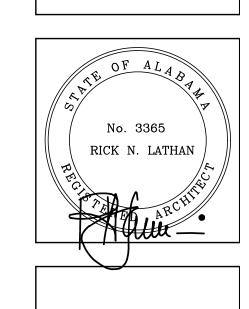


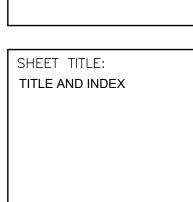






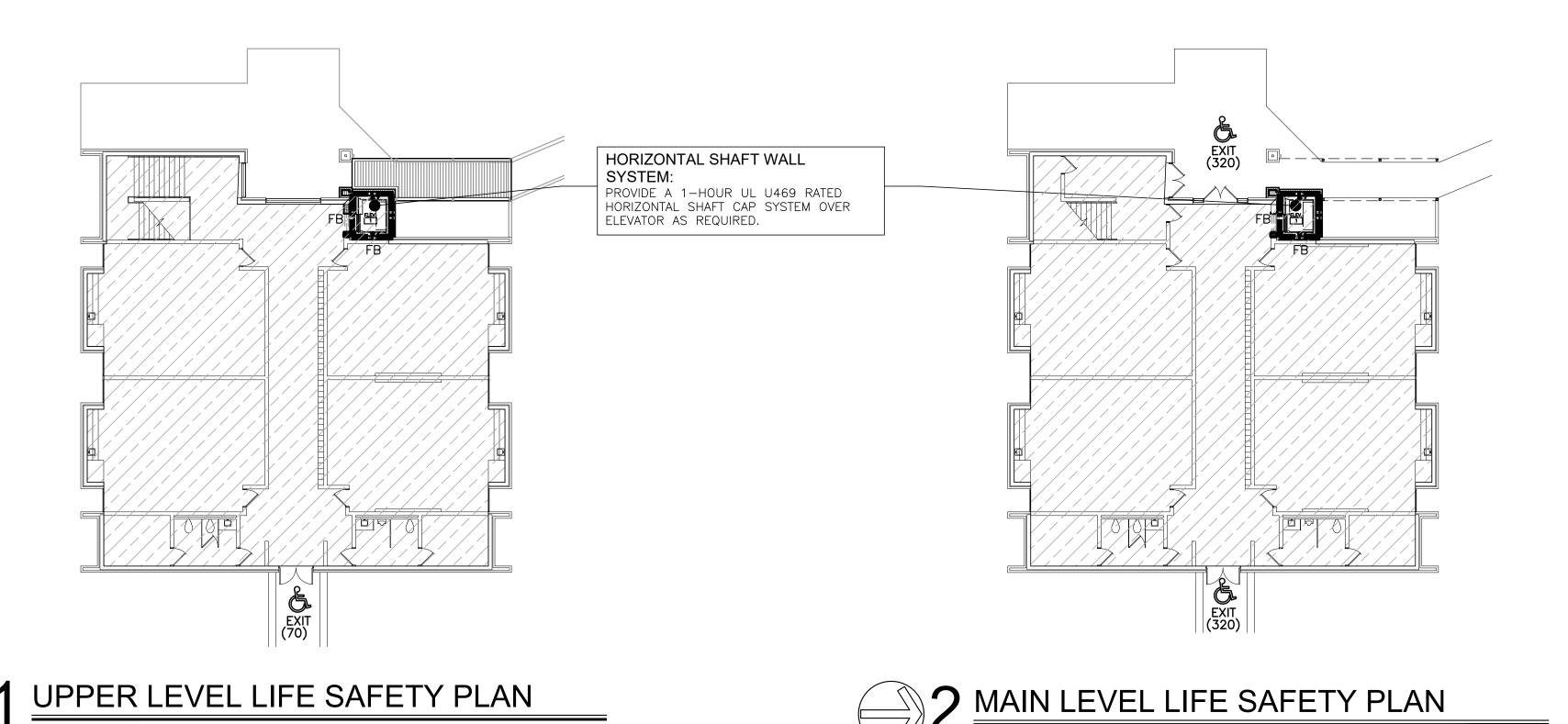






PROJ. MGR.:	Ryan Vernon
DRAWN:	PPh
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DATE:	8-28-2024
REVISIONS	

JOB NO. **24-39** SHEET NO: 1 OF 2

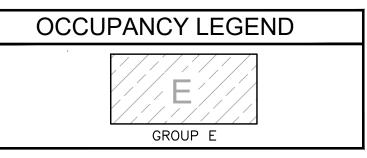


1/16" = 1'-0"

THE EGRESS CAPACITIES ARE UNCHANGED BY THE SCOPE OF THIS ADDITION

THE OCCUPANT LOAD IS UNCHANGED BY THE SCOPE OF THIS ADDITION

2021 INTERNATIONAL BUILDING LOCUST FORK ELEMENTA		NRC	H		
OCCUPANCY CLASSIFICATION:	GROUP E				
TYPE OF CONSTRUCTION :	TYPE I	IB UNS	SPRINKLERE	D	
EXISTING MAIN LEVEL AREA	5,13	33 S.F			
ADDITION AREA	-	73 S.F	.		
EXISTING UPPER LEVEL AREA	5,13	33 S.F	•		
TOTAL BUILDING AREA	10,33	39 S.F	•		
TABLE 504.4 ALLOWABLE NUMBER OF STORIES:	ALLOWABLE STORIES:	ACT	UAL STORIE 2	S:	
TABLE 506.2 ALLOWABLE AREA:	AREA FACTOR: NS	1	4,500 S.F.		
SECTION 506.3.3 FRONTAGE INCREASE :	FRONTAGE INCREASE	NOT	REQUIRED		
TABLE 601 AND 602	CONSTRUCTION TYPE:		IIB		
FIRE—RESISTANCE RATING REQUIREMENTS FOR BUILDING ELEMENTS:	STRUCTURAL FRAME:		0		
	BEARING WALLS:		0		
	T. 602 EXTER	RIOR:	< 5'	1hr	
			≥ 5'< 10' ≥ 10'< 30'	1hr 0	
		-	≥ 10 < 30 ≥ 30'	10	
	INTERIOR:	•	_	0	
	NONBEARING WALLS:				
	T. 602 EXTE	RIOR:	< 5'	1hr	
			≥ 5'< 10'	1hr	
			≥ 10'< 30' ≥ 30'	0	
	INTERIOR:		0	1 0	
	FLOOR CONSTRUCTIO	N:	0		
	ROOF CONSTRUCTION	I:	0		
TABLE 1020.1 CORRIDOR FIRE—RESISTANCE RATING PARTITIONS AND OPENING PROTECTIVES	GROUP E UNSPRINKLEREI	D	0		



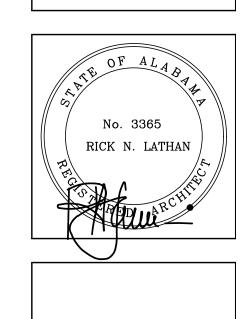
LIFE SAFETY NOTES							
FIRE EXTINGUISHER AND CABINET (PROVIDE FIRE RATED CABINETS IN RATED WALLS.)							
FIRE EXTINGUISHER	ACCESSIBLE						
K-TYPE FIRE FE(K) EXTINGUISHER	EXIT——EXIT (320)——EXIT CAPACITY						
EXTEND AND KEY ALL RATED AND/OR BOTTOM OF ROOF A	WALLS TO SHAFT WALL SYSTEM, ASSEMBLY						
STENCIL LABEL ALL RATED WA ABOVE CEILING EACH SIDE @							
ALL RATED DOORS AND FRAM EMBOSSED LABELS INDICATING	ES TO BE LABELED WITH						
COORDINATE W/ ELECTRICAL CONCRETE EQUIPMENT PAD AS							
HE — HORIZONTAL EXIT							
FB – FIRE BARRIER							
FP - FIRE PARTITION							
FW — FIRE WALL							
SM - SMOKE PARTITION							

DOOR/WINDOV	V RATING LEGEND
20 MINUTE DOOR	60 MINUTE DOOR
AND FRAME	AND FRAME
45 MINUTE DOOR	90 MINUTE DOOR
AND FRAME	90 AND FRAME
60 MINUTE DOOR AND FRAME	180 MINUTE DOOR AND FRAME

WALL TYPE LEGEND				
	1 HR WALL 2 HR WALL 3 HR WALL			
S-S-S-S-S-S-S-S-S-S- D-D-D-D-D-D-D-D-D-D				



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SHEET TITLE:
LIFE SAFETY PLAN

PROJ. MGR.: **Ryan Vernon** DRAWN:

hdrDATE: 8-28-2024

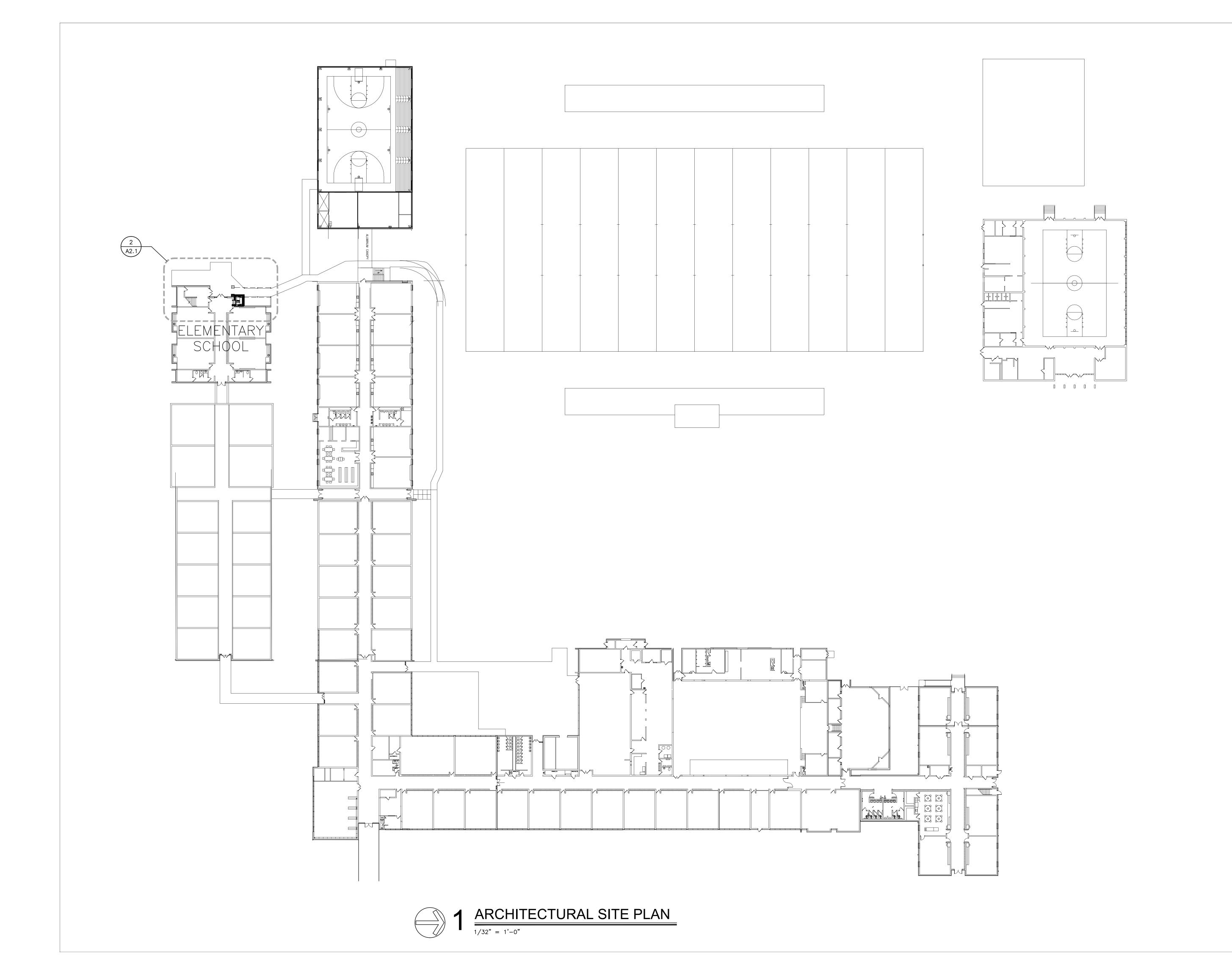
REVISIONS

JOB NO. **24-39**

SHEET NO:

LS1.0

0 1" 2'



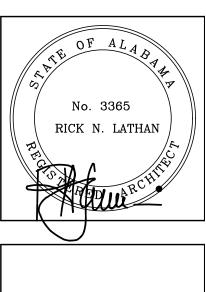


TOR ENTARY SCHOOL ork, AL 35097

NEW ELEVATOR FOR

LOCUST FORK ELEMENTARY S

155 School Road Locust Fork, AL 35097

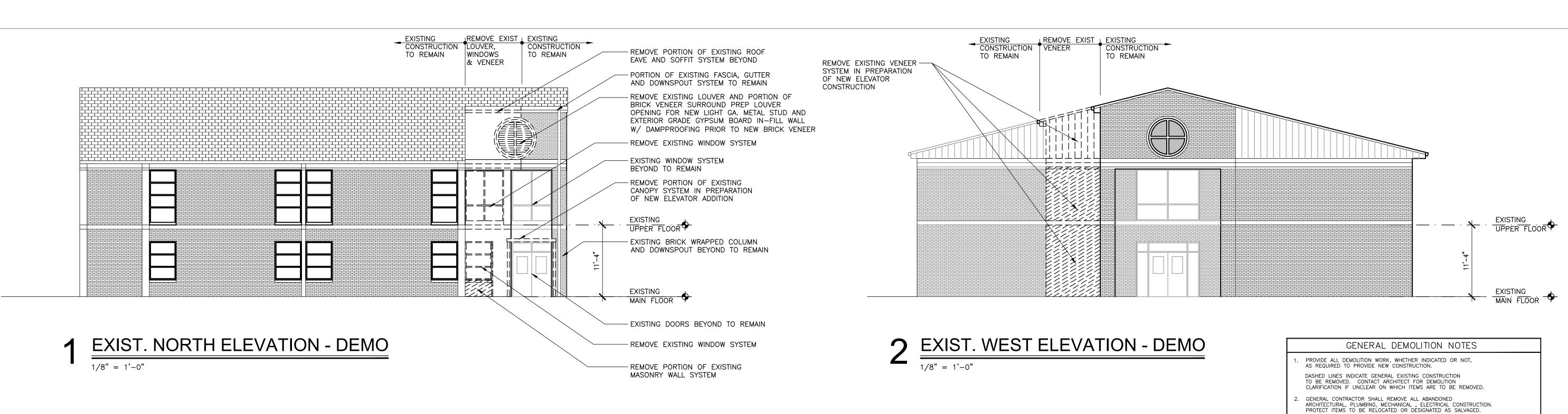


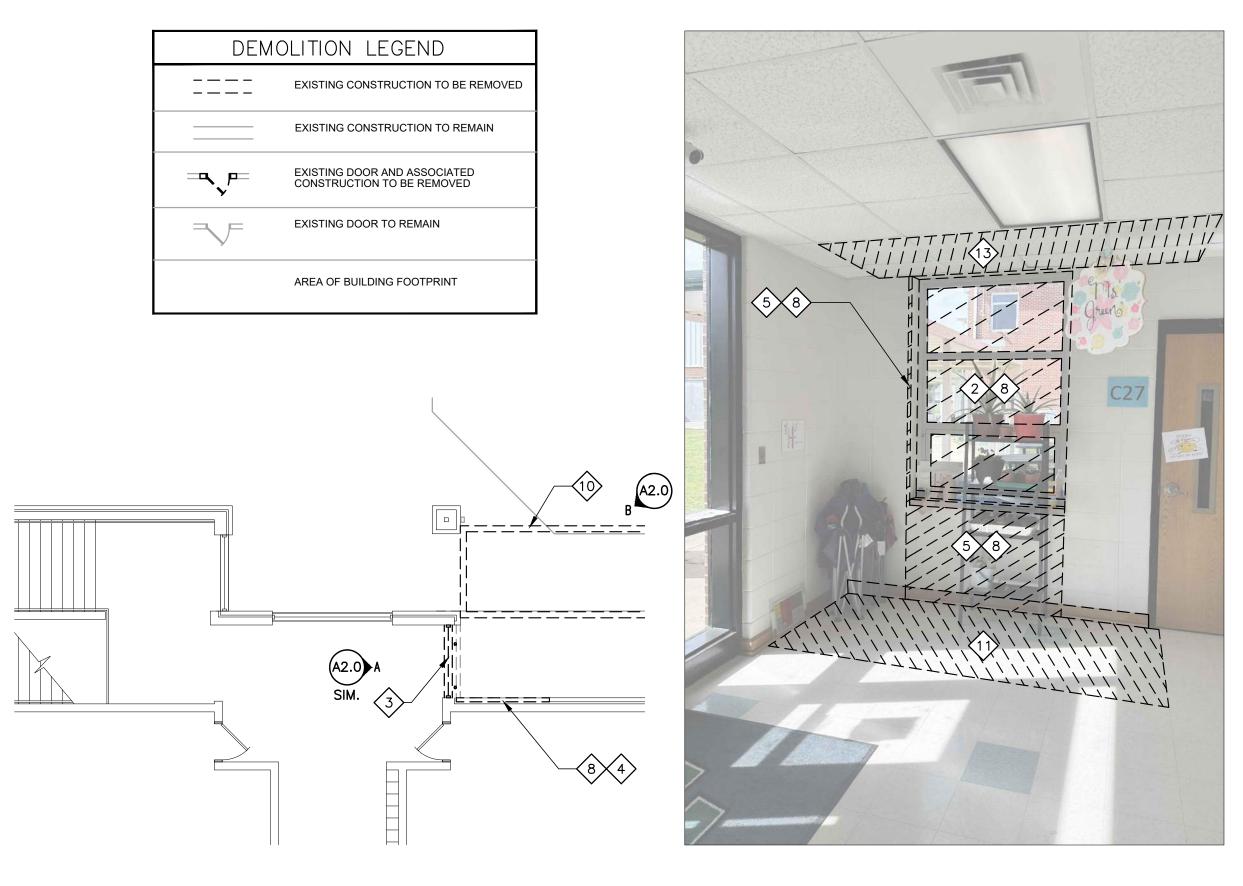
SHEET TITLE:
ARCHITECTURAL SITE PLAN

PROJ.	MGR.:	Ryan	Vernon
DRAWN	•	PPh	
hdr			
DATE:		8-28	3-2024
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JOB NO. **24-39**

SHEET NO: **A1.1**



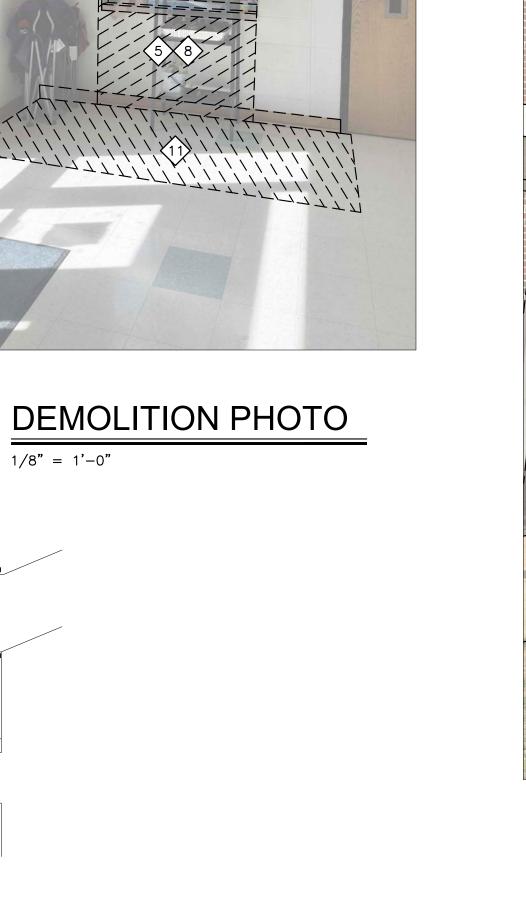


DEMO PLAN UPPER FLOOR

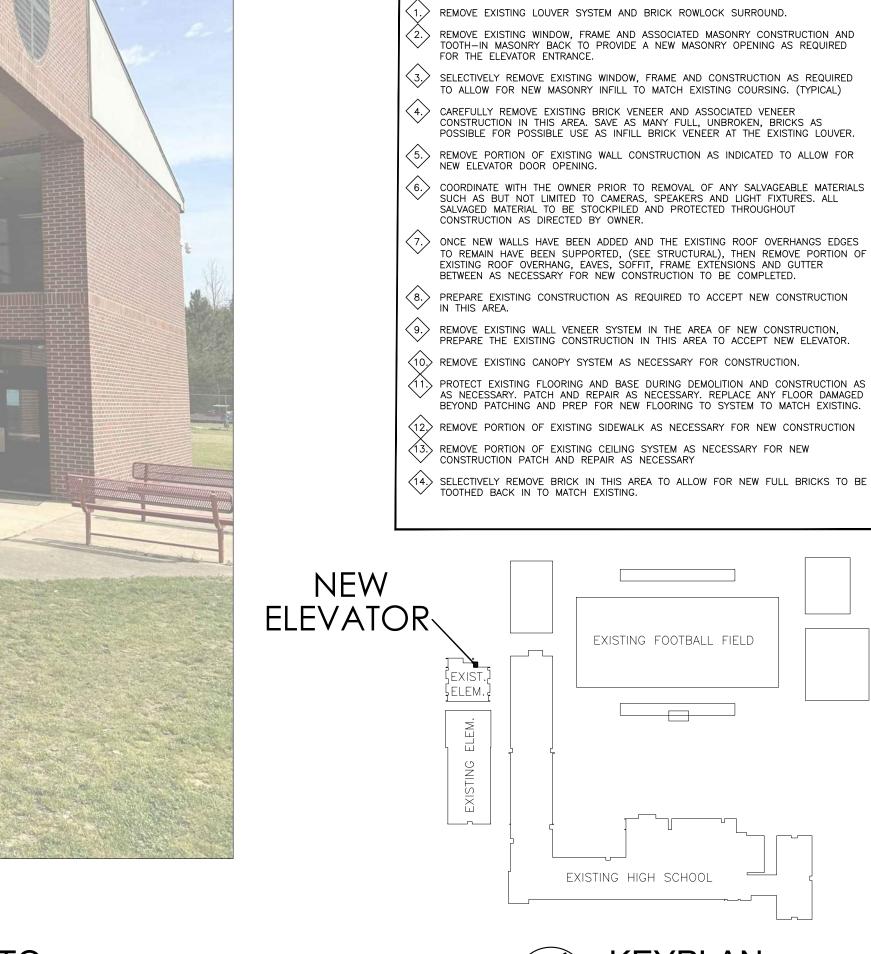
DEMOLITION PLAN MAIN FLOOR

1/8" = 1'-0"

1/8" = 1'-0"











S 35097

EME

No. 3365 RICK N. LATHAN

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155 BLC

SHEET TITLE: DEMOLITION PLANS, **ELEVATIONS AND PHOTOS**

PROJ. MGR.: Ryan Vernon DRAWN:

REVISIONS

8-28-2024

JOB NO. **24-39**

SHEET NO:

CONTRACTOR SHALL PROTECT EXISTING CONSTRUCTION & SYSTEMS TO REMAIN AND CORRECT ANY DAMAGE RESULTING FROM DEMOLITION WORK. MAINTAIN AND REROUTE EXISTING MP&E IN THE PATH OF DEMOLITION AND SERVING THAT TO REMAIN OPERATIONAL. PROTECT FIRE ALARM SYSTEM AND MAINTAIN OPERATIONAL. MAINTAIN EXISTING FIRE WALLS FUNCTIONAL.

CONTACT AND COORDINATE W/ ARCHITECT & STRUCTURAL ENGINEER BEFORE REMOVING OR ALTERING ANY STRUCTURAL COMPONENTS.

SEE RESPECTIVE STRUCTURAL, PLUMBING, HVAC AND ELECTRICAL DRAWINGS FOR ADDITIONAL DEMOLITION REQUIREMENTS.

COORDINATE WITH THE OWNER BEFORE REMOVING ANY SALVAGEABLE MATERIALS & EQUIPMENT.

DEMOLITION WORK SHALL NOT CHANGE THE INTEGRITY OF EXISTING STRUCTURE, FIRE ALARM SYSTEM & FIRE RATED CONSTRUCTION TO REMAIN. ANY EXISTING FIRE RATED CONSTRUCTION TO REMAIN WHICH HAS BEEN AFFECTED BY DEMOLITION WORK MUST BE CORRECTED AND MADE TO MEET THE ORIGINAL RATING.

COORDINATE WITH ARCHITECTURAL, STRUCTURAL, MECHANICAL, PLUMBING & ELECTRICAL DRAWINGS TO DETERMINE LIMITS OF DEMOLITION REQUIRED FOR

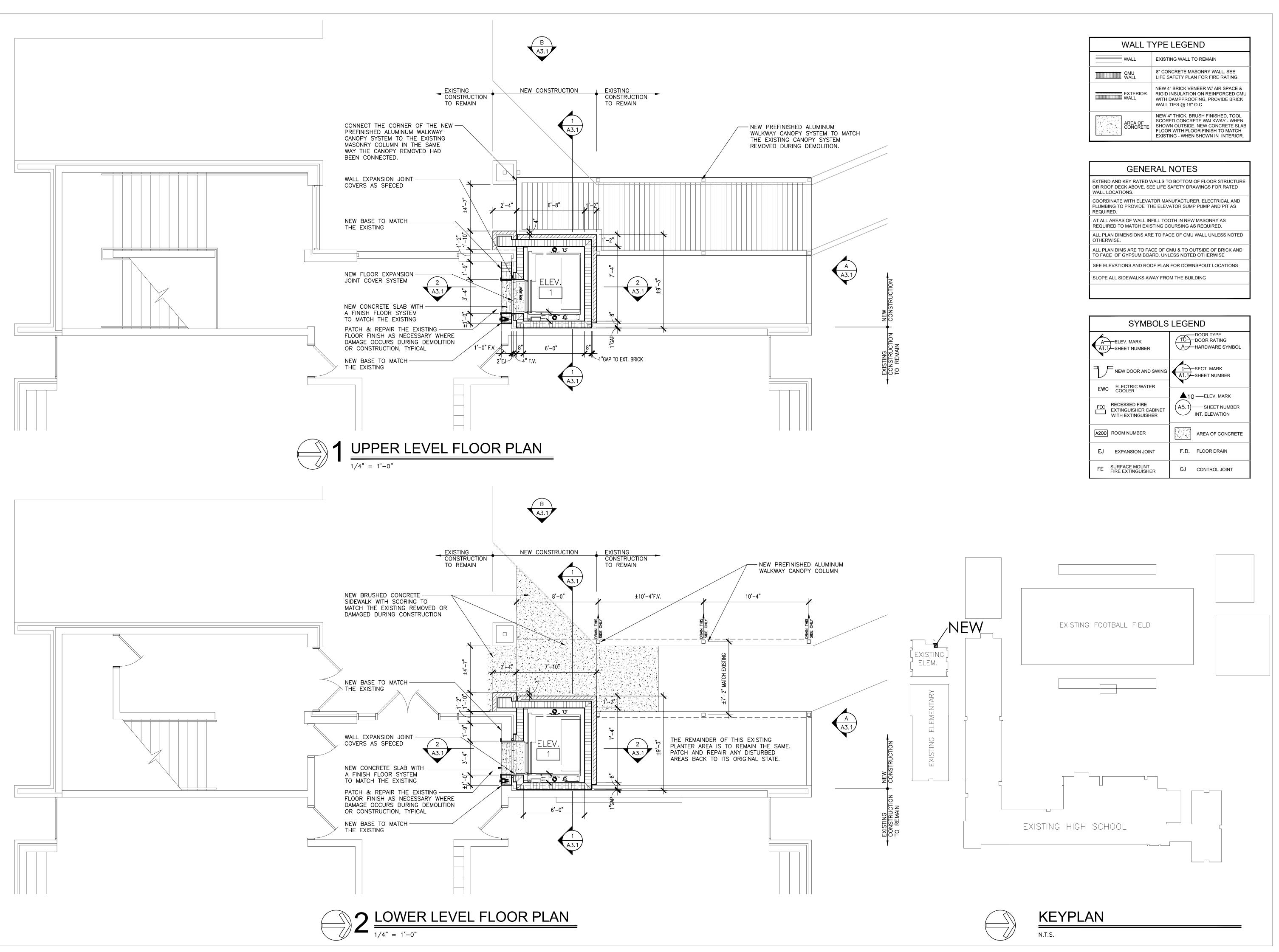
COORDINATE ALL UTILITY OUTAGES OVER WEEKENDS OR HOLIDAYS WITH OWNER PRIOR TO DEMOLITION.

GENERAL DEMOLITION KEY NOTES

COORDINATE WITH FINISH LEGEND AND SCHEDULE TO

REPAIR EXISTING ADJACENT CONSTRUCTION TO REMAIN.

DETERMINE EXISTING SURFACES TO RECEIVE NEW FINISHES REMOVE EXISTING FINISHES AS REQUIRED AND MAKE EXISTING SURFACES READY TO RECEIVE NEW FINISHES. PATCH AND/OR





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No. 3365 RICK N. LATHAN

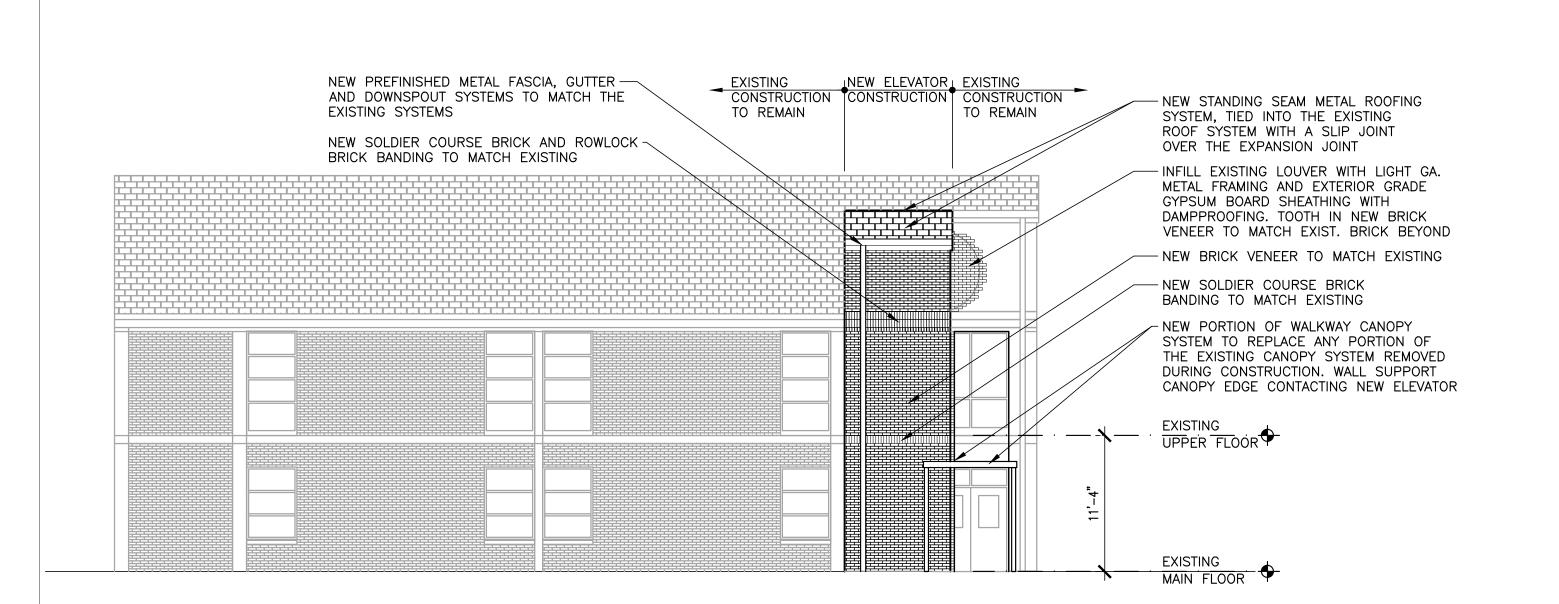
SHEET TITLE: FLOOR PLANS

PROJ. MGR.: **Ryan Vernon** DRAWN: 8-28-2024

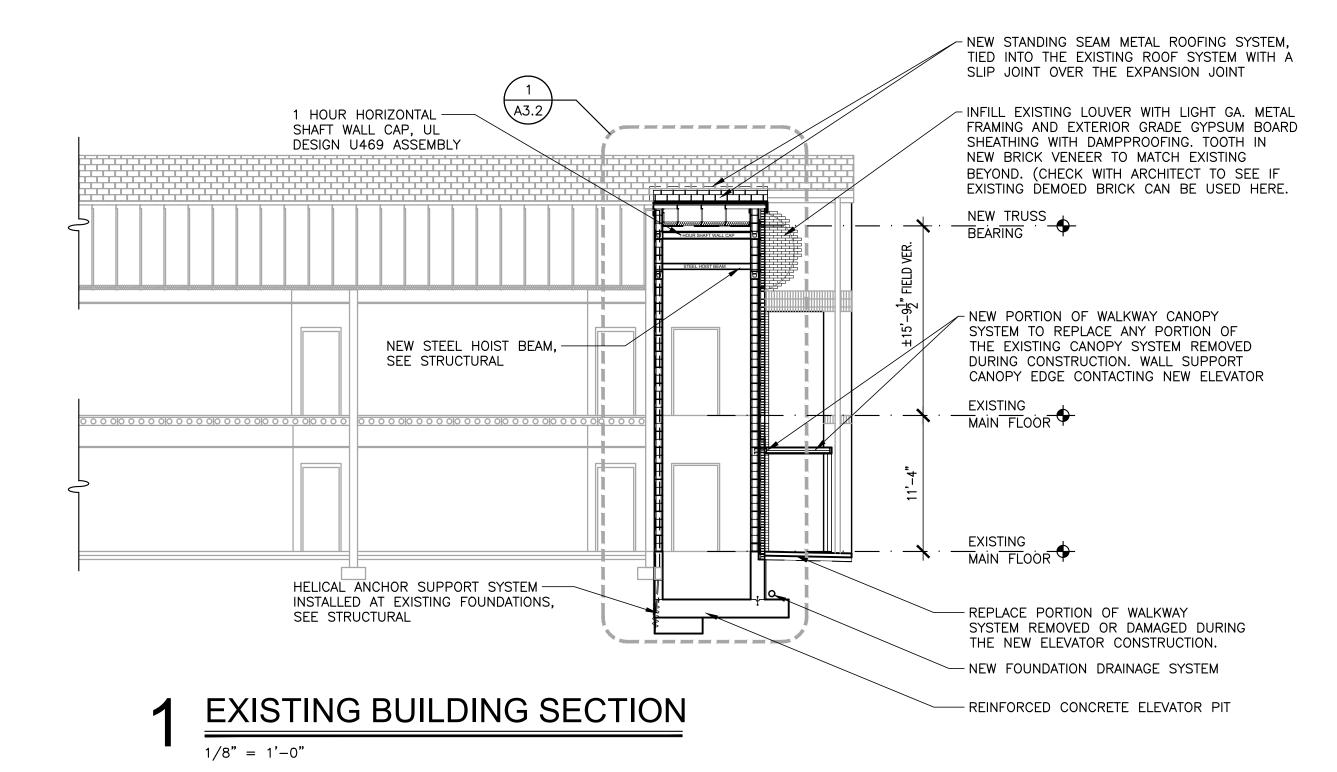
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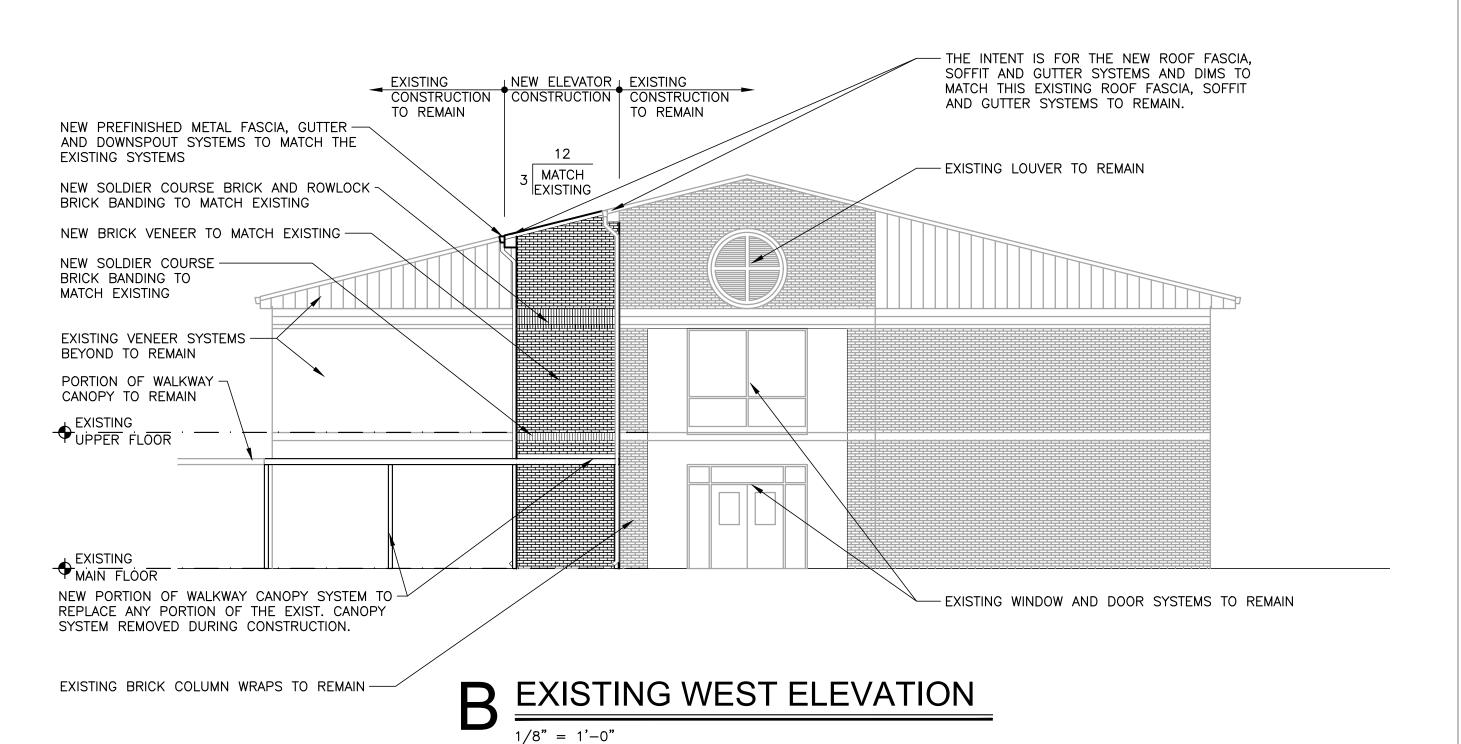
JOB NO. **24-39**

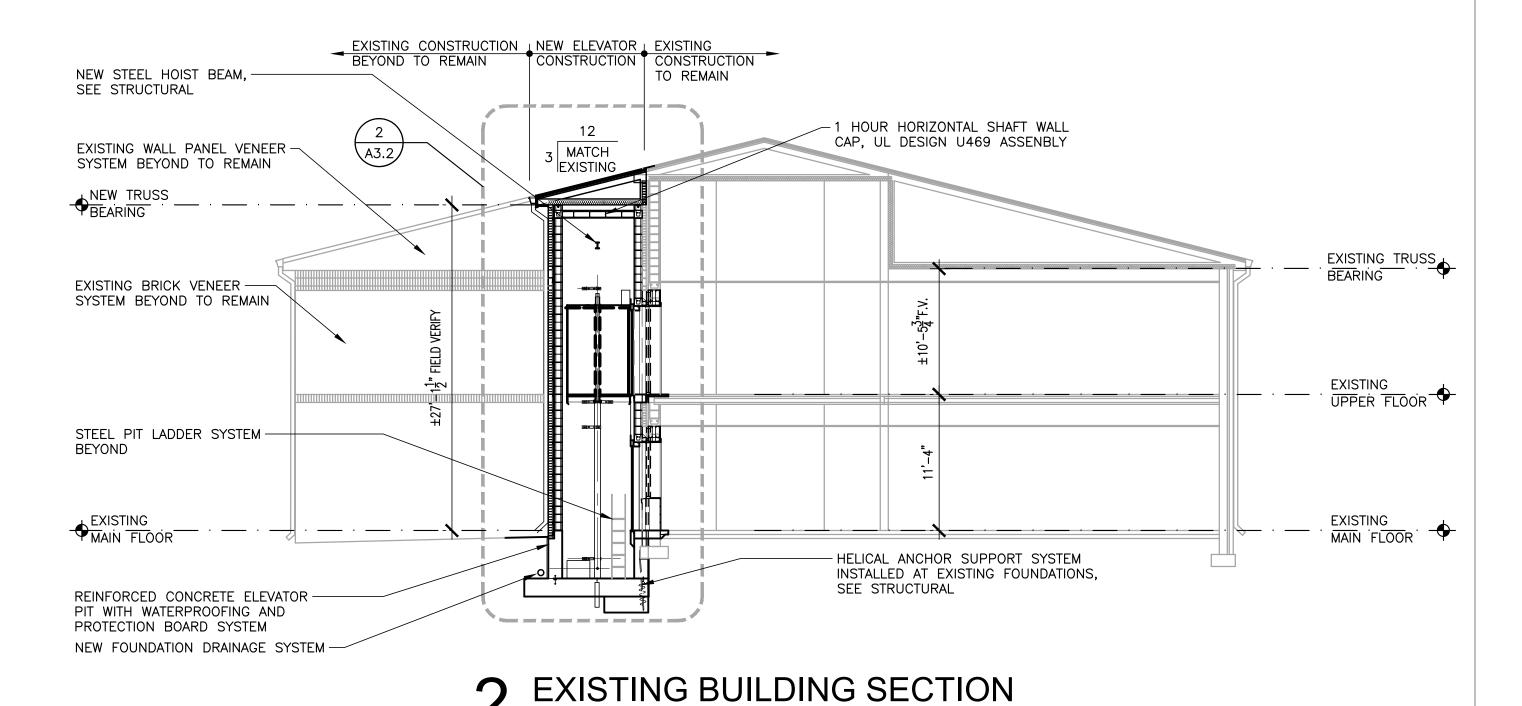
SHEET NO:



$A = \frac{\text{EXISTING NORTH ELEVATION}}{\frac{1}{8"} = \frac{1}{-0"}}$



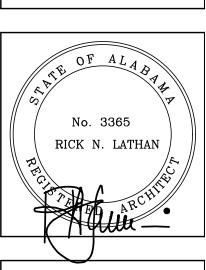




1/8" = 1'-0"

LATHAN ARCHITECTS

EVATOR FOR ORK ELEMENTARY SCHOOL

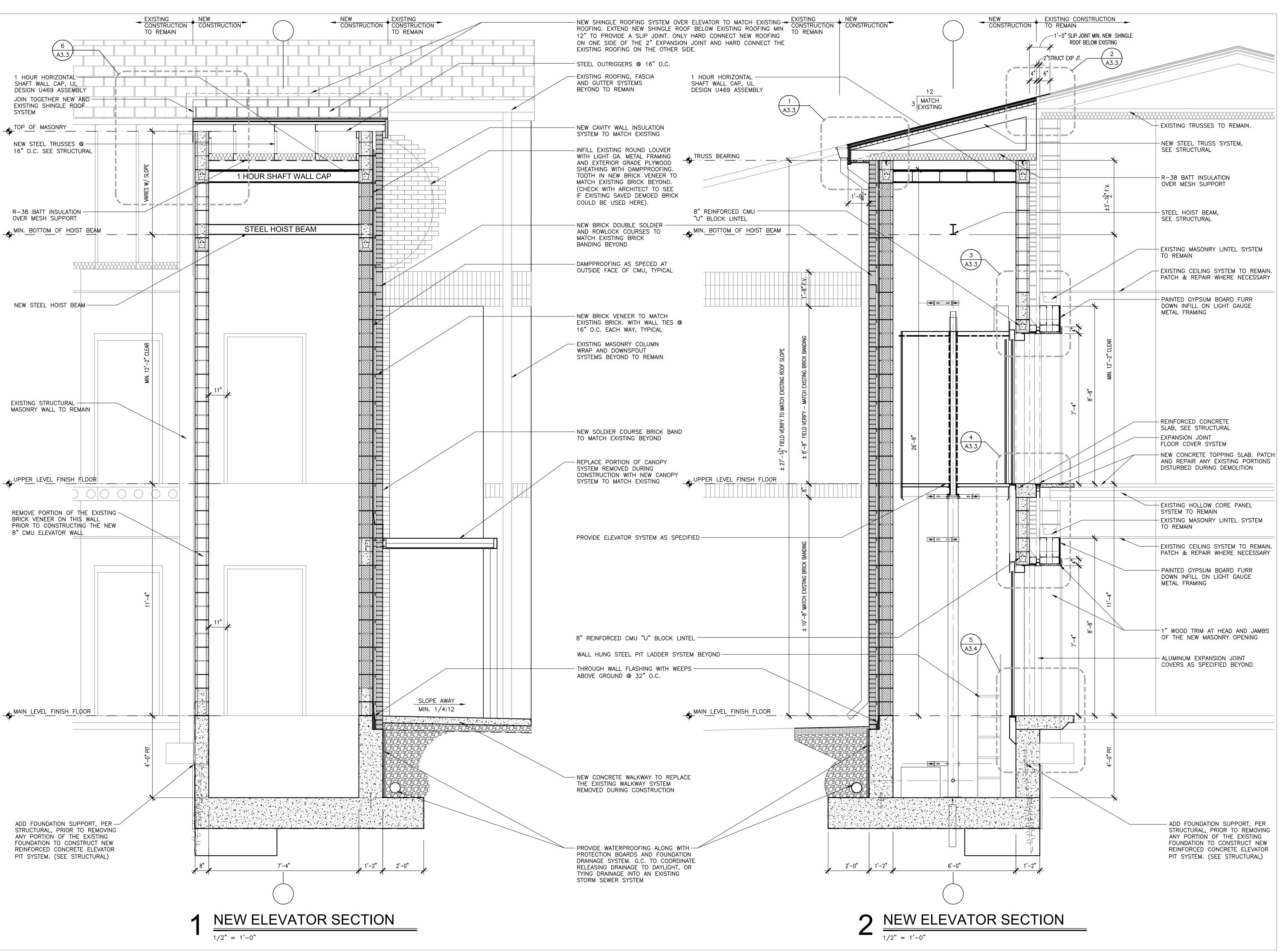


SHEET TITLE:
BUILDING ELEVATIONS AND
BUILDING SECTIONS

PROJ. MGR.:	Ryan Vernon
DRAWN:	PPh
hdr	
DATE:	8-28-2024
REVISIONS	
	DRAWN: hdr DATE:

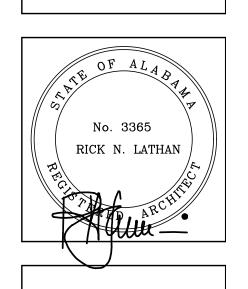
JOB NO. **24-39**SHEET NO:

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ELEVATOR FOR ST FORK ELEMENTARY SCHOO



SHEET TITLE:
ELEVATOR SECTIONS

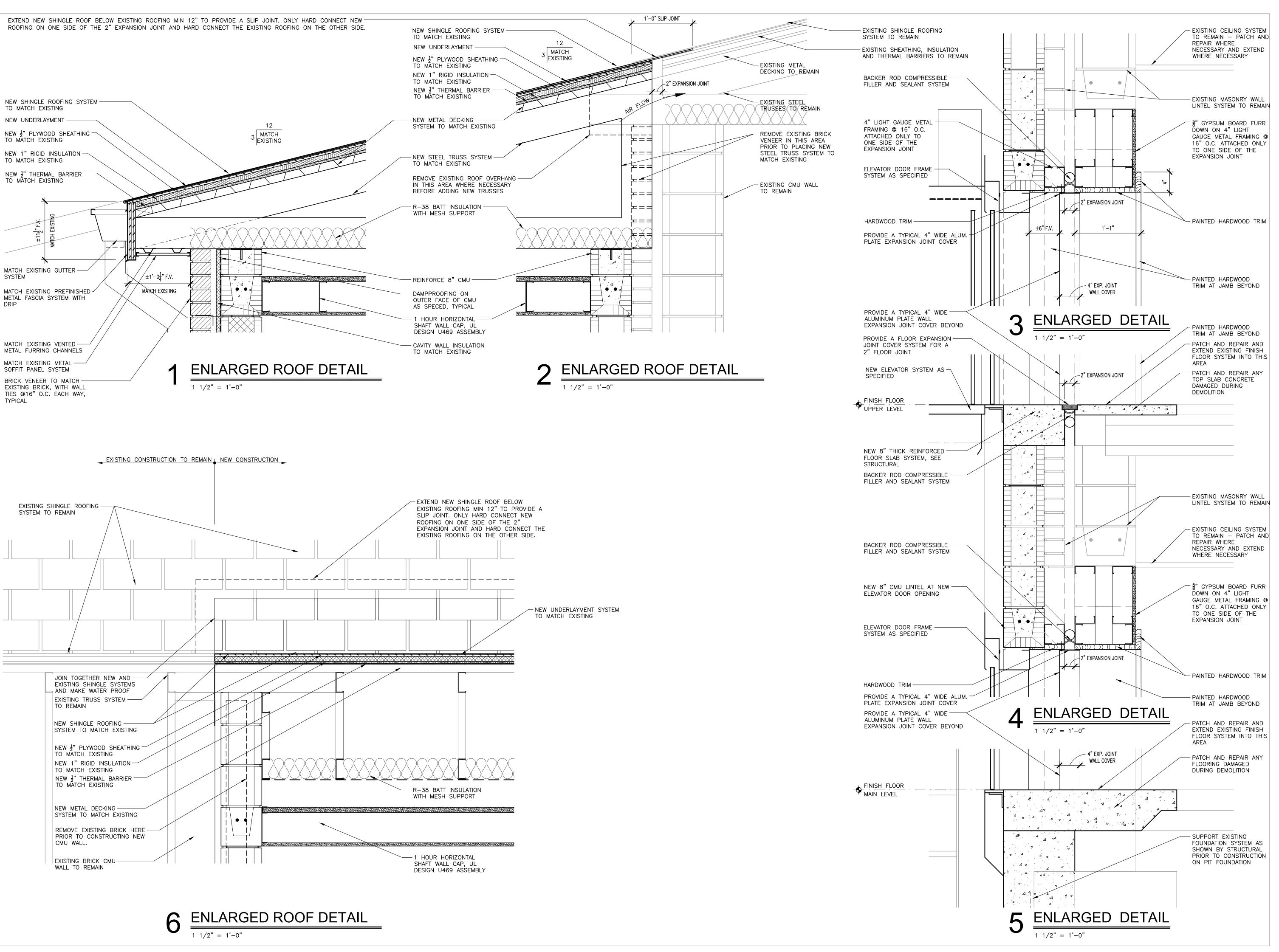
PROJ. MGR.: Ryan Vernon
DRAWN: PPh

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DATE: 8-28-2024
REVISIONS

JOB NO. **24-39**

A3.2

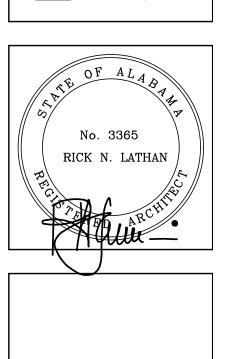




VEW ELEVATOR FOR

OCUST FORK ELEMENTARY SCHOO

55 School Road Locust Fork, AL 35097



SHEET TITLE:
ENLARGED DETAILS

PROJ. MGR.: Ryan Vernon
DRAWN: PPh

hdr

DATE: 8-28-2024
REVISIONS

JOB NO. **24-39**

A3.3

tel 205-824-5200 fax 205-824-5280

Job Number 24-120

JOB NO. **24-39**

SHEET NO:

GENERAL NOTES

1.0 DESIGN CRITERIA

- 1.1 CODES AND SPECIFICATIONS:
- A GENERAL BUILDING CODE: INTERNATIONAL BUILDING CODE, 2021 EDITION.
- B CONCRETE:
- BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-19)
- SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AMERICAN INSTITUTE OF STEEL CONSTRUCTION (ANSI/AISC 360-16)
- D. STEEL DECK:
- STEEL DECK INSTITUTE DESIGN MANUALS FOR COMPOSITE DECKS, NON-COMPOSITE DECKS, AND ROOF DECKS, LATEST EDITIONS.
- SPECIFICATIONS FOR MASONRY STRUCTURES (TMS 602-16)
- BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (TMS 602-16)
- NATIONAL CONCRETE MASONRY ASSOCIATION'S STANDARD PRACTICES AND "SPECIFICATION FOR THE DESIGN AND CONSTRUCTION OF LOAD BEARING CONCRETE
- 1.2 DESIGN GRAVITY LOADS (PSF):
- A DEAD LOADS:
- ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE GENERAL CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE.

NON-REDUCIBLE PARTITION LIVE LOAD OF 20 PSF HAS BEEN INCLUDED PER IBC

- B FLOOR LIVE LOADS:
- SECTION 1607.5. LIVE LOAD REDUCTIONS AS DETERMINED BY IBC SECTION 1607.12 HAVE BEEN
- STAIRS & EXITWAYS-----
- WHERE PERMITTED ROOF LIVE LOADS ARE REDUCED FROM THE BASE VALUE SHOWN
- D ROOF SNOW LOADS: GROUND SNOW LOAD (Pg)-IMPORTANCE FACTOR (I)-EXPOSURE FACTOR (Ce)--

BELOW IN ACCORDANCE WITH IBC SECTION 1607.14

1.3 DESIGN LATERAL LOADS:

THERMAL FACTOR (Ct)---

- ULTIMATE DESIGN WIND SPEED (3-SECOND GUST)-----114MPH BASIC WIND SPEED (3-SECOND GUST)-----92MPH WIND IMPORTANCE FACTOR (I)-----WIND EXPOSURE CATEGORY-----INTERNAL PRESSURE COEFFICIENTS----- +/- 0.18
- SEE TYPICAL DETAILS FOR COMPONENT AND CLADDING LOADS B. SEISMIC LOADS: OCCUPANCY CATEGORY III (GROUP E OCCUPANCIES WITH OCCUPANCY > 250) SEISMIC IMPORTANCE FACTOR-----1.25
- MAPPED SPECTRAL RESPONSE ACCELERATIONS: SS-----0,269 SPECTRAL RESPONSE COEFFICIENTS: SDS-----0.309 SD1-----0.177 SEISMIC DESIGN CATEGORY-----C BASIC SEISMIC-FORCE-RESISTING SYSTEM: INTERMEDIATE REINFORCED MASONRY SHEAR WALLS
- DESIGN BASE SHEAR: BASE BID (NON-STORM)-----20 KIPS SEISMIC RESPONSE COEFFICIENT, Cs-----0.1015 RESPONSE MODIFICATION FACTOR, R-----3.5
- ANALYSIS PROCEDURE: EOUIVALENT LATERAL FORCE PROCEDURE

2.0 GENERAL CONDITIONS

- 2.1 THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE A PORTION OF THE CONSTRUCTION DOCUMENTS. THE GENERAL CONTRACTOR AND SUBCONTRACTORS SHALL REFERENCE AND COORDINATE WITH OTHER DISCIPLINE'S DRAWINGS. ANY DISCREPANCIES OR OMISSIONS SHALL BE IMMEDIATELY REPORTED TO THE ARCHITECT AND STRUCTURAL DESIGN GROUP.
- 2.2 ALL REPORTS, PLANS, SPECIFICATIONS, COMPUTER FILES, FIELD DATA, NOTES, AND OTHER DOCUMENTS AND INSTRUMENTS PREPARED BY STRUCTURAL DESIGN GROUP AS INSTRUMENTS OF SERVICE SHALL REMAIN THE PROPERTY OF STRUCTURAL DESIGN GROUP. STRUCTURAL DESIGN GROUP SHALL RETAIN ALL COMMON LAW, STATUTORY, AND OTHER RESERVED RIGHTS, INCLUDING THE COPYRIGHT THERETO.
- 2.3 CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS, ELEVATIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION.
- 2.4 WHERE SHOP DRAWINGS, CALCULATIONS, OR SUBMITTALS ARE CALLED FOR IN THE PROJECT DOCUMENTS (DRAWINGS AND SPECIFICATIONS) AND ARE NOT PROVIDED BY THE CONTRACTOR, THE CONTRACTOR ASSUMES TOTAL RESPONSIBILITY FOR THE DESIGN AND ASSOCIATED WORK.
- 2.5 ENGINEER'S SHOP DRAWING REVIEW IS LIMITED TO REVIEW FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT REFLECTED IN THE STRUCTURAL PORTION OF THE CONTRACT DOCUMENTS. THIS REVIEW DOES NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE DRAWINGS, SPECIFICATIONS OR OTHER PROJECT CONTRACT DOCUMENTS. NO RESPONSIBILITY IS ASSUMED OR IMPLIED FOR THE CORRECTNESS OF DIMENSIONS OR DETAILS. THIS REVIEW DOES NOT AUTHORIZE CHANGES TO THE CONTRACT SUM UNLESS STATED IN A SEPARATE WRITTEN FORM OR CHANGE ORDER. CONTRACTOR SHALL CONFIRM AND CORRELATE ALL QUANTITIES AND DIMENSIONS, SELECT FABRICATION PROCESSES AND TECHNIQUES OF CONSTRUCTION, COORDINATE HIS WORK WITH THAT OF OTHER TRADES, AND PERFORM HIS WORK IN A SAFE AND SATISFACTORY MANNER. CONTRACTOR SHALL ALSO REFER TO THE REQUIREMENTS OF THE GENERAL AND SUPPLEMENTARY GENERAL CONDITIONS.

- 2.6 ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS, UNLESS NOTED.
- 2.7 VERIFY ALL DIMENSIONS AND DETAILS SHOWN ON THESE DRAWINGS. ANY DISCREPANCIES OR OMISSIONS FOUND SHALL BE REPORTED TO THE ENGINEER AND OTHER DESIGN PROFESSIONALS AS APPROPRIATE FOR RESOLUTION PRIOR TO PROCEEDING WITH ANY RELATED WORK.
- 2.8 THESE DRAWINGS DO NOT INCLUDE PROVISIONS TO SATISFY JOB SITE SAFETY REQUIREMENTS. CONTRACTOR IS SOLELY RESPONSIBLE FOR ENSURING SAFETY DURING CONSTRUCTION AND FOR CONFORMANCE TO ALL APPLICABLE OSHA STANDARDS. JOBSITE VISITS BY ENGINEER SHALL NOT CONSTITUTE APPROVAL. AWARENESS OR LIABILITY FOR ANY HAZARDOUS CONDITIONS.
- 2.9 STRUCTURAL DESIGN GROUP IS NOT RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS, SAFETY PROCEDURES, CONSTRUCTION SUPERVISION OR SITE SAFETY, AND DOES NOT HAVE THE AUTHORITY TO STOP WORK FOR THESE ITEMS. DRAWINGS FURTHER DO NOT PROVIDE ENGINEERING CONTROLS FOR SILICA STANDARD OR ANY OTHER SAFETY STANDARD.
- 2.10 THE CONTRACTOR IS SOLELY RESPONSIBLE FOR BRACING AND SHORING ALL EXCAVATIONS, DEWATERING OF EXCAVATION FROM EITHER SURFACE WATER, GROUND WATER OR SEEPAGE, TEMPORARY AND EXISTING STRUCTURES, AND PARTIALLY COMPLETED PORTIONS OF THE WORK TO ASSURE THE SAFETY OF ANY PERSON COMING IN CONTACT WITH THE WORK.
- 2.11 THE STRUCTURAL INTEGRITY OF THE BUILDING IS DEPENDENT UPON COMPLETION ACCORDING TO THE PLANS AND SPECIFICATIONS. THE STRUCTURAL ENGINEER OF RECORD ASSUMES NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION. THE METHOD OF CONSTRUCTION AND SEQUENCE OF OPERATIONS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL SUPPLY ANY NECESSARY BRACING, GUYS, ETC. TO PROPERLY BRACE THE STRUCTURE AGAINST WIND, DEAD AND LIVE LOADS UNTIL THE BUILDING IS COMPLETED ACCORDING TO THE PLANS AND SPECIFICATIONS. ANY QUESTIONS REGARDING TEMPORARY BRACING REQUIREMENTS SHOULD BE FORWARDED TO A STRUCTURAL ENGINEER FOR REVIEW.
- 2.12 MECHANICAL UNITS AND ANY OTHER EQUIPMENT SUPPORTED BY THE STRUCTURE WITH WEIGHTS IN EXCESS OF 200 LBS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO INSTALLATION.
- 2.13 WHERE NOTED IN DRAWINGS AND SPECIFICATIONS TO INSTALL PRODUCTS PER THE MANUFACTURER'S RECOMMENDATIONS, IT SHALL BE REQUIRED THAT THE CONTRACTOR FOLLOWS THE MANUFACTURER'S RECOMMENDATIONS.
- 2.14 STRUCTURAL OBSERVATION IS VISUAL OBSERVATION OF THE IN PLACE STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS AT THE TIME OF THE OBSERVATION AND SHALL NOT BE CONSTRUED AS INSPECTION OR APPROVAL OF CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING TESTING AND SPECIAL INSPECTIONS PER THE REQUIREMENTS IN THE PROJECT MANUAL.
- 2.15 OBSERVATION BY THE ENGINEER OF RECORD'S OFFICE DOES NOT REPLACE INSPECTIONS AND TESTING BY THE TESTING AGENCY OR SPECIAL INSPECTOR.

3.0 FOUNDATIONS

- 3.1 A GEOTECHNICAL ENGINEER, EMPLOYED BY THE GENERAL CONTRACTOR, SHALL PROVIDE COMPACTED FILL REQUIREMENTS FOR THE BUILDING PAD AND REVIEW THE FOUNDATION BEARING SURFACE TO VERIFY THE ASSUMED ALLOWABLE BEARING PRESSURE AND SEISMIC SITE CLASS NOTED. DO NOT PLACE CONCRETE PRIOR TO GEOTECHNICAL ENGINEER'S APPROVAL.
- 3.2 ASSUMED MAXIMUM ALLOWABLE BEARING PRESSURES (PSF):
- CONTINUOUS WALL FOOTINGS-----2000
- 3.3 HELICAL EARTH ANCHORS
- A. HELICAL EARTH ANCHORS SHALL BE DESIGNED FOR THE LOADS NOTED ON THE DRAWINGS.
- B. HELICAL EARTH ANCHOR SUPPLIER IS TO BE RESPONSIBLE FOR THE DESIGN OF ALL HELICAL EARTH ANCHOR AND THEIR CONNECTIONS TO THE STRUCTURE.
- C. HELICAL EARTH ANCHOR MANUFACTURER SUPPLIER SHALL SUBMIT TO THE STRUCTURAL AND GEOTECHNICAL ENGINEERS, FOR RECORD, CALCULATIONS TO SUBSTANTIATE THE DESIGN CAPACITY OF THE ANCHORS. CALCULATIONS SHALL BEAR THE SEAL OF A PROFESSIONAL GEOTECHNICAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
- D. HELICAL EARTH ANCHOR MANUFACTURE SHALL PROVIDE SHOP DRAWINGS INDICATING THE LAYOUT, ANTICIPATED DEPTH/LENGTH, AND ANCHORAGE DETAILS TO THE STRUCTURAL AND GEOTECHNICAL ENGINEERS FOR THEIR RECORDS.
- E. SITE TESTS:
- 1. LOAD TEST PROCEDURES (ASTM D1143, ASTM D3689): A SITE LOAD TEST SHALL BE PREFORMED ON ONE ANCHOR, OF THE TYPE TO BE USED AT THE PILE CAPS. BY AN INDEPENDENT TESTING AGENCY ACCEPTABLE TO THE ARCHITECT. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ADDITIONAL ANCHORS, MATERIALS, LABOR, AND EQUIPMENT AS REQUIRED TO TEST THE ANCHOR. THE TESTED ANCHOR SHALL BE ABANDONED. TEST ANCHOR IN COMPRESSION AND TENSION.
- 2. ACCEPTANCE CRITERIA: TESTED ANCHOR SHALL RESIST A LOAD OF 40 KIPS IN TENSION AND 5 KIPS IN COMPRESSION. TOP OF ANCHOR SHALL NOT MOVE VERTICALLY FROM AT REST POSITION MORE THAN 1" AT 100 KIP LOADING.
- 3.4 ALL FOUNDATION BEARING SURFACES SHALL BE REVIEWED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE THEIR COMPLIANCE WITH PRESSURES NOTED. ALL FOOTING ELEVATIONS ARE ESTIMATED AND MAY BE ADJUSTED IN THE FIELD BY THE GEOTECHNICAL ENGINEER.
- 3.5 COMPACTED FILL WITHIN THE BUILDING AREA (AND EXTENDING 10'-0" OUTSIDE THE EXTERIOR BUILDING LINE) SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT.
- 3.6 BACKFILL FOR FOUNDATION AND RETAINING WALLS SHALL BE A FREE DRAINING GRANULAR MATERIAL, SUCH AS SIZE #57 STONE. BACKFILL SHALL BE COMPACTED SUFFICIENTLY TO PREVENT SUBSIDENCE OF SURFACE ADJACENT TO WALL. THE GRANULAR MATERIAL SHALL BE PLACED IN A 45 DEGREE WEDGE EXTENDING FROM THE BASE OF THE FOOTING TO WITHIN 18" OF FINSH GRADE ON EXTERIOR AND TO UNDERSIDE OF SLAB ON INTERIOR.
- 3.7 GRANULAR BACKFILL SUPPORTING A FOOTING SHALL BE COMPACTED UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER OR HIS APPROVED REPRESENTATIVE. PROVIDE A 12" THICK CAP OF PROPERLY COMPACTED CRUSH AND RUN STONE BETWEEN THE FOOTING AND THE PROPERLY COMPACTED GRANULAR BACKFILL. EXTEND CRUSH AND RUN CAP TWO FEET BEYOND THE PERIMETER OF THE FOOTING OR AS DIRECTED BY THE GEOTECHNICAL ENGINEER.
- 3.8 FOUNDATION AND RETAINING WALLS SHALL NOT BE BACKFILLED UNTIL CONCRETE HAS ATTAINED THE REQUIRED 28 DAY COMPRESSIVE STRENGTH.
- 3.9 DO NOT PLACE BACKFILL AGAINST FOUNDATION WALLS UNTIL UPPER BRACING FLOORS ARE IN PLACE FOR AT LEAST SEVEN DAYS AND HAVE ATTAINED 75% OF DESIGN

- 3.10 REINFORCING STEEL IN CONTINUOUS WALL FOOTINGS SHALL EXTEND THRU SPREAD FOOTINGS AT THE SAME ELEVATION AS WALL FOOTING, STEP WALL FOOING DOWN ON SPREAD FOOTING WHERE SPREAD FOOTING IS BELOW CONTINUOUS WALL FOOTING.
- 3.11 SUBGRADE AND GRANULAR FILL SUPPORTING SLABS ON GRADE SHALL BE AS RECOMMENDED BY THE GEOTECHNICAL REPORT AND COMPACTED UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER OR HIS APPROVED REPRESENTATIVE. SEE SPECIFICATIONS FOR VAPOR RETARDER BENEATH SLABS ON GRADE.
- 3.12 GRANULAR FILL BENEATH SLABS, UNLESS NOTED OTHERWISE, SHALL BE 4" COMPACTED #57 STONE.
- 3.13 VAPOR RETARDER BENEATH SLABS ON GRADE, UNLESS NOTED, SHALL MEET ASTM E 1745, CLASS A, 15 MIL MINIMUM THICKNESS WITH MANUFACTURER'S RECOMMENDED ADHESIVE OR PRESSURE-SENSITIVE TAPE AND PIPE BOOTS, SUCH AS W.R. MEADOWS INC. PRODUCT PERMINATOR 15.
- 3.14 NO EXCAVATION SHALL BE CLOSER THAN AT A SLOPE OF 2:1 (TWO HORIZONTAL TO ONE VERTICAL) TO A FOOTING.

4.0 CONCRETE

- 4.1 CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.
- 4.2 CONCRETE STRENGTH AND DURABILITY REQUIREMENTS -- MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS (PSI), TYPE OF CONCRETE, MAXIMUM WATER/CEMENTITIOUS RATIO, AIR CONTENT, SLUMP, AND CONCRETE USE:

STREN	GTH TYPE	MAX W/C	AIR	SLUMP	USE
	NORMAL WT.			3" TO 5"	FOOTINGS SLABS ON GRADE
2200	NORMAL WT.			3" TO 5"	UNLESS NOTED

- A. CONCRETE MIX DESIGN SHALL BE WORKABLE WITH LOWEST TOTAL WATER PER CUBIC YARD USING LARGEST PRACTICAL MAXIMUM SIZE OF COURSE AGGREGATE.
- 4.3 REINFORCING BARS: ASTM A615 GRADE 60.
- 4.4 WATERSTOPS: FLEXIBLE PVC WATERSTOPS, CE CRD-C 572 UNLESS NOTED OTHERWISE, WITH FACTORY-INSTALLED METAL EYELETS, FOR EMBEDDING IN CONCRETE TO PREVENT PASSAGE OF FLUIDS THROUGH JOINTS. FACTORY FABRICATE CORNERS, INTERSECTIONS, AND DIRECTIONAL CHANGES. ACCEPTABLE MANUFACTURER IS THE GREENSTREAK GROUP, INC, 800-325-9504, OR EQUAL. PROFILE SHALL BE FLAT, DUMBBELL WITH CENTER BULB WITH DIMENSIONS OF 6 INCHES BY 3/8 INCH THICK.
- A. FLEXIBLE WATERSTOP INSTALLATION: INSTALL IN CONSTRUCTION JOINTS AND AT OTHER JOINTS INDICATED TO FORM A CONTINUOUS DIAPHRAGM. INSTALL IN LONGEST LENGTHS PRACTICABLE. SUPPORT AND PROTECT EXPOSED WATERSTOPS DURING PROGRESS OF THE WORK.
- 4.5 REINFORCING STEEL SHOWN IN SECTIONS AND DETAILS ARE A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES AND GENERAL NOTES FOR ACTUAL REINFORCING REOUIRED.
- 4.6 REINFORCING BAR PLACING ACCESSORIES IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE. WHERE CONCRETE IS EXPOSED IN FINISHED BUILDING, PROVIDE ACCESSORIES WITH RUSTPROOF LEGS. WHERE CONCRETE IS SAND-BLASTED OR BUSH-HAMMERED, PROVIDE ACCESSORIES OF STAINLESS STEEL.
- 4.7 DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315. REINFORCEMENT SHALL NOT BE WELDED UNLESS NOTED OR APPROVED BY THE ENGINEER.
- 4.8 ALL SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.
- 4.9 ALL REINFORCING MARKED "CONT." INDICATES REINFORCING SHALL BE "CONTINUOUS" AND SHALL BE SPLICED WITH CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.
- 4.10 PROVIDE CORNER BARS AT ALL CORNERS OF CONTINUOUS REINFORCING IN FOOTINGS, SLABS OR WALLS. CORNER BARS SHALL BE LONG ENOUGH TO PROVIDE A CLASS "B"
- 4.11 CONCRETE COVERAGE OF REINFORCEMENT, UNLESS NOTED:

LAP SPLICE OF REINFORCING BARS.

- FOOTINGS-----2" TOP & 3" BOTTOM & SIDES FOUNDATION RETAINING WALLS-----2" BOTH FACES SUMP AND PIT WALLS-----3" BOTH FACES
- NOTE: SLAB ON GRADE WWR OR REINFORCEMENT EACH WAY SHALL BE 2" CLEAR FROM TOP OF SLAB. SEE EARTH SUPPORTED SLABS SECTION BELOW.
- 4.12 WALL VERTICAL REINFORCING: DOWEL TO FOUNDATION WITH HOOKED BARS OF SAME SIZE AND SPACING AS VERTICAL REINFORCING.
- 4.13 WELDED WIRE REINFORCEMENT (WWR): ASTM A185. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2 INCHES OR 6 INCHES.
- 4.14 EARTH SUPPORTED SLABS:

OF FLOOR COVERING.

- 4" THICK (UNLESS NOTED), REINFORCED WITH 6X6 W2.9/W2.9 WWR FLAT SHEETS SUPPORTED 2" CLEAR OF TOP OF SLAB, UNLESS NOTED, WWR TO BE CHAIRED AT 36 INCHES EACH WAY MINIMUM. SEE FOUNDATION NOTES FOR SUBGRADE
- PROVIDE CONTROL AND CONSTRUCTION JOINTS AT MAXIMUM OF 3-4 TIMES SLAB THICKNESS IN FEET OR AS REQUIRED TO PREVENT UNCONTROLLED CRACKING PER ACI RECOMMENDATIONS. AS AN EXAMPLE, FOR A 4" THICK SLAB, PROVIDE JOINTS SPACED 12 - 16 FEET MAXIMUM. PANELS TO BE RECTANGULAR WITH LONG SIDE NOT TO EXCEED 1-1/2X SHORT SIDE. CUTTING SHOULD BE STARTED AS SOON AS CONCRETE HAS HARDENED SUFFICIENTLY TO PREVENT AGGREGATE FROM BEING DISLODGE. CONTRACTOR SUBMIT PLAN SHOWING LOCATION OF CONSTRUCTION AND CONTROL JOINTS.
- FLOOR DESIGN AND CONSTRUCTION BASIS IS ACI 302 AND 360, AND IT IS UNREALISTIC TO EXPECT CRACK-FREE OR CURL-FREE FLOORS. IT IS NORMAL TO EXPECT SOME AMOUNT OF CRACKING AND CURLING IN THE SLAB ON GRADE, AND SUCH OCCURRENCE DOES NOT NECESSARILY REFLECT ADVERSELY ON EITHER THE ADEQUACY OF THE FLOOR DESIGN OR THE QUALITY OF ITS CONSTRUCTION.
- EARTH SUPPORTED SLABS SHALL BE MOIST CURED FOR A MINIMUM OF SEVEN DAYS. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION. CURING COMPOUNDS, UNLESS NOTED, SHALL BE A MINIMUM OF CLEAR, WATERBORNE, MEMBRANE-FORMING CURING COMPOUND MEETING ASTM C 309, TYPE 1, CLASS B, SELF-DISSIPATING, CERTIFIED BY CURING COMPOUND MANUFACTURER TO NOT INTERFERE WITH BONDING
- WHERE CONTROL JOINTS TERMINATE INTO NON-PARALLEL CONTROL JOINTS, PROVIDE 2#4 X 6'-0" BARS MID DEPTH OF SLAB PERPENDICULAR TO TERMINAL CONTROL JOINT.
- PROVIDE 2#4 X 6'-0" BARS MID DEPTH OF SLAB AT REENTRANT CORNERS.
- WHERE CONTROL JOINTS TERMINATE AT EMBEDDED STEEL ELEMENTS (SUCH AS EDGE REINFORCEMENT AT LOADING DOCKS), PROVIDE JOINT IN STEEL ELEMENT.

- 4.15 WALL AND SLAB OPENINGS AND SLEEVES SMALLER THAN 12" (IN LARGER DIMENSION) ARE NOT SHOWN ON PLANS. CONTRACTOR SHALL SUBMIT ALL OPENINGS (SIZE AND LOCATIONS) AS A SINGLE COORDINATED SLEEVE PLAN FOR REVIEW AND APPROVAL.
- 4.16 CAST IN PLACE ALL SLEEVES AND INSERTS.
- 4.17 NO CONDUIT OR PIPE SHALL BE CAST IN THE SLAB ON GRADE WITHOUT THE WRITTEN APPROVAL OF STRUCTURAL DESIGN GROUP.

5.0 STRUCTURAL STEEL

- 5.1 FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- 5.2 THE STEEL FRAME IS "NON-SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR UNTIL REQUIRED CONNECTIONS OR ELEMENTS ARE IN
- 5.3 STRUCTURAL STEEL: ASTM A992 FOR WIDE FLANGE BEAMS AND COLUMNS: ASTM A36 FOR ALL OTHER SHAPES
- 5.4 WELDED CONNECTIONS: E70XX ELECTRODES. MINIMUM SIZE FILLET WELD 3/16". WELDING QUALIFICATION, PROCEDURES AND PERSONNEL SHALL BE CERTIFIED ACCORDING TO AWS D1.1, THE STRUCTURAL WELDING CODE - STEEL.
- 5.5 THREADED AND PLAIN STEEL RODS: ASTM A36.
- 5.6 HEADED STUDS: TYPE B SHEAR STUD CONNECTORS MADE FROM ASTM A108, GRADE 1015 OR 1020, COLD-FINISHED CARBON, AND COMPLYING WITH AWS D1.1.
- 5.7 CONNECTIONS:
- A. BEARING TYPE A325-N IN ACCORDANCE WITH RCSC (LRFD OR ASD VERSION) "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". BOLTS THROUGH 4" WIDE BEAM FLANGES SHALL BE 5/8" DIAMETER. OTHER BOLTS SHALL BE 3/4" DIAMETER. USE FULL DEPTH DOUBLE SHEAR TAB CONNECTIONS.
- 5.8 ALL STEEL EXPOSED TO WEATHER, INCLUDING STEEL LINTELS FOR MASONRY OPENINGS SHALL BE HOT DIP GALVANIZED
- 5.9 ALL HANDRAILS, GUARDRAILS, AND EMBEDS NOT SPECIFICALLY DETAILED ON THE DRAWINGS SHALL BE DESIGNED IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE NOTED ABOVE, BY THE CONTRACTOR, UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. CALCULATIONS SHALL BEAR THE SEAL OF THE PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED AND SHALL BE SUBMITTED FOR THE FILES OF THE ARCHITECT AND SHALL BE INCLUDED WITH THE SHOP DRAWINGS.

6.0 STEEL DECK

- 6.1 DECK PROPERTIES AND ATTACHMENTS SHALL BE IN ACCORDANCE WITH THE STEEL DECK
- 6.2 DECK SHALL BE CONTINUOUS OVER THREE OR MORE SPANS. WHERE DECK SPANS LESS THAN THREE SPANS ARE REQUIRED, THEY SHOULD BE CLEARLY MARKED ON THE SHOP
- 6.3 DECK SHALL BE CONNECTED TO SUPPORTING STRUCTURE AS SHOWN IN SECTIONS AND/OR
- PLAN NOTES. PROVIDE A MINIMUM OF 5/8" DIAMETER PUDDLE WELDS AT 6". 6.4 COLD-FORMED STEEL FRAMING, SUSPENDED CEILINGS, LIGHT FIXTURES, DUCTS PIPING, AND/OR OTHER UTILITIES SHALL NOT BE SUPPORTED BY THE STEEL ROOF
- 6.5 DECK SIZE, TYPE AND SPACING SHALL BE AS INDICATED IN THE PLAN NOTES OR

7.0 MASONRY

LOCAL BUILDING CODE.

GREATER THAN 2650 PSI.

PSI AT 28 DAYS.

- 7.1 MASONRY CONSTRUCTION SHALL CONFORM TO ACI 530.1-16 SPECIFICATION.
- 7.2 ALL MASONRY MATERIALS AND CONSTRUCTION SHALL COMPLY WITH THE RECOMMENDATIONS OF BRICK INSTITUTE OF AMERICA (BIA) AND NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA) AND MINIMUM REQUIREMENTS ESTABLISHED BY THE
- 7.3 MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNIT (f'm) SHALL BE 2000
- 7.4 NET COMPRESSIVE STRENGTH FOR EACH CMU UNIT SHALL MEET OR EXCEED 2000 PSI AT 28 DAYS. FOR TYPE N MORTAR, NET COMPRESSIVE STRENGTH FOR BLOCK SHALL BE
- 7.5 ALL MASONRY SHALL BE NORMAL WEIGHT IN ACCORDANCE WITH ASTM C90.
- 7.6 GROUT COMPRESSIVE STRENGTH SHALL BE 2500 PSI AT 28 DAYS. GROUT SHALL ADDITIONALLY COMPLY WITH TABLE 7 OF ACI 530.1/ASCE 6/TMS 602 FOR DIMENSIONS OF GROUT SPACES AND POUR HEIGHTS. COURSE GROUT SHALL BE USED WHERE
- 7.7 MORTAR SHALL BE TYPE S OR M. TYPE N MORTAR ALLOWED ONLY IF THE CMU NET COMPRESSIVE STRENGTH IS GREATER THAN 2650 PSI.
- 7.8 ALL MASONRY SHALL BE STACK BOND, UNLESS NOTED.

OPENINGS AND WALL ENDS.

- 7.9 ALL BLOCK CELLS AND CAVITIES BELOW GRADE SHALL BE FILLED WITH CONCRETE OR
- 7.10 MASONRY REINFORCING LAP SPLICE LENGTHS SHALL BE 72 BAR DIAMETERS
- 7.11 THE CONTRACTOR SHALL PROVIDE DETAILED SHOP DRAWINGS OF THE CMU REINFORCEMENT. A. SHOP DRAWINGS SHALL INCLUDE AN ELEVATION VIEW OF EACH REINFORCED WALL

WITH ALL VERTICAL AND HORIZONTAL REINFORCING AS WELL AS WALL

OPENINGS/PENETRATIONS SHOWN. REINFORCING SHOP DRAWINGS NOT CONTAINING

- THESE ELEVATION DRAWINGS WILL BE RETURNED AS AN INCOMPLETE SUBMITTAL.
- 7.13 WHEN REINFORCING IS SPECIFIED, PROVIDE AT EACH SIDE OF CONTROL JOINTS,

7.12 MODIFY CMU BLOCKS AS REQUIRED TO INSTALL REINFORCING AS NOTED / SHOWN.

- 7.14 EXTEND REBAR AT WALL OPENINGS A MINIMUM OF 2'-0" PAST THE OPENING AT ALL CORNERS, UNLESS NOTED. AT WINDOWS, PROVIDE A MINIMUM OF 2#4 BARS AT THE SILL OF THE WINDOWS.
- 7.15 GROUT SHALL COMPLY WITH TABLE 7 OF ACI 530.1/ASCE 6/TMS 602 FOR DIMENSIONS OF GROUT SPACES AND POUR HEIGHTS. USE COURSE GROUT FOR OPENINGS LARGER THAN 4" OTHERWISE USE TYPE FINE GROUT. MINIMUM COMPRESSIVE STRENGTH FOR GROUT SHALL BE 2,500PSI AT 28 DAYS.



GENERAL NOTES

PROJ. MGR.:

Job Number 24-120

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SHEET TITLE: GENERAL NOTES CONTINUED

PROJ. MGR.: HCW DRAWN: ABS

DATE: AUGUST 28, 2024

REVISIONS

JOB NO. **24-39**

SHEET NO:

GENERAL NOTES CONTINUED

- 7.16 PROVIDE HORIZONTAL JOINT REINFORCING IN REINFORCED MASONRY WALLS AS DIRECTED BY THE ARCHITECT. AT WALL CORNERS AND INTERSECTIONS, PROVIDE PREFABRICATED T AND L SHAPES, FIELD BENDING IS NOT PERMITTED. MINIMUM OF LADDER TYPE ZINC COATED CONFORMING TO ASTM A82 HOHMANN & BARNARD 220 LADDER-MESH OR EQUIVALENT AT EVERY OTHER BLOCK COURSE ABOVE FOOTING. REINFORCEMENT SHOULD CONSIST OF TWO OR MORE LONGITUDINAL WIRES, NO. 9 GAUGE OR LARGER, WELDED WITH NO. 9 GAUGE OR LARGER CROSS WIRES. LAP SPLICE HORIZONTAL JOINT REINFORCING A MINIMUM OF 12".
- 7.17 PROVIDE DOVETAIL ANCHORS AT 16" O/C, UNLESS NOTED OTHERWISE, WHERE MASONRY WALLS ABUT CONCRETE SURFACES.
- 7.18 WHERE MASONRY WALLS SUPPORT EARTH ON BOTH SIDES, BACKFILL EACH SIDE SIMULTANEOUSLY.
- 7.19 CONDUITS, REFRIGERANT PIPING (WITH ANY REQUIRED INSULATION INCLUDED), CONDENSATE DRAIN LINES, ETC. UP TO 2" IN OUTSIDE DIAMETER MAY EXTEND CONT THRU MASONRY WALLS & BOND BEAMS. COORDINATE WITH MECHANICAL, ELECTRICAL, PLUMBING, ETC. DRAWINGS FOR SIZE AND LOCATION. DO NOT INTERRUPT CONTINUOUS REINFORCING STEEL IN PLACEMENT OF CONDUITS, PIPING, DRAIN LINES, ETC.
- 7.20 THE MASONRY WALLS ARE "NON-SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR UNTIL REQUIRED CONNECTIONS OR ELEMENTS ARE IN PLACE. BRACING SHALL BE PER THE FOLLOWING, AND CONTRACTOR SHALL PROVIDE ADDED REINFORCING AND GROUT IF REQUIRED BY THE BRACING.
 - A. THE "2012 STANDARD PRACTICE FOR BRACING MASONRY WALLS UNDER
- B. THE "MASONRY WALL BRACING HANDBOOK" AS PUBLISHED BY THE MASONRY CONTRACTORS ASSOCIATION OF AMERICA (MCAA) SHOULD BE USED IN CONJUNCTION WITH THE "STANDARD PRACTICE".

8.0 POST-INSTALLED REINFORCING, ANCHORS AND **FASTENERS**

- 8.1 POST-INSTALLED ANCHORS AND/OR REINFORCING SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS AND/OR REINFORCING IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS AND/OR REINFORCING.
- 8.2 THE BELOW PRODUCTS ARE THE DESIGN BASIS FOR THIS PROJECT. PRODUCT DIAMETER AND EMBEDMENT SHALL BE SHOWN IN THE DETAILS.

8.3 FOR ANCHORING INTO CONCRETE:

- A. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. PRE-APPROVED PRODUCTS INCLUDE:
 - 1. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-2713 & IAPMO-UES ER-493) 2. SIMPSON STRONG-TIE "STRONG-BOLT 2" (ICC-ES ESR-3037)
 - 3. SIMPSON STRONG-TIE "TITEN-HD ROD HANGER" (ICC-ES ESR-2713) 4. SIMPSON STRONG-TIE "TITEN TURBO" (IAPMO-UES ER-712) - FOR
 - UNCRACKED CONCRETE ONLY 5. HILTI KWIK HUS-EZ (KH-EZ), KH-EZ CRC, KH-EZ SS316, KH-EZ C, KH-EZ E, KH-EZ-I, AND KH-EZ P SCREW ANCHOR SAFE SET SYSTEM WITH HOLLOW
- DRILL BIT AND VACUUM (ICC ESR-3027) 6. HILTI KWIK BOLT-TZ2 EXPANSION ANCHOR SAFE SET SYSTEM WITH HOLLOW DRILL BIT AND VACUUM AND SI-AT-A22 TOOL WITH ADAPTIVE TORQUE FOR
- APPLICABLE SIZES (ICC ESR-4266) 7. HILTI KWIK BOLT 1 EXPANSION ANCHOR SAFE SET SYSTEM WITH HOLLOW DRILL BIT AND VACUUM AND SI-AT-A22 TOOL WITH ADAPTIVE TORQUE FOR
- APPLICABLE SIZES (ICC ESR-678) 8. HILTI HDA UNDERCUT ANCHORS (ICC ESR 1546)
- 9. HILTI HSL-3 EXPANSION ANCHORS (ICC ESR 1545)
- 10. DEWALT SCREW-BOLT+ (ICC-ES ESR-3889) DEWALT POWER-STUD+ SD2 (ICC-ES ESR-2502)
- DEWALT POWER-STUD SD1 (ICC-ES ESR-2818)
- DEWALT HANGERMATE+ (ICC-ES ESR-3889) DEWALT CCU+ UNDERCUT (ICC-ES ESR-4810)
- 15. DEWALT POWER-BOLT+ (ICC-ES ESR-3260)
- B. ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. DESIGN ADHESIVE BOND STRENGTH HAS BEEN BASED ON ACI 355.4 TEMPERATURE CATEGORY B WITH INSTALLATIONS INTO DRY HOLES DRILLED USING A CARBIDE DRILL BIT INTO CRACKED CONCRETE THAT HAS CURED FOR AT LEAST 21 DAYS. ADHESIVE ANCHORS REQUIRING CERTIFIED INSTALLATIONS, SUCH AS HORIZONTAL TO UPWARD INCLINED ORIENTATION UNDER SUSTAINED TENSION LOADING, SHALL BE INSTALLED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER PER ACI 318-11 D.9.2.2. INSTALLATIONS REQUIRING CERTIFIED INSTALLERS SHALL BE INSPECTED PER ACI 318-11 D.9.2.4. PRE-APPROVED PRODUCTS INCLUDE:
 - 1. SIMPSON STRONG-TIE "SET-3G" (ICC-ES ESR-4057)
 - 2. SIMPSON STRONG-TIE "AT-XP" (IAPMO-UES ER-263)
 - 3. SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508) 4. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT AND
 - VACUUM WITH CONTINUOUSLY DEFORMED REBAR (ICC ESR-3187) 5. HILTI HIT-RE 500 V3 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT
 - AND VACUUM WITH CONTINUOUSLY DEFORMED REBAR (ICC ESR-3814) 6. DEWALT PURE110+ FOR WARM WEATHER/SLOW CURE (ICC-ES ESR-3298); FOR ANCHORS AND REBAR: WHEN DEWALT DUSTX+ EXTRACTION SYSTEM IS USED, TRADITIONAL HOLE CLEANING METHODS USING STEEL BRUSHES AND
 - COMPRESSED DRY AIR MAY BE COMPLETELY OMITTED PER ICC-ES ESR-3298 7. DEWALT AC200+ FOR COLD WEATHER/RAPID CURE (ICC-ES ESR-4027); FOR
 - ANCHORS AND REBAR: WHEN DEWALT DUSTX+ EXTRACTION SYSTEM IS USED, TRADITIONAL HOLE CLEANING METHODS USING STEEL BRUSHES AND COMPRESSED DRY AIR MAY BE COMPLETELY OMITTED PER ICC-ES ESR-4027

8.4 FOR ANCHORING INTO MASONRY:

A. SOLID-GROUTED CONCRETE MASONRY

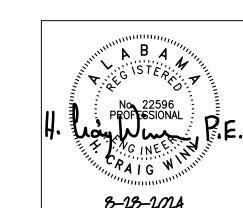
1. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC01 OR ICC-ES AC106. PRE-APPROVED PRODUCTS INCLUDE:

- a. SIMPSON STRONG-TIE "TITEN-HD" & "STAINLESS STEEL TITEN HD"
- (ICC-ES ESR-1056) b. SIMPSON STRONG-TIE "STRONG-BOLT 2" (IAPMO-UES ER-240)
- c. SIMPSON STRONG-TIE "WEDGE-ALL" (ICC-ES ESR-1396)
- d. SIMPSON STRONG-TIE "TITEN TURBO" (IAMPO-UES ER-716) e. HILTI KH-EZ, KH-EZ CRC, KH-EZ SS316, KH-EZ C, AND KH-EZ P
- SCREW ANCHORS (ICC ESR-3056) f. HILTI KWIK BOLT-1 EXPANSION ANCHOR (ICC ER-677)
- g. HILTI KWIK BOLT-TZ2 EXPANSION ANCHOR (ICC ESR-4561)
- h. DEWALT "SCREW-BOLT+" (ICC-ES ESR 4042) i. DEWALT "POWER-STUD+ SD1" (ICC-ES ESR 2966)

- 2. ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC58. PRE-APPROVED PRODUCTS INCLUDE:
- a. SIMPSON STRONG-TIE "AT-XP" (IAPMO-UES ER-281)
- b. SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265) c. HILTI HIT-HY 270 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT AND VACUUM (ICC ESR-4143): STEEL ANCHOR ELEMENT SHALL BE HILTI-HAS CONTINUOUSLY THREADED ROD OR CONTINUOUSLY DEFORMED
- STEEL REBAR d. DEWALT AC100+ GOLD (ICC-ES ESR-3200)
- 8.5 REFER TO THE PROJECT BUILDING CODE AND/OR EVALUATION REPORT FOR SPECIAL INSPECTIONS AND PROOF LOAD REQUIREMENTS.
- 8.6 SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED MAY BE SUBMITTED BY THE CONTRACTOR TO THE EOR FOR REVIEW NO LESS THAN TWO WEEKS PRIOR TO BID. SUBSTITUTIONS WILL ONLY BE CONSIDERED FOR PRODUCTS HAVING A RESEARCH REPORT RECOGNIZING THE PRODUCT FOR THE APPROPRIATE APPLICATION UNDER THE PROJECT BUILDING CODE. SUBSTITUTION REQUESTS SHALL INCLUDE CALCULATIONS PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATE THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE EQUIVALENT. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE, AND INSTALLATION TEMPERATURE.
- 8.7 INSTALL ANCHORS PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII), OR AS INCLUDED IN THE ANCHOR PACKAGING.
- 8.8 THE CONTRACTOR SHALL ARRANGE FOR AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
- 8.9 THE CONTRACTOR SHALL COORDINATE WITH THE OWNER'S SPECIAL INSPECTION AGENCY FOR CONTINUOUS SPECIAL INSPECTION OF ADHESIVE ANCHORS AND PERIODIC INSPECTION OF MECHANICAL ANCHORS, SEE SPECIAL INSPECTION SCHEDULE FOR ADDITIONAL INFORMATION.
- 8.10 ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
- 8.11 EXISTING REINFORCING BARS AND/OR CONDUIT IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS AND/OR REINFORCING TO AVOID CONFLICTS WITH EXISTING REBAR AND/OR CONDUIT. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY GPR, X-RAY, HILTI PS 1000 X-SCAN, CHIPPING, OR OTHER MEANS.

9.0 ELEVATOR

- 9.1 CONTRACTOR PROVIDE W8x28 ELEVATOR HOIST BEAM WITH 3/8" x 7 1/2" x 7 1/2" BEARING PLATE EACH END. FASTEN BEARING PLATE TO CMU WITH (2) 5/8" DIAMETER 5" HEADED STUDS. TOP OF HOIST BEAM TO BE (MINIMUM) 2" CLEAR FROM BOTTOM OF ROOF FRAMING. FILL CELLS UNDER BEARING FOR 32" MIN. DEPTH. POSITION AS REQUIRED BY ELEVATOR MANUFACTURER. COORDINATE ELEVATION WITH ELEVATOR RUN BY REOUIREMENT.
- 9.2 CONTRACTOR FILL ALL CELLS WITH GROUT OR CONCRETE AT ANY ELEVATOR ATTACHMENT POINT. COORDINATE EXACT LOCATIONS WITH ELEVATOR MANUFACTURER AND/OR
- 9.3 IF FRONT OF ELEVATOR SHAFT IS TO BE OMITTED AT BASE FOR ELEVATOR INSTALLATION, CONTRACTOR PROVIDE (MINIMUM) 32" DEEP BOND BEAM REINFORCED WITH 4 LAYERS OF 2#7 CONTINUOUS AND #4 TIES @8 WITH 180 DEGREE HOOK AT EACH END OF THE TIES. ALTERNATE TIE DIRECTION.
- 9.4 ANY ADDITIONAL STEEL REQUIRED FOR ELEVATOR INSTALLATION (SAFETY BEAMS. CLIPS, EMBEDS, ETC.) SHALL BE PROVIDED BY THE ELEVATOR MANUFACTURER AND INCLUDED IN THEIR ORIGINAL PRICE TO THE CONTRACTOR. CONTRACTOR COORDINATE INSTALLATION WITH ELEVATOR MANUFACTURER.
- 9.5 CONTRACTOR COORDINATE HOIST BEAM [AND SEPARATOR BEAM] ELEVATION WITH ELEVATOR MANUFACTURER.



tel 205-824-5200 fax 205-824-5280 Job Number 24-120

MAX HEIGHT OF WALL

ABOVE LINTEL

22'-0"

9'-4"

4'-8"

16'-0"

12'-0"

10'-8"

12" WALL

REINFORCING

2#5 BOT & 2#4 TOP

2#5 BOT & 2#4 TOP

2#6 BOT & 2#4 TOP

2#7 BOT & 2#5 TOP

1#4 BOT

2#4 BOT

LATHAN ARCHITECTS

SCHOOL

퓝 BO,

8-18-1014

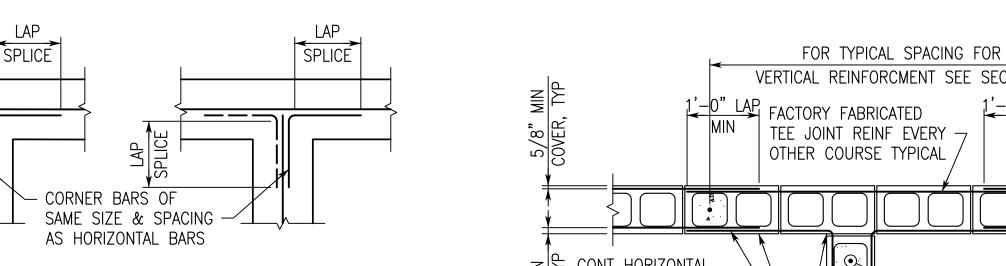
SHEET TITLE: TYPICAL DETAILS

DATE: AUGUST 28, 2024

REVISIONS

JOB NO. **24-39**

SHEET NO:



WALL STEEL TIE-SPREADER DETAIL

HORIZ STEEL

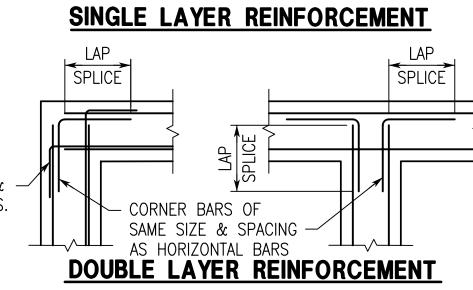
VERT STEEL

TIE SPREADER BAR. LOCATE AT

4 BAR SPACE EACH WAY IN ALL

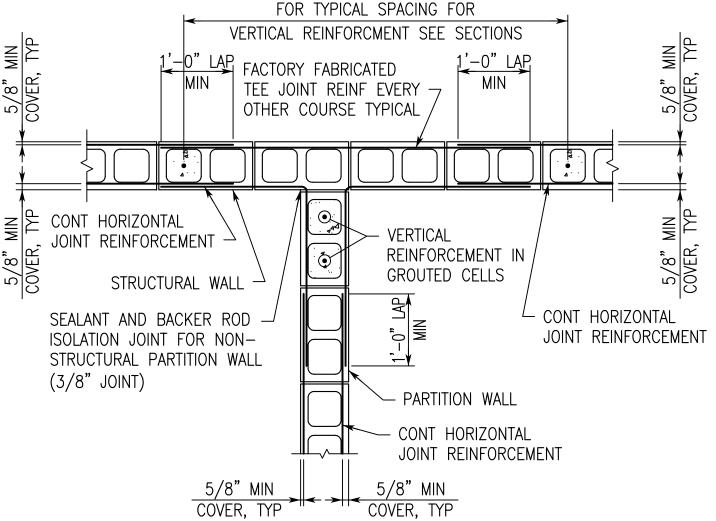
WALLS W/TWO LAYERS OF REINF

DOWEL BARS OF SAME SIZE & SPACING AS HORIZONTAL BARS. LAP 48 BAR DIAMETERS PAST INTERSECTION OF HORIZONTAL



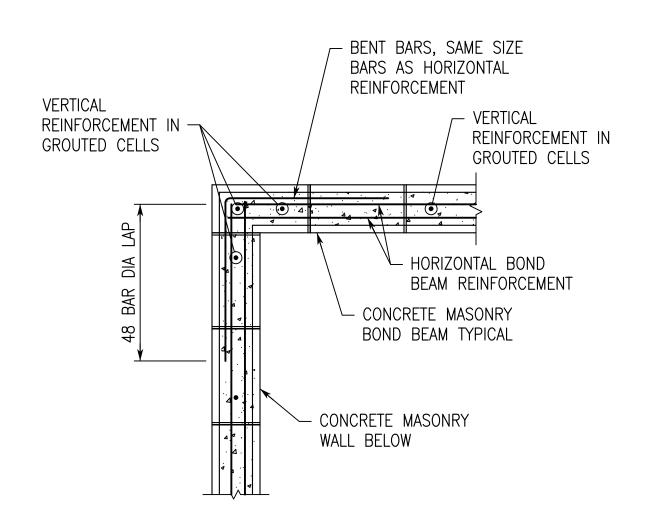
FOOTING, SLAB OR WALL CORNER REINFORCING DETAIL

NOTE: ALL LAP SPLICES CLASS "B" TENSION

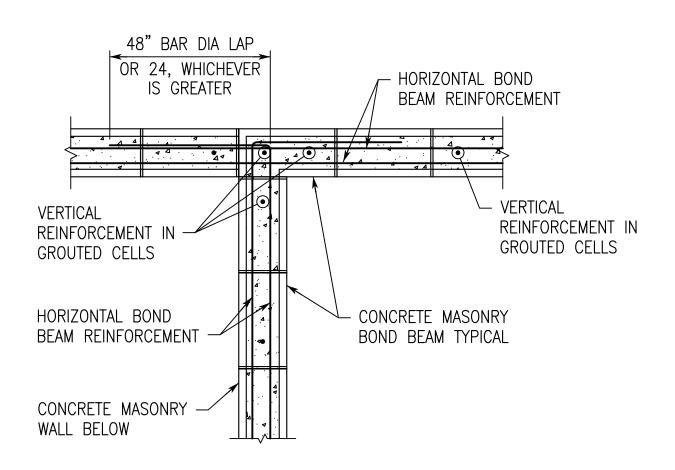


PARTITION WALLS ABUTTING STRUCTURAL WALLS

CONT HORIZONTAL FACTORY FABRICATED CORNER JOINT REINFORCEMENT SECTION OF HORIZONTAL JOINT IN GROUTED CELLS AT FACTORY FABRICATED REINFORCEMENT TEE JOINT REINF EVERY 5/8" COVER, TYPICAL SPACING OTHER COURSE TYPICAL 7 VERTICAL REINFORCEMENT IN GROUTED CELLS-- VERTICAL REINFORCEMENT CONT HORIZONTAL IN GROUTED CELLS JOINT REINFORCEMENT SEE VERTICAL MORTAR JOINT CONT HORIZONTAL JOINT REINFORCEMENT CONT HORIZONTAL JOINT REINFORCEMENT MASONRY WALL COVER. TYP COVER, TYP **PLAN SHOWING JOINT** PLAN SHOWING JOINT REINFORCING



REINFORCEMENT AT WALL CORNER



AT STRUCTURAL WALL INTERSECTION

PLAN SHOWING BOND BEAM REINFORCEMENT AT WALL CORNER

PLAN SHOWING BOND BEAM AT STRUCTURAL WALL INTERSECTION

LOAD BEARING STACK BOND MASONRY LINTEL SCHEDULE

NON-LOAD BEARING

STACK BOND MASONRY LINTEL SCHEDULE

LINTEL DIMENSIONS AND REINFORCING

MAX HEIGHT OF WALI

ABOVE LINTEL

20'-0"

10'-0"

4'-0"

15'-4"

10'-0"

7'-4"

1. DO NOT USE THIS SCHEDULE IF WALL IS LOAD BEARING SUPPORTING ANYTHING OTHER THAN WALL WEIGHT IF WALL IS LOAD BEARING USE THE LOAD BEARING STACK BOND MASONRY LINTEL SCHEDULE.

5. PROVIDE 8" DEEP BOND BEAM REINFORCED WITH 2#4 CONT AT BOTTOM OF ALL OPENINGS. EXTEND 2'-0"

2. PROVIDE 2'-0" MINIMUM BEARING FOR ALL LINTELS. FILL CELLS SOLID AT EACH SIDE OF OPENING AND

3. WHERE MAXIMUM HEIGHT OF WALL ABOVE LINTEL IS EXCEEDED, PROVIDE ADDITIONAL LINTELS EQUALLY

SPACED ABOVE TO LIMIT WALL HEIGHTS ABOVE LINTEL TO THAT SHOWN IN THE TABLE ABOVE.

8" WALL

REINFORCING

1#5 BOT & 1#4 TOP

1#6 BOT & 1#5 TOP

1#7 BOT & 1#5 TOP

1#8 BOT & 1#5 TOP

4. SHORE LINTEL UNTIL MORTAR AND GROUT HAVE SET AND CURED.

1#4 BOT

1#4 BOT

REINFORCE WITH 1#5 BAR CONTINUOUS.

PAST OPENING ON EACH SIDE OF OPENING.

MAXIMUM

OPENING WIDTH

2'-0"

4'-0"

6'-0"

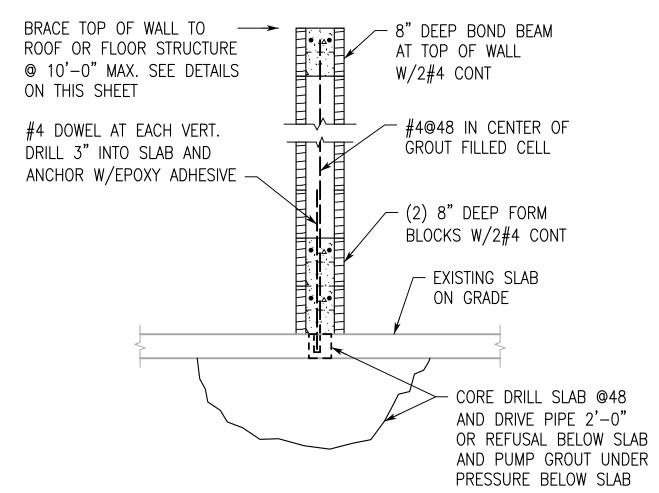
8'-0"

10'-0"

DEPTH

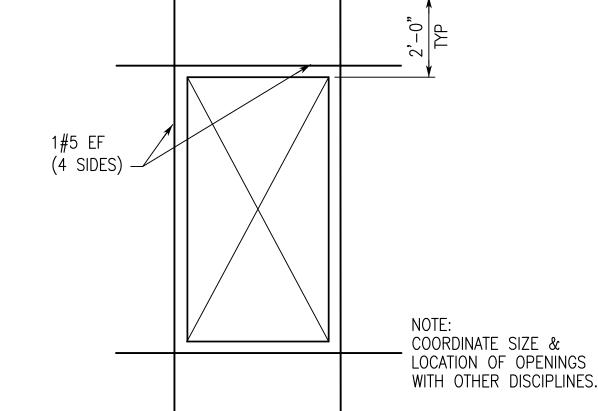
MAXIMUM		LINTEL DIMENSIONS AND REINFORCING					
OPENING WIDTH	DEPTH	8" WALL	12" WALL				
4'-0"	24	2#5 BOT & 2#5 TOP	2#5 BOT & 2#5 TOP				
6'-0"	32	2#5 BOT & 2#5 TOP	2#6 BOT & 2#6 TOP				
8'-0"	32	2#6 BOT & 2#6 TOP	2#6 BOT & 2#6 TOP				
10'-0"	48	2#6 BOT & 2#6 TOP	2#6 BOT & 2#6 TOP				
12'-0"	48	2#6 BOT & 2#6 TOP	2#6 BOT & 2#6 TOP				

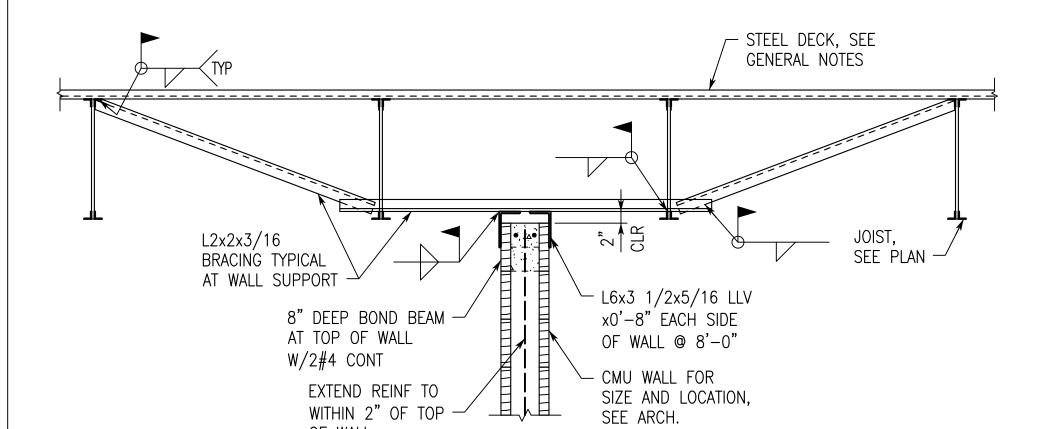
- 1. PROVIDE 24" MINIMUM BEARING FOR ALL LINTELS. FILL CELLS SOLID AT EACH SIDE OF OPENING AND REINFORCE WITH 1#5 BAR CONTINUOUS. (JAMB BARS OF SAME SIZE AS VERTICAL WALL
- REINFORCING BARS.) SHORE LINTEL UNTIL MORTAR AND GROUT HAVE SET AND CURED.
- PROVIDE 8" DEEP BOND BEAM REINFORCED WITH 2#5 CONT AT BOTTOM OF ALL OPENINGS. EXTEND 24" PAST OPENING ON EACH SIDE OF OPENING.



TYPICAL

WALL OPENING NEW WALL BEARING REINFORCEMENT DETAIL **ON EXISTING SLAB**

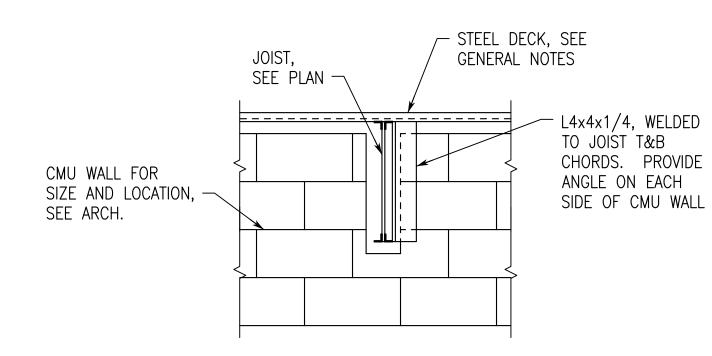




CMU WALL SUPPORT DETAIL (WALL BETWEEN PARALLEL JOISTS) INTERIOR MASONRY WALL BRACING DETAILS

PROVIDE WALL SUPPORT EACH SIDE OF WALL @ 8'-0". PROVIDE WALL SUPPORT WHERE CONTINUOUS WALL SPAN BETWEEN

PERPENDICULAR WALL EXCEEDS 20'-0". UNLESS NOTED.



CMU WALL SUPPORT DETAIL (WALL PERPENDICULAR TO JOIST) INTERIOR MASONRY WALL BRACING DETAILS

PROVIDE WALL SUPPORTS AT EVERY JOIST WHERE CONTINUOUS WALL SPAN BETWEEN PERPENDICULAR WALLS EXCEEDS 20'-0", UNLESS NOTED.

	COMPONENTS AND CLADDING WIND LOADS FOR ROOF (PSF)										
114 MPH VELOCITY (3-SEC. GUST) ROOF OVERHANG											
H = 23'-4'' 0:12 Roof Slope	EFFECTIVE WIND AREA (FT²)	Positive Max. Net Pressure 'p' (PSF)	Zone 1' (Int.) (PSF)	Zone 1 (Int.) (PSF)	Zone 2 (Edge) (PSF)	Zone 3 (Corner) (PSF)	Zone 1' & 1 (Int.) - Max. Net Pressure 'p' (PSF)	Zone 2 (Edge) - Max. Net Pressure 'p' (PSF)	Zone 3 (Corner) - Max. Net Pressure 'p' (PSF)		
	10	16.0	-28.5	-49.5	-65.4	-89.1	-44.8	-60.6	-84.3		
	20	16.0	-28.5	-46.3	-61.2	-80.7	-44.0	-55.0	-74.5		
	50	16.0	-28.5	-42.0	-55.6	-69.6	-43.0	-47.6	-61.6		
	100	16.0	-28.5	-38.7	-51.4	-61.2	-42.2	-42.0	-51.8		
	200	16.0	-24.5	-35.4	-47.2	-52.8	-35.4	-36.4	-42.0		
	500	16.0	-19.3	-31.1	-41.6	-41.6	-26.4	-29.0	-29.0		

- WIDTH OF EDGE STRIP 'a' = 3'-0".
- 2. VALUES SHOWN ABOVE HAVE BEEN ADJUSTED FOR BUILDING HEIGHT AND EXPOSURE ACCORDING TO ASCE 7-16 STANDARD TABLE 30.3-1. VALUES SHOWN ARE ULTIMATE
- PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACES.
- 4. EFFECTIVE WIND AREA IS THE SPAN LENGTH MULTIPLIED BY AN EFFECTIVE WIDTH THAT NEED NOT BE LESS THAN ONE-THIRD THE SPAN LENGTH.
- 5. CONSIDER 5 PSF MINIMUM DEAD LOAD FOR UPLIFT CALCULATIONS FOR ROOF FRAMING MEMBERS AND 2 PSF MINIMUM DEAD LOAD FOR UPLIFT CALCULATIONS FOR ROOF DECK.
- WIND PRESSURES IN THESE TABLES SHALL BE MULTIPLIED BY 0.6 TO OBTAIN NOMINAL WIND PRESSURES.

	1	0.6h	0.6h		I
		2	 	1')	
EDGE ZONE	INT ZONE	0.2h	1	<	0.6h
		3	0.2	2	0.6h
a →	' -		Ť		1

WALLS FLAT ROOFS WALL AND ROOF WIND PRESSURE ZONE DIAGRAMS

COMPONENTS AND CLADDING WIND LOADS FOR WALLS (PSF)

FOR WALLS (PSF)							
	EFFECTIVE	114 MPH VELOCITY (3-SEC. GUST)					
H = 23'-4" 0:12 Roof Slope	WIND AREA (FT²)	ZONES 4 & 5	ZONES 4 (Int.)	ZONES 5 (Edge)			
	10	28.5	-30.8	-30.8			
	20	27.2	-29.6	-30.8			
	50	25.5	-27.î	-32.1			
	100	24.3	-26.6	-29.6			
	200	23.0	-25.3	-27.1			
	500	21.3	-23.7	-23.7			

- 1. WIDTH OF EDGE STRIP 'a' = 3'-0".
- 2. VALUES SHOWN ABOVE HAVE BEEN ADJUSTED FOR BUILDING HEIGHT AND EXPOSURE ACCORDING TO ASCE 7-16 STANDARD TABLE 30.3-1. VALUES SHOWN ARE ULTIMATE
- 3. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACES.
- 4. EFFECTIVE WIND AREA IS THE SPAN LENGTH MULTIPLIED BY AN EFFECTIVE WIDTH THAT NEED NOT BE LESS THAN ONE-THIRD THE SPAN LENGTH.
- 5. WIND PRESSURES IN THESE TABLES SHALL BE MULTIPLIED BY 0.6 TO OBTAIN NOMINAL WIND PRESSURES.

8-18-1014

LATHAN

ARCHITECTS

SCHOOL

SHEET TITLE: TYPICAL DETAILS

PROJ. MGR.: HCW DRAWN: ABS

DATE: AUGUST 28, 2024 REVISIONS

JOB NO. **24-39**

SHEET NO:

SCHOOL

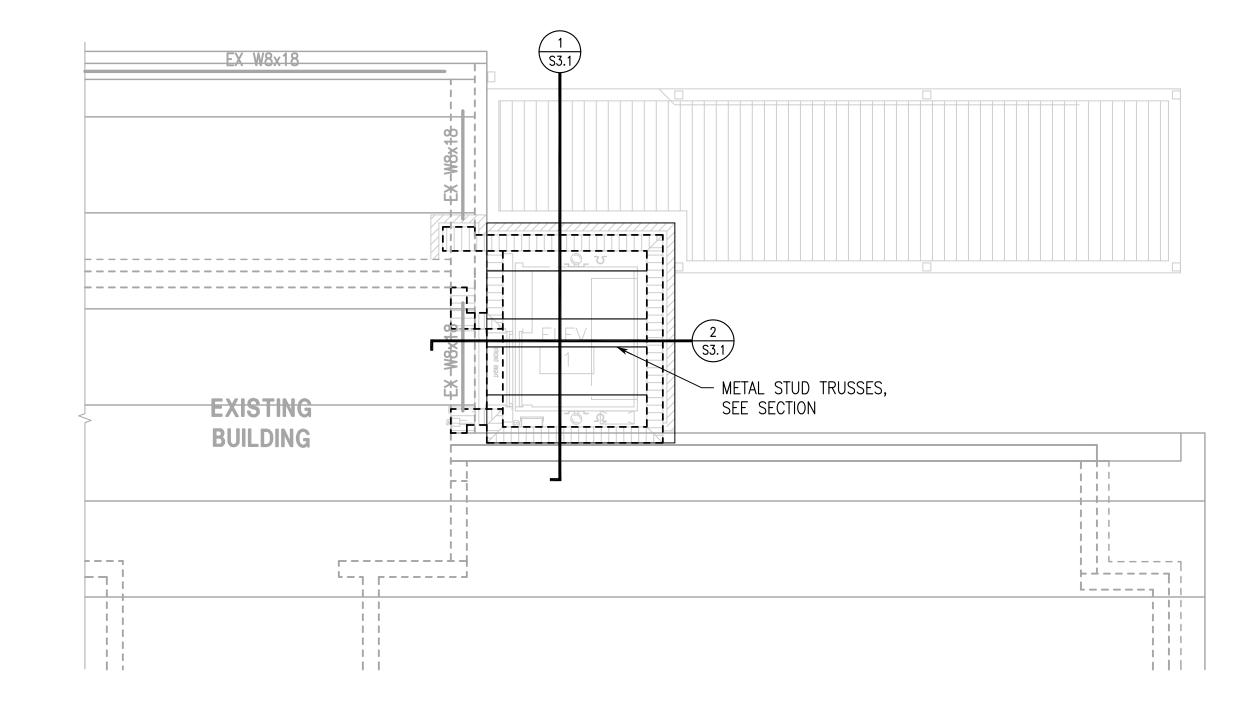
LOCUST FORK ELEMENTARY
155 School Road Locust Fork, AL 35097
BLOUNT COUNTY BOARD OF EDUCATION

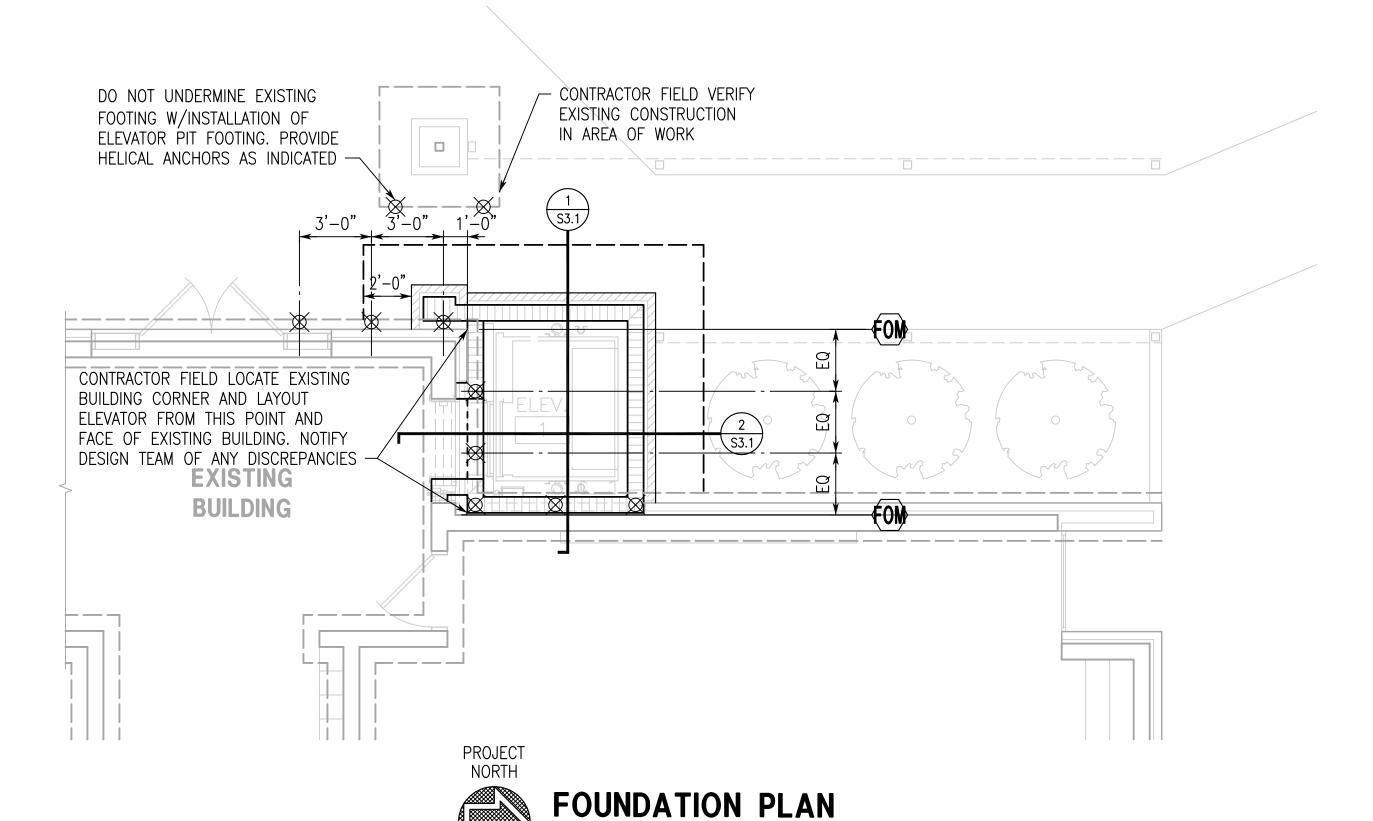
PROJECT NORTH

ROOF FRAMING PLAN

BUILDING IS EXISTING. ELEVATOR SHAFT AND FLOOR ACCESS IS NEW. TOP OF WALL, SEE ARCHITECTURAL DRAWINGS.

. TOP OF WALL, SEE ARCHITECTURAL DRAWINGS.
. ROOF SYSTEM: METAL STUD TRUSSES SUPPORTING 1 1/2" DEEP x22 GAGE
METAL DECKING. ANCHOR DECKING TO TRUSS WITH #12 SCREWS AT 6".





BUILDING IS EXISTING. ELEVATOR SHAFT AND FLOOR ACCESS IS NEW.

3. TOP OF FOOTING ELEVATION -2'-0" UNLESS NOTED. CONTRACTOR TO VERIFY ALL EXISTING FOOTING TYPES AND ELEVATIONS IN AREA OF WORK.

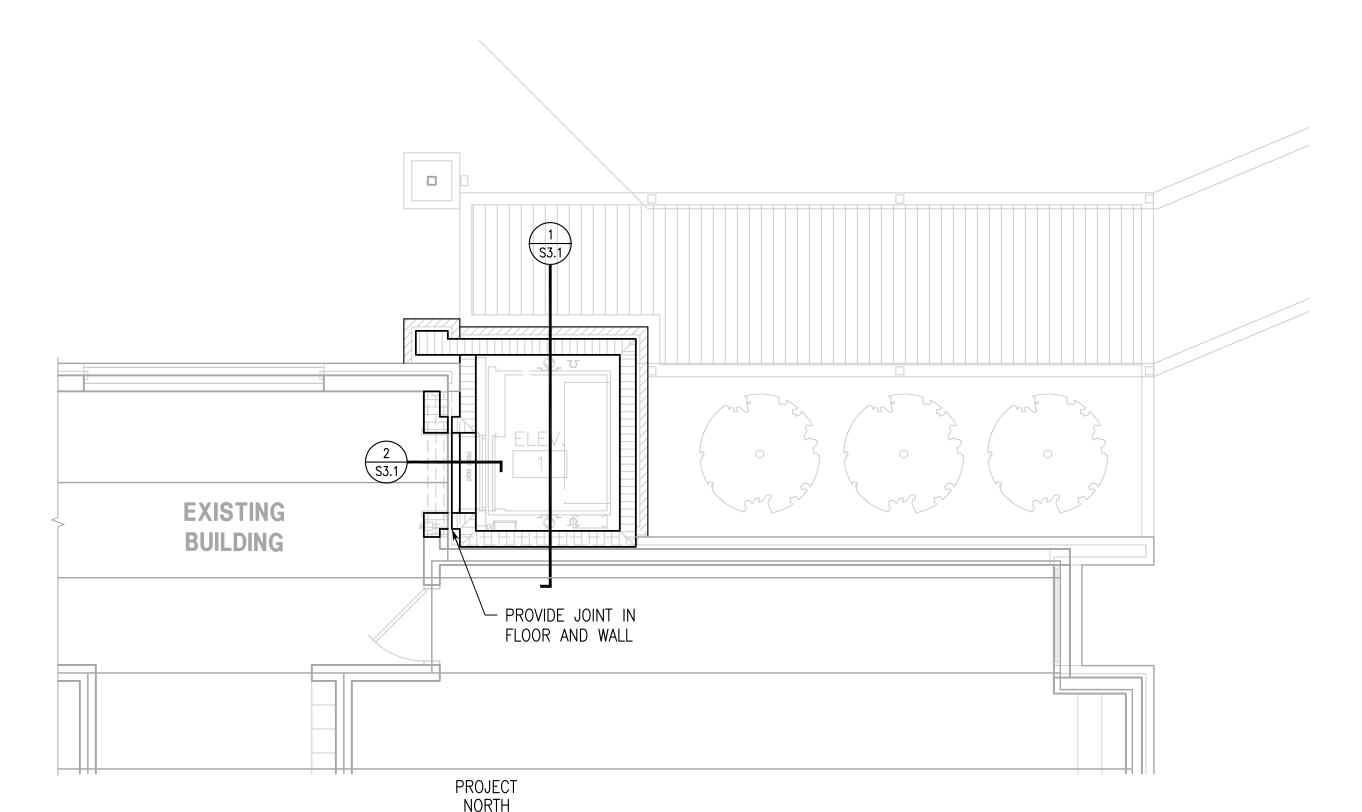
4. 💢 INDICATES POST-INSTALLED FOUNDATION ANCHOR. ANCHOR SHALL BE DESIGNED BY THE

TO EXISTING FOOTING TO TRANSFER NOTED LOADING FROM FOOTING INTO ANCHOR BY THE

CONTRACTOR TO RESIST A 40k AXIAL LOAD AND A 5k TENSILE LOAD. ANCHOR TO BE ATTACHED

2. FINISH FLOOR ELEVATION 0'-0", MATCH EXISTING.

CONTRACTOR.



UPPER FRAMING PLAN

4"=1'-0"

1. FINISH FLOOR ELEVATION 11'-4", MATCH EXISTING.

DRAWN: ABS

DATE: AUGUST 28, 2024

REVISIONS

8-18-2014

SHEET TITLE:

FOUNDATION,

AND ROOF

PROJ. MGR.:

UPPER FRAMING

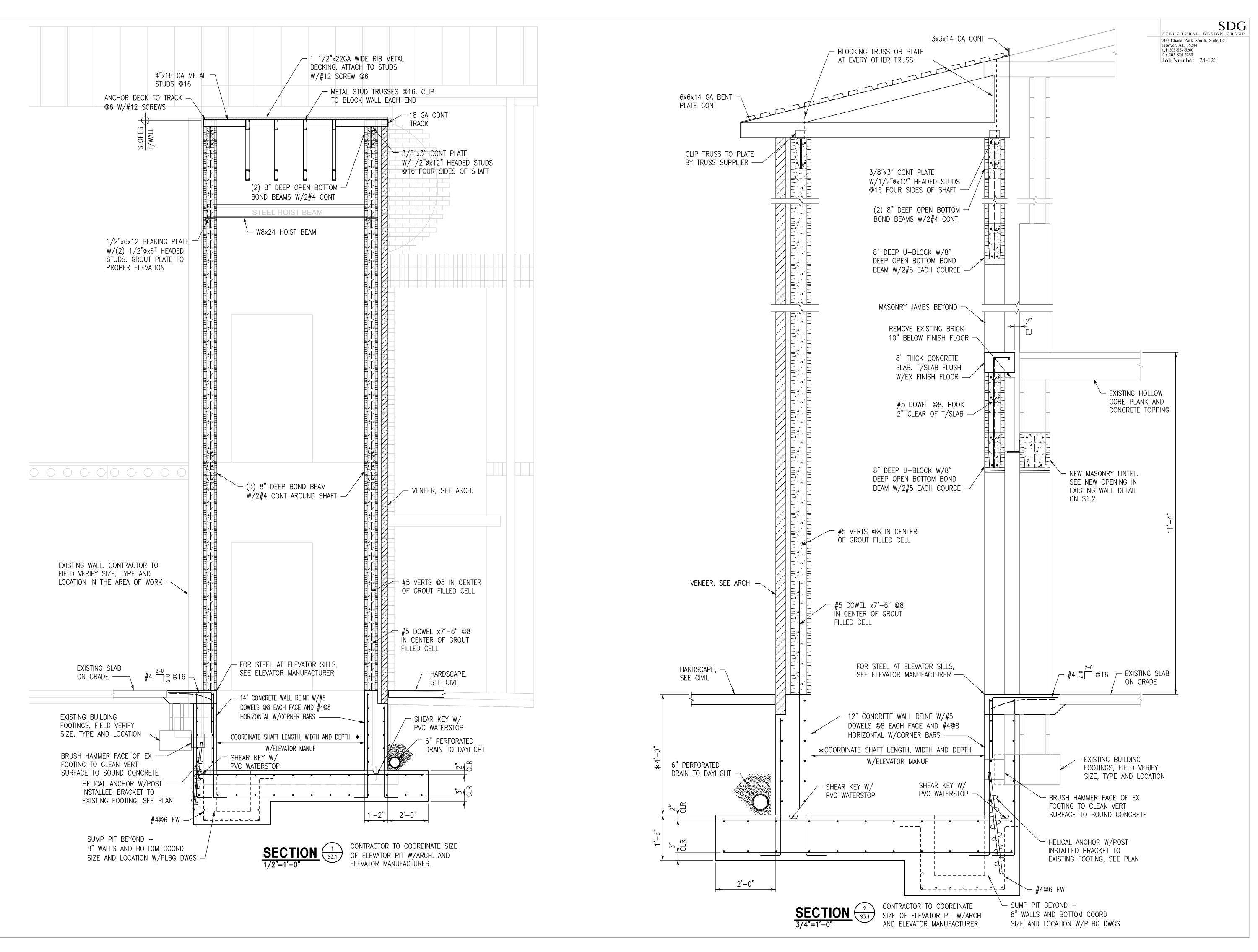
FRAMING PLAN

JOB NO. **24-39**

SHEET NO:

5 OF 6

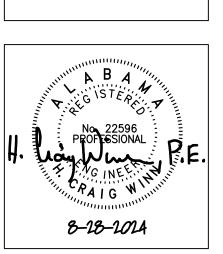
0 1" 2





W ELEVATOR FOR
UST FORK ELEMENTARY SCHOo

BO,



SHEET TITLE:
SECTIONS
AND DETAILS

PROJ. MGR.: HC

DATE: AUGUST 28, 2024

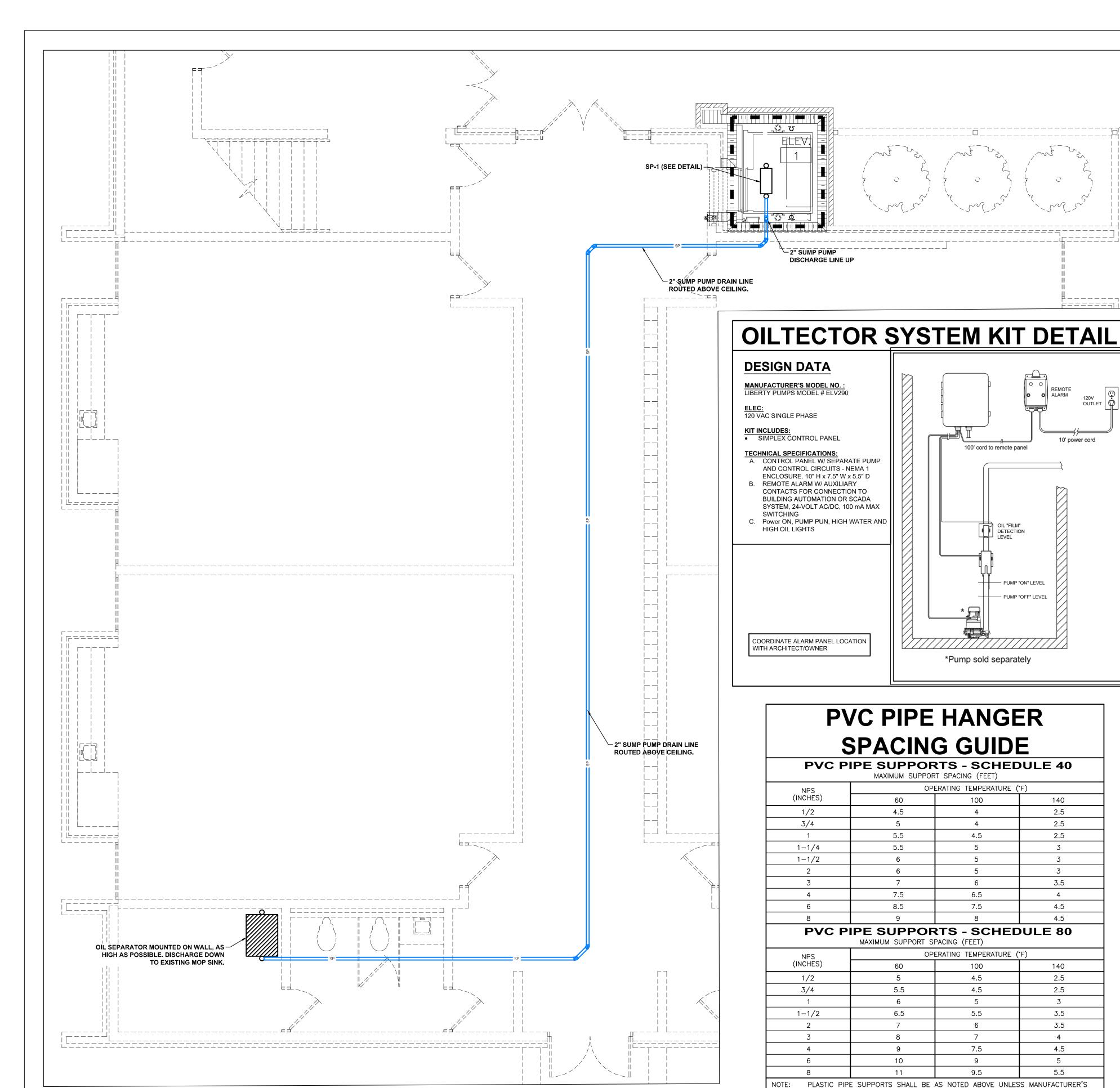
REVISIONS

JOB NO. **24-39**

SHEET NO:

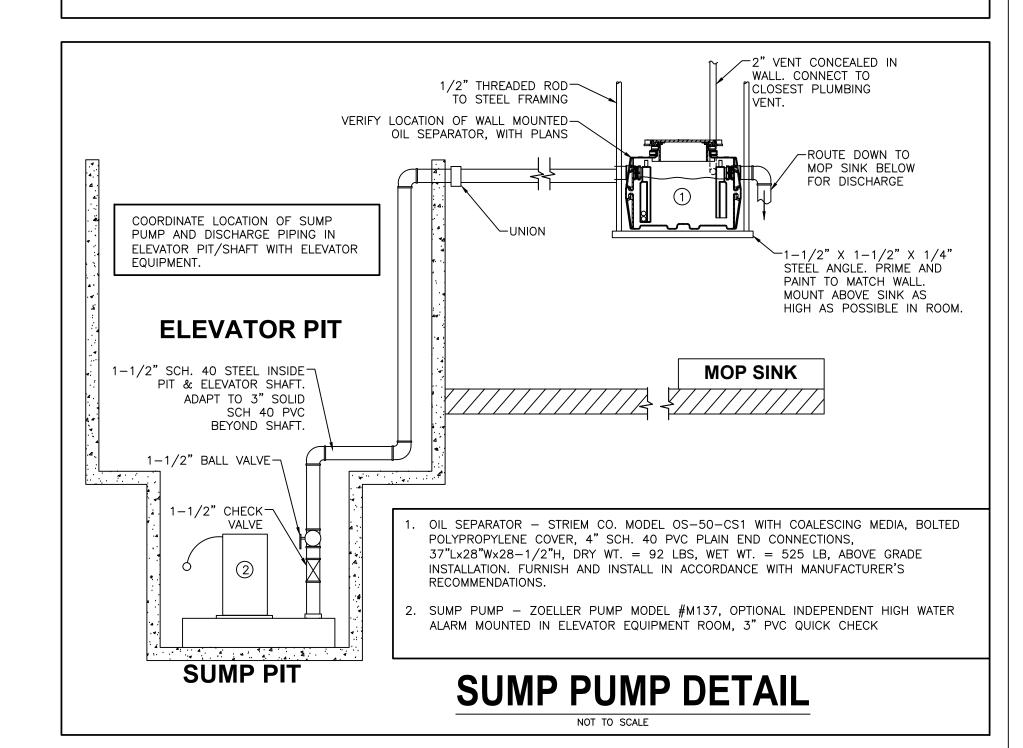
6 OF 6

0 1"



PLUMBING NOTES

- THESE DRAWINGS ARE SCHEMATIC IN NATURE AND ARE NOT INTENDED TO SHOW ALL POSSIBLE CONDITIONS. IT IS INTENDED THAT A COMPLETE PLUMBING SYSTEM BE PROVIDED WITH ALL NECESSARY EQUIPMENT, ACCESSORIES, AND CONTROLS COMPLETELY COORDINATED WITH ALL TRADES. ALL REQUIREMENTS GIVEN IN THESE DOCUMENTS SHALL BE STRICTLY CONFORMED TO. ANY ITEMS AND LABOR REQUIRED FOR A COMPLETE PLUMBING SYSTEM IN ACCORDANCE WITH ALL APPLICABLE CODES, STANDARDS, LOCAL AUTHORITIES, AND THESE CONTRACT DOCUMENTS SHALL BE FURNISHED WITHOUT INCURRING ANY ADDITIONAL COST TO THE OWNER. CAREFULLY REVIEW ALL CONTRACT DOCUMENTS AND THE DESIGN OF OTHER TRADES BEFORE PREPARING SHOP DRAWINGS.
- COORDINATE ALL WORK WITH ARCHITECTURAL, STRUCTURAL, HVAC, AND ELECTRICAL TRADES. PIPE ROUTING SHOWN IS DIAGRAMMATIC. PROVIDE ALL OFFSETS, ETC., TO AVOID INTERFERENCES WITH EQUIPMENT, PIPING, DUCTWORK, LIGHTS, CONDUIT, ETC.
- 3. FIELD VERIFY EXACT SIZE, MATERIAL, AND LOCATION OF ALL EXISTING UTILITIES BEFORE BEGINNING WORK.
- 4. COORDINATE ALL FLOOR PENETRATIONS WITH STRUCTURAL DRAWINGS. SET SLEEVES IN FLOORS/WALLS AND ATTACHMENTS FOR HANGERS AS CONSTRUCTION PROGRESSES. ALL PENETRATIONS MUST BE SEALED AND HELD AS TIGHT TO COLUMNS OR WALLS AS POSSIBLE.
- 5. ALL PIPING SHALL BE CONCEALED INSIDE WALLS, WITHIN PIPE CHASES, OR ABOVE CEILINGS. HOLD ALL PIPING ABOVE CEILING AS HIGH AS POSSIBLE.
- PLUMBING CONTRACTOR SHALL MAKE FINAL CONNECTIONS TO ALL EQUIPMENT INDICATED ON DRAWINGS FINAL CONNECTION SHALL INCLUDE ANY ADAPTORS, NIPPLES, SHUT-OFF VALVES, PRV'S, SHOCK ABSORBERS, BACKFLOW PREVENTION DEVICES, REGULATORS, ETC.
- ALL STRUCTURAL PENETRATIONS (SLEEVES, BLOCK OUTS, ETC.) ARE TO BE LOCATED AND COORDINATED IN THE FIELD BY THE CONTRACTOR IN RELATION TO THE REQUIREMENTS OF FINAL EQUIPMENT AND FIXTURES SELECTED



SUMP PUMP SCHEDULE FLOW DATA MANUFACTURER'S ELECTRICAL RPM DISCHARG MODEL NO. LIBERTY PUMPS ELV290 115-1-60 1750 1-1/2" OR APPROVED EQUAL 10.4 FLA 20 NOTE:

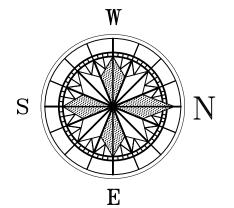
PLUMBING LEGEND

SUMP PUMP DISCHARGE

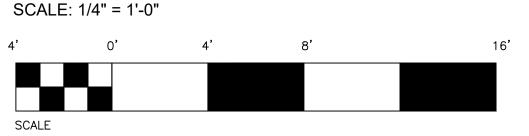
PROVIDE AND INSTALL WITH OILTECTOR SYSTEM KIT (SEE DETAIL)

CODES AND STANDARDS

- 2021 INTERNATIONAL PLUMBING CODE
- 2021 INTERNATIONAL MECHANICAL CODE
- 2021 EXISTING BUILDING CODE



MAIN FLOOR PLUMBING PLAN





RECOMMENDATION IS MORE STRINGENT FOR THE APPLICATION.

PLUMBING DRAWING INDEX						
EET NO.	SHEET TITLE					
P1.1	MAIN FLOOR PLUMBING PLAN		F			

10' power cord

100' cord to remote panel

PUMP "ON" LEVEL

*Pump sold separately

OPERATING TEMPERATURE (°F)

6.5

7.5

140

2.5

2.5

3.5

4.5

2.5

2.5

3.5

3.5

5.5

MAXIMUM SUPPORT SPACING (FEET)

4.5

5.5

5.5

7.5

8.5

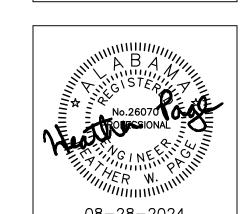
5.5

MAXIMUM SUPPORT SPACING (FEET)

WHORTON ENGI	NEERING, INC.
HVAC — PLUMBING — F	PROCESS CONTROL
RANDALL WHORTON, P.E. PHONE: (256) 820–9897	25 SUMMERALL GATE ROAD ANNISTON, ALABAMA 36205



S



MAIN FLOOR PLUMBING PLAN

DATE: AUGUST 28, 2024 REVISIONS

JOB NO. **24-39**

SHEET NO:

1 OF 1

WHORTON ENGINEERING PROJECT NO. 24153

LIGHTING FIXTURE SCHEDULE

MARK	MANUFACTURER	FACTURER CATALOG NO.	LAMPS		MOUNTING	TYPE	RECESS	REMARKS
IVIALIN			NO.	WATTS	TYPE	HEIGHT	MOUNTING	DEPTH
А	PHOENIX	VA-W-17LED- WW-FGC-120	FURNISHED WITH FIXTURE		AS NOTED	BRACKET		

NOTES:

- 1. VERIFY ALL FIXTURE COLORS WITH ARCHITECT PRIOR TO SUBMITTALS.
- 2. EQUAL FIXTURES BY LITHONIA, PARKER, DAYBRITE, AND COLUMBIA WILL BE CONSIDERED APPROVED EQUALS.

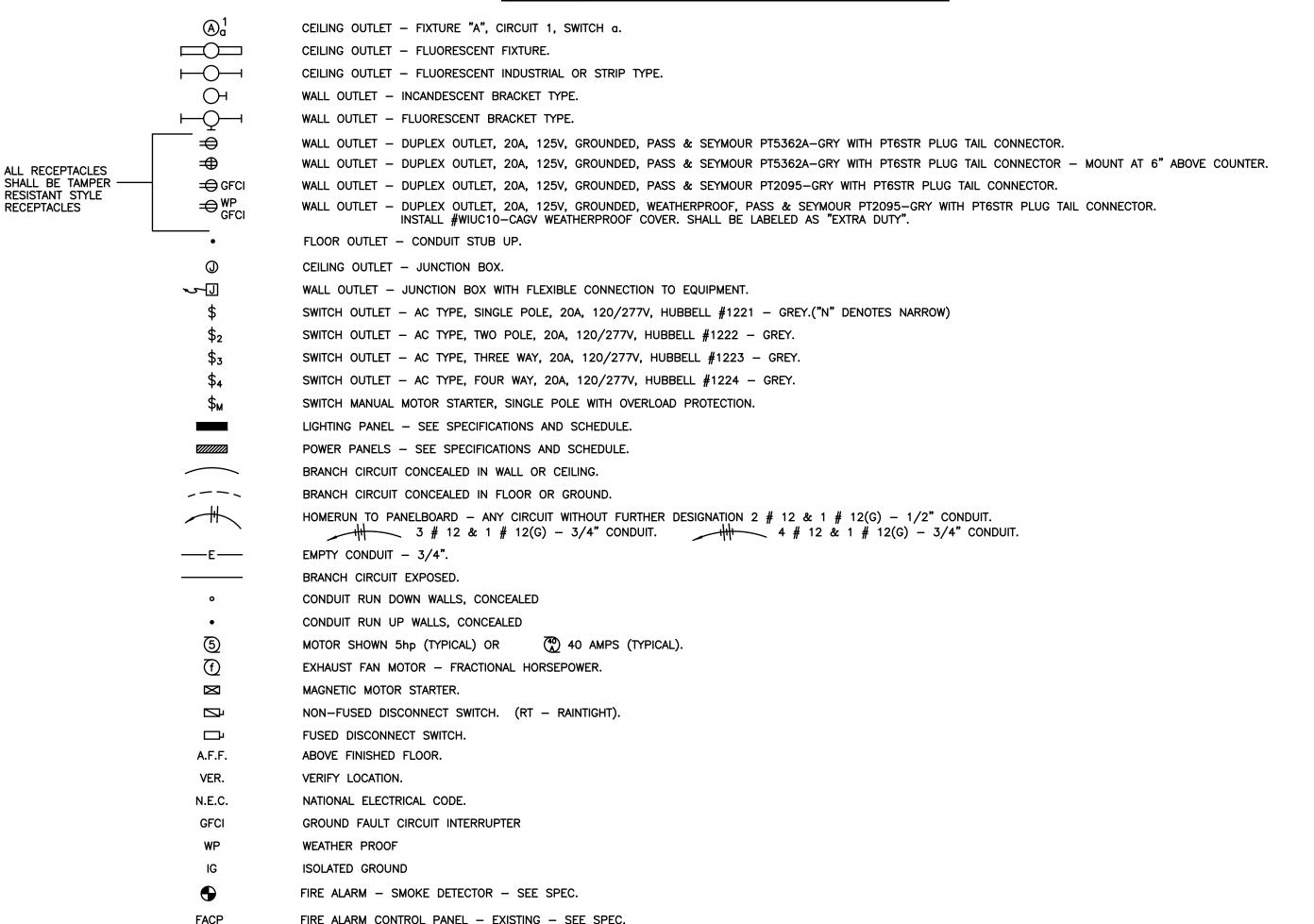
GENERAL NOTES

- 1. SERVICE TO PROJECT IS 120/208 VOLTS, 3 PHASE, 4 WIRE.
- 2. VERIFY ALL DOOR SWINGS WITH ARCHITECTURAL DRAWINGS BEFORE ROUGHING IN SWITCHES.
- 3. VERIFY EXACT LOCATION OF ALL MOTORS AND EQUIPMENT BEFORE ROUGHING IN.
- 4. CONTRACTOR TO VERIFY LOCATION OF ALL OUTLETS PRIOR TO INSTALLATION.
- 5. THE ELECTRICAL CONTRACTOR SHALL VERIFY THE EXACT LOCATION OF COUNTERTOPS AND BACKSPLASHES ON ARCHITECTURAL DETAILS AND/OR CASEWORK SHOP DRAWINGS AND ADJUST SPECIFIED MOUNTING HEIGHT OF WALL OUTLETS AS REQUIRED TO AVOID CONFLICTS.
- CONTRACTOR WILL CHECK ALL LIGHTING FIXTURES FOR EXACT TYPE MOUNTING AND SPACE REQUIRED BEFORE ROUGHING IN.
- 7. SUPPORT OF ALL LIGHTING FIXTURES TO BE THE RESPONSIBILITY OF THIS CONTRACTOR. FIXTURES TO BE SUPPORTED INDEPENDENT OF CEILING FROM STRUCTURAL MEMBERS OF THE BUILDING.
- 8. ELECTRICAL CONTRACTOR MUST CHECK THE CORRESPONDING MECHANICAL SHEETS AND BE RESPONSIBLE FOR INCLUDING PROPER SERVICE AND CONTRACTOR TO ALL MECHANICAL ITEMS SHOWN THEREON REGARDLESS OF ITS BEING OR NOT BEING SHOWN ON
- 9. ALL CONDUIT CONCEALED UNLESS SPECIFICALLY SHOWN EXPOSED.
- 10. COORDINATE SERVICES WITH POWER AND COMMUNICATIONS COMPANIES. REMOVE OR RELOCATE ALL POWER AND COMMUNICATIONS CIRCUITS ABOVE OR BELOW GRADE THAT WOULD OBSTRUCT THE CONSTRUCTION OF THE PROJECT OR CONFLICT IN ANY MANNER WITH COMPLETION OF THE PROJECT OR ANY CODE PERTAINING THERETO. IF UTILITY COMPANY REQUIREMENTS ARE AT VARIANCE WITH THESE DRAWINGS AND SPECIFICATIONS, THE CONTRACT PRICE SHALL INCLUDE THE ADDITIONAL COST.
- 11. IT IS INTENDED THAT SPECIFICATIONS AND PLANS SHALL INCLUDE EVERYTHING REQUIRED AND NECESSARY FOR PROPER AND COMPLETE INSTALLATION OF THE COMPLETE SYSTEMS SHOWN EVEN THOUGH EVERY ITEM MAY NOT BE PARTICULARLY MENTIONED IN DETAIL. THE CONTRACTOR SHALL DELIVER TO OTHER TRADES ANY EQUIPMENT THAT MUST BE INSTALLED DURING CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR FIELD MEASUREMENTS AND COORDINATION OF THE PHYSICAL SIZE OF ALL EQUIPMENT WITH THE ARCHITECTURAL REQUIREMENTS OF THE SPACES INTO WHICH THE EQUIPMENT WILL BE INSTALLED.
- 12. THIS CONTRACTOR SHALL INSTALL EQUIPMENT GROUNDS THROUGHOUT THIS PROJECT, USING GREEN INSULATED GROUND WIRE. USE OF CONDUIT AS THE ONLY GROUND CONDUCTOR WILL NOT BE ALLOWED. (SIZE GROUND WIRES PER N.E.C.)
- 13. REMOVE ALL EXISTING PANELBOARDS, DISCONNECTS, FIXTURES, RECEPTACLES, AUXILIARY SYSTEM DEVICES, CONDUIT, CONDUCTORS, ETC. BEING RENDERED OBSOLETE BY THIS PROJECT.
- 14. WHERE EXISTING REMAINING CIRCUITS ARE BEING INTERRUPTED DUE TO STRUCTURAL AND/OR DESIGN CHANGES, THIS CONTRACTOR WILL EXTEND EXISTING CIRCUITS AS REQUIRED TO MAINTAIN CIRCUIT CONTINUITY TO REMAINING ACTIVE DEVICES.

FIRE ALARM SYSTEM NOTES

- 1. PROVIDE FIRE ALARM COMPLETION DOCUMENTS AT THE STATE FINAL INSPECTION. THIS ITEM WILL BE REQUIRED BY STATE BUILDING INSPECTOR AT THE TIME OF FINAL INSPECTION (OLD CERTIFICATION FORM).
- 2. ADDITIONS AND ALTERATIONS TO THE FIRE ALARM SYSTEM REQUIRE TESTING, A RECORD OF COMPLETION, AND RECERTIFICATION. ALL FIRE ALARM WORK SHALL BE PERFORMED BY QUALIFIED PERSONNEL AS DEFINED IN NFPA-72 (2013) 10.4.2, 10.5.2, AND 10.18.1.
- 3. ALL WORK SHALL BE PERFORMED BY A CERTIFIED FIRE ALARM CONTRACTOR SEE SPECS.

ELECTRICAL SYMBOLS



COLOR CODE FOR JUNCTION BOXES

NOTE:
PAINT ALL JUNCTION BOXES AND COVERS WITH COLORS AS SHOWN BELOW. PAINTING COVERS ONLY IS NOT ACCEPTABLE.

FUNCTION:

LIGHTING

POWER

FIRE ALARM

COLOR:

GREEN

RED

COLOR CODE FOR ELECTRICAL WIRING

1. 120/208 V, 60Hz, 3 PHASE, 4 WIRE SYSTEM PHASE A-BLACK
B-RED
C-BLUE
N-WHITE

2. GROUND-GREEN

STEWART ENGINEERING ELECTRICAL CONSULTANTS

P.O. Box 2233 (36202)
300 East 7th Street (36207)
Anniston, Alabama
Phone: 256/237-0891
Fax No.: 256/237-1077
Email: services@stewartengineering.org

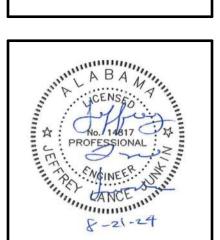
Engineer:

J. Lance Junkin, P.E. Alabama Reg. 14817 <u>Project Nun</u> 2482

Project Number:

LATHAN ARCHITECTS

EW ELEVATOR FOR SCHOOL ROAD School Road Locust Fork, AL 35097

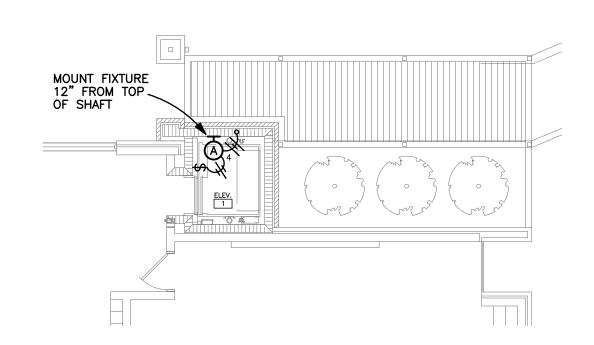


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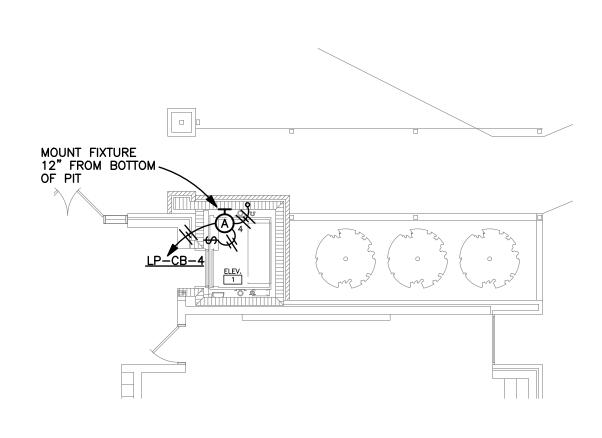
SCHEDULES, SYMBOLS,
AND NOTES

PROJ.	MGR.:	LANG	CE .	JUNKIN
DRAWN	l:			SEC
DATE:	AUG	SUST	28,	2024
REVISION	SNC			

JOB NO. **24-39**SHEET NO: **E1.1**1 OF 2



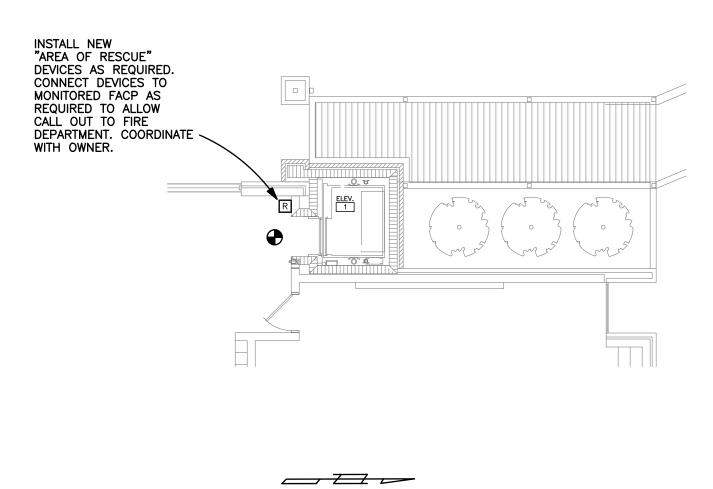
PARTIAL UPPER LEVEL FLOOR PLAN - LIGHTING SCALE: 1/8" = 1'-0"



PARTIAL MAIN LEVEL

FLOOR PLAN - LIGHTING

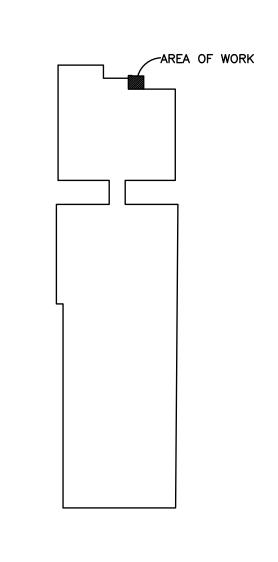
SCALE: 1/8" = 1'-0"



PARTIAL UPPER LEVEL

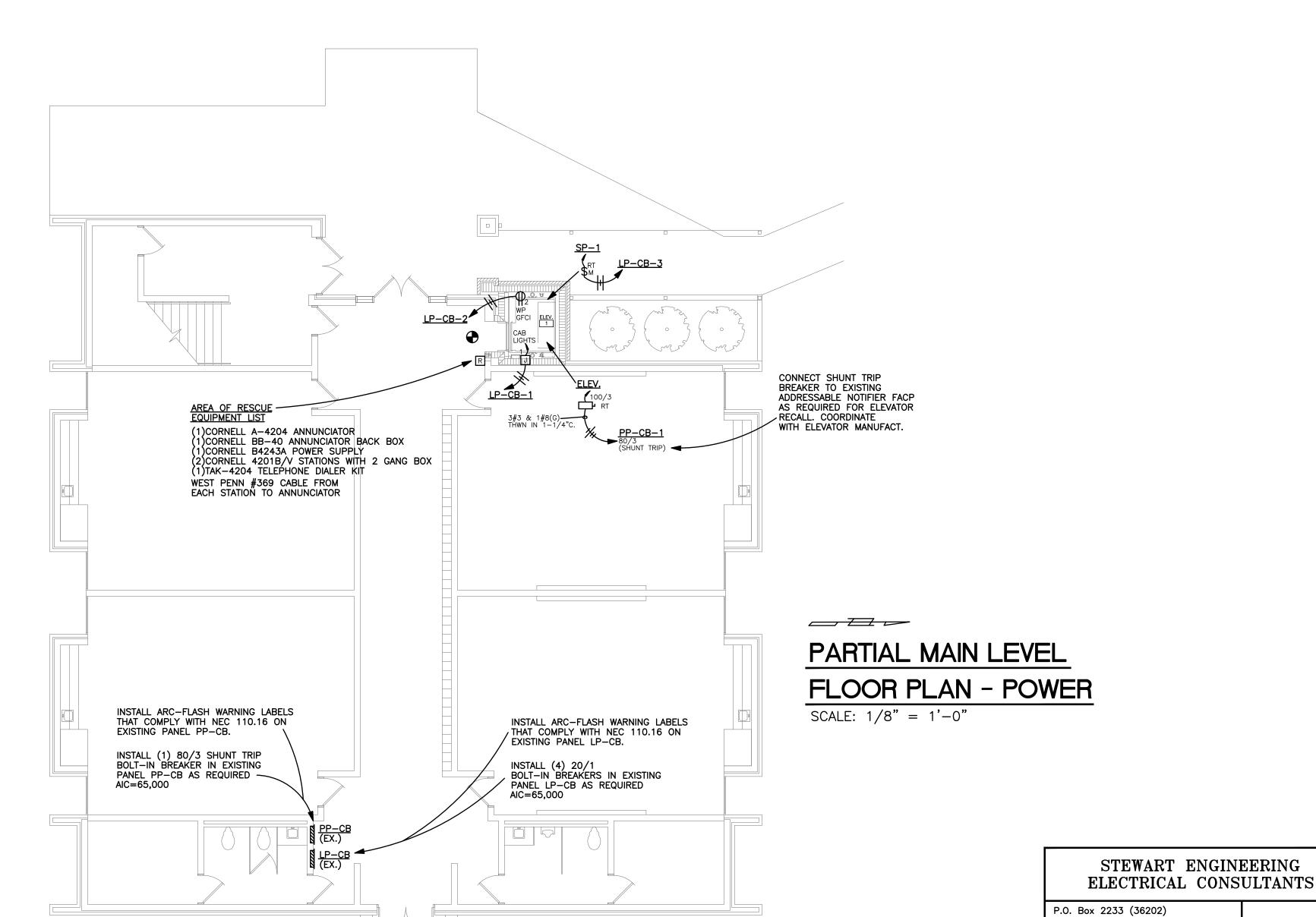
FLOOR PLAN - POWER

SCALE: 1/8" = 1'-0"



KEY PLAN

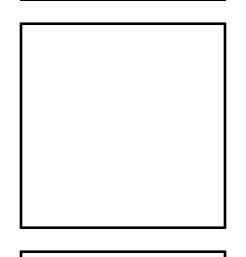
N.T.S.



LATHAN

RY SCHOOL
L 35097

EW ELEVATOR FOR CUST FORK ELEMENTARY School Road Locust Fork, AL 350





SHEET TITLE:

ELECTRICAL PLAN

PROJ. MGR.: LANCE JUNKIN
DRAWN: SEC
DATE: AUGUST 28, 2024
REVISIONS

JOB NO. 24-39
SHEET NO: **E2.1**

300 East 7th Street (36207)

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J. Lance Junkin, P.E. Alabama Reg. 14817 Project Number:

Anniston, Alabama

Engineer:

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2 OF 2

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