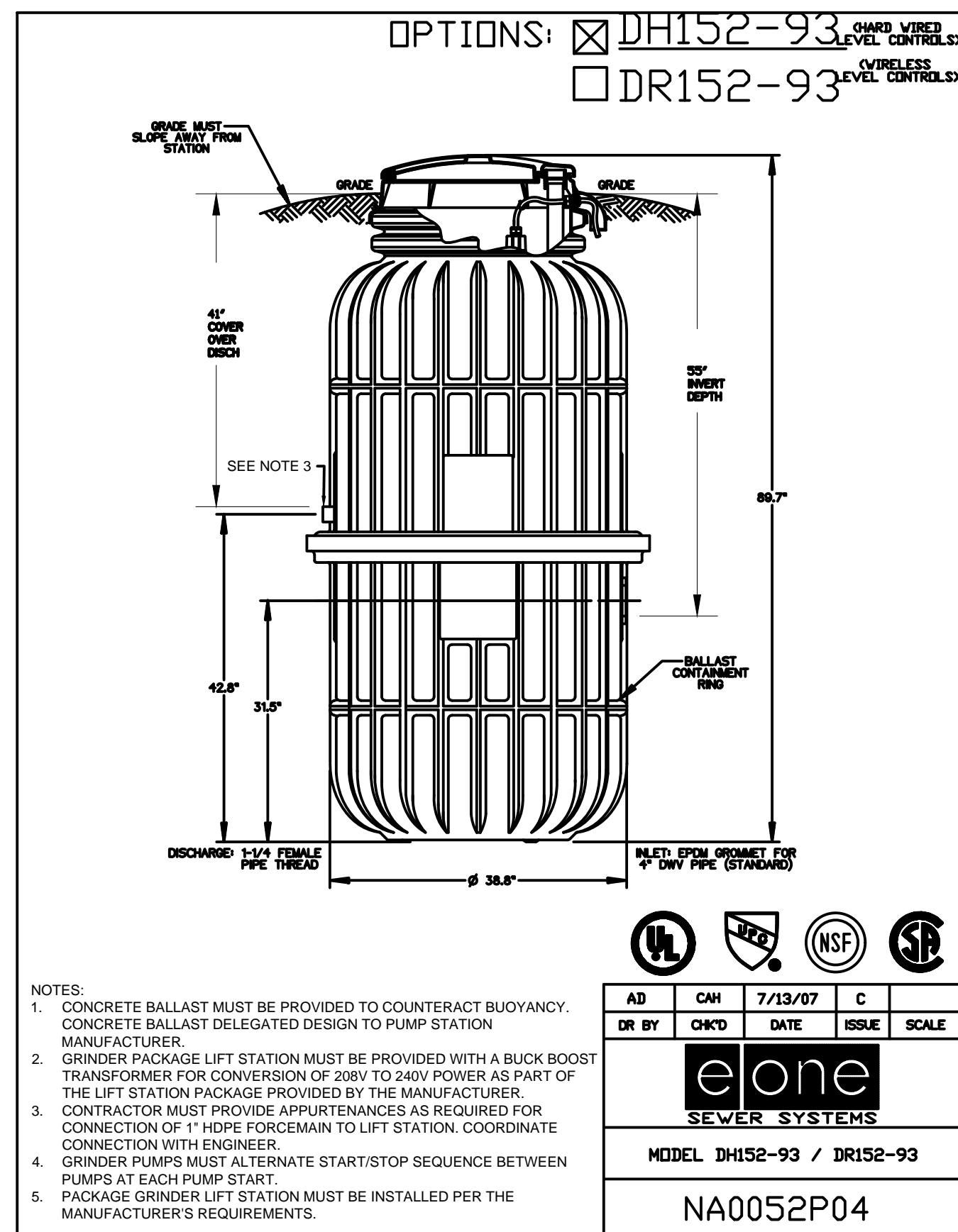


METHOD 1

SIDE VIEW

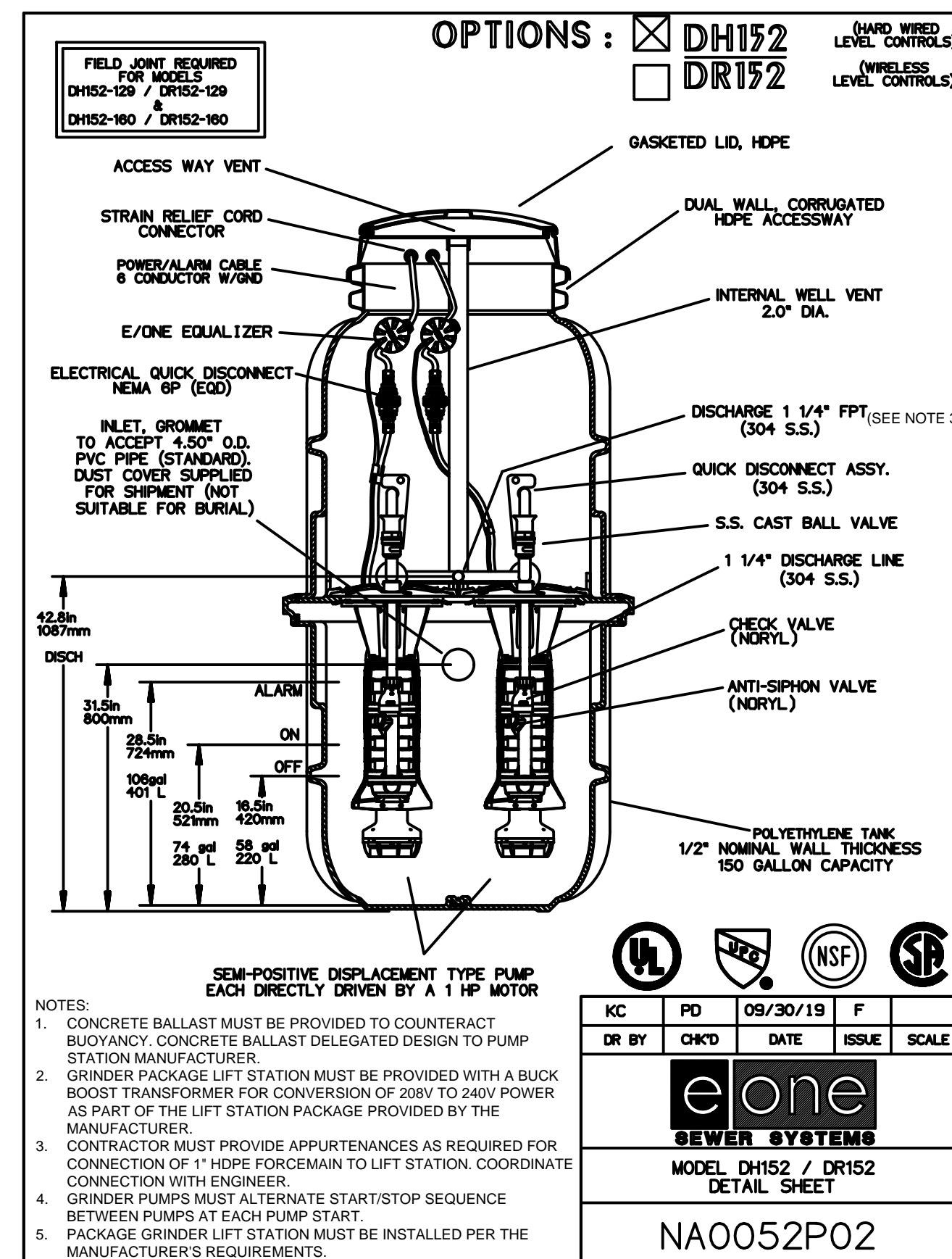
ELEVATION VIEW

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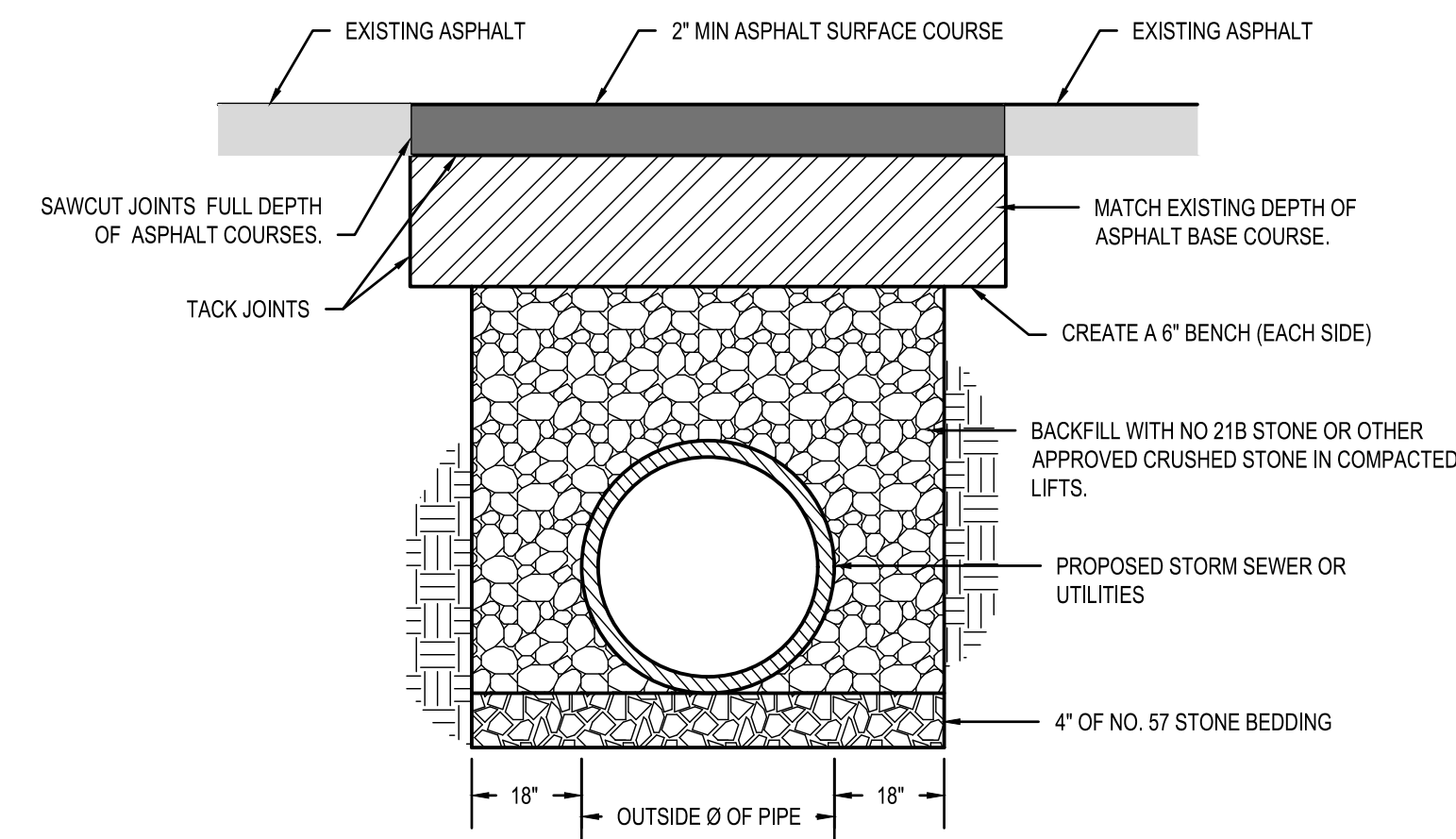
- NOTES:
- CONCRETE BALLAST MUST BE PROVIDED TO COUNTERACT BUOYANCY. CONCRETE BALLAST DELEGATED DESIGN TO PUMP STATION MANUFACTURER.
 - GRINDER PACKAGE LIFT STATION MUST BE PROVIDED WITH A BUCK BOOST TRANSFORMER FOR CONVERSION OF 208V TO 240V POWER AS PART OF THE LIFT STATION PACKAGE PROVIDED BY THE MANUFACTURER.
 - CONTRACTOR MUST PROVIDE APPURTENANCES AS REQUIRED FOR CONNECTION OF 1" HDPE FORCEMAIN TO LIFT STATION. COORDINATE CONNECTION WITH ENGINEER.
 - GRINDER PUMPS MUST ALTERNATE START/STOP SEQUENCE BETWEEN PUMPS AT EACH PUMP START.
 - PACKAGE GRINDER LIFT STATION MUST BE INSTALLED PER THE MANUFACTURER'S REQUIREMENTS.

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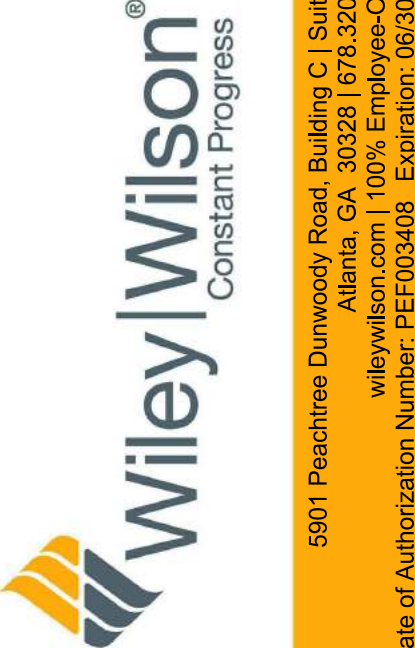
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3 PAVEMENT PATCH FOR UTILITY TRENCH
NOT TO SCALE

2 GRINDER LIFT STATION
NOT TO SCALE



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A. GENERAL

- 1. STRUCTURAL GENERAL NOTES ARE INTENDED TO HIGHLIGHT OR IN SOME CASES SUPPLEMENT PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR COMPLETE WORK COVERAGE.
2. THE DESIGN OF THE STRUCTURE COMPLIES WITH THE INTERNATIONAL BUILDING CODE 2021 AS ADOPTED BY THE 2021 ALABAMA BUILDING CODE.
3. GRAVITY DESIGN LOADS ARE AS FOLLOWS:

Table with columns for load types (SUPERIMPOSED DEAD LOADS, LIVE LOADS) and their corresponding values (ACTUAL, 5 PSF, 1 PSF, AS REQ'D, 150 PSF, 125 PSF, etc.).

- 4. HANDRAILS AND GUARDS RAILS MUST BE DESIGNED FOR A LINEAR LOAD OF 50 POUNDS PER FOOT OR CONCENTRATED LOAD OF 200 POUNDS...
5. IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE SECTION 1607.4, THE FLOOR SYSTEM IS DESIGNED TO WITHSTAND CONCENTRATED LOADS ASSUMED ACTING UNIFORMLY ON A 2-1/2 FEET SQUARE AREA AND LOCATED TO PRODUCE THE MAXIMUM EFFECT IN STRUCTURAL MEMBERS.
6. THE DESIGN OF THE STRUCTURE TO RESIST SNOW LOADS IS IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE'S SECTION 1608 AND ASCE 7, CHAPTER 7

Table with columns for snow load factors (GROUND SNOW LOAD, FLAT ROOF SNOW LOAD, EXPOSURE FACTOR, IMPORTANCE FACTOR, etc.) and their values (Ps = 0.0 PSF, Pt = 0.0 PSF, etc.).

- 7. THE DESIGN OF THE STRUCTURE TO RESIST WIND PRESSURES IS IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE'S SECTION 1609.0, WHICH IS BASED ON ASCE 7, CHAPTER 26. DESIGN INFORMATION IS AS FOLLOWS:

Table with columns for wind design parameters (DESIGN WIND SPEED, RISK CATEGORY, WIND EXPOSURE CATEGORY, etc.) and their values (V = 160 MPH, VASD = 124 MPH, etc.).

- 8. THE DESIGN OF THE STRUCTURE TO RESIST RAIN LOADS IS IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE'S SECTION 1611, AND ASCE 7 CHAPTER 8. THE 60-MINUTE RAIN INTENSITY, I, IS 4.94 IN/HR.

- 9. THE DESIGN OF THE STRUCTURE TO RESIST SEISMIC FORCES IS IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE'S SECTION 1613, AND ASCE 7, CHAPTERS 11 TO 18:

Table with columns for seismic design parameters (RISK CATEGORY, IMPORTANCE FACTOR, MAPPED SPECTRAL RESPONSE ACCELERATIONS, etc.) and their values (II, Ia = 1.0, Ss = 0.085g, etc.).

- 10. IMPOSED CONSTRUCTION LOADS IN EXCESS OF STATED DESIGN LOADS MUST BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO THE IMPOSITION OF SUCH LOADS.

- 11. THE GENERAL CONTRACTOR AND SUB-CONTRACTORS MUST DETERMINE THE SCOPE OF THE STRUCTURAL WORK FROM THE CONTRACT DOCUMENTS TAKEN AS A WHOLE. THE STRUCTURAL DRAWINGS MUST NOT BE CONSIDERED SEPARATELY FOR PURPOSES OF BIDDING THE STRUCTURAL WORK...

- 12. THE REPRODUCTION OF THE STRUCTURAL CONTRACT DOCUMENTS IN ANY FASHION AS STRUCTURAL SHOP DRAWING DOCUMENTS IS PROHIBITED.

- 13. SCALES NOTED IN THE DRAWINGS ARE FOR GENERAL INFORMATION ONLY. NO DIMENSIONAL INFORMATION MUST BE OBTAINED BY DIRECT SCALING OF THE DRAWINGS.

- 14. DETAILS, SECTIONS AND NOTES SHOWN ON THESE DRAWINGS ARE INTENDED TO BE TYPICAL AND MUST APPLY TO SIMILAR CONDITIONS ELSEWHERE UNLESS OTHERWISE SHOWN OR NOTED.

- 15. STRUCTURAL MEMBERS HAVE BEEN LOCATED AND DESIGNED TO ACCOMMODATE THE MECHANICAL EQUIPMENT AND OPENINGS SPECIFIED BY THE MECHANICAL ENGINEER. ANY SUBSTITUTIONS RESULTING IN REVISIONS TO THE STRUCTURE MUST BE THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE WITH THE STRUCTURAL ENGINEER.

- 16. COORDINATE STRUCTURAL SUPPORT LOCATIONS WITH EQUIPMENT SERVICE ACCESS CLEARANCES PER EACH EQUIPMENT MANUFACTURER AND PER BUILDING CODE.

- 17. PRINCIPAL OPENINGS IN THE STRUCTURE ARE INDICATED ON THE CONTRACT DOCUMENTS. REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS, ETC. NOT HERIN INDICATED. OPENINGS IN SLABS WITH A MAXIMUM SIDE DIMENSION OR DIAMETER OF 12 INCHES OR LESS MUST NOT REQUIRE ADDITIONAL FRAMING OR REINFORCEMENT, UNLESS NOTED OTHERWISE. LOCATION OF SLEEVES OR OPENINGS IN STRUCTURAL MEMBERS MUST BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL.

- 18. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ALL RESULTING REVISIONS TO THE STRUCTURAL SYSTEM AS A RESULT OF ACCEPTANCE OF CONTRACTOR PROPOSED ALTERNATIVES OR SUBSTITUTIONS.

- 19. THE GENERAL CONTRACTOR (OR CONSTRUCTION MANAGER) MUST SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS FOR APPROVAL. THE STRUCTURAL ENGINEER WILL NOT BE RESPONSIBLE FOR THE STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT IF THE GENERAL CONTRACTOR FAILS TO OBTAIN APPROVAL OF THE SHOP DRAWINGS. SHOP DRAWINGS ARE REVIEWED AS A CONVENIENCE TO THE GENERAL CONTRACTOR AND ARE NOT A CONTRACT DOCUMENT. THE GENERAL CONTRACTOR MUST STATE ON THE SHOP DRAWINGS THAT CONTRACT DOCUMENT REQUIREMENTS HAVE BEEN MET AND THAT ALL DIMENSIONS, CONDITIONS, AND QUANTITIES HAVE BEEN REVIEWED AND VERIFIED AS SHOWN AND/OR CORRECTED ON THE SHOP DRAWINGS.

- 20. THE CONTRACTOR MUST BE RESPONSIBLE FOR PROVIDING TEMPORARY BRACING AND SHORING, AS REQUIRED, TO ENSURE VERTICAL AND LATERAL STABILITY OF THE ENTIRE STRUCTURE OR PORTION THEREOF DURING CONSTRUCTION.

- 21. TEMPORARY BRACING MUST BE PROVIDED FOR ALL WALLS SUBJECT TO UNBALANCED BACKFILL. BRACE WALL PLUMB UNTIL STABILIZING ELEMENT ABOVE IS IN PLACE.

- 22. ALL WALLS ARE DESIGNED AS Laterally Braced by the Floor Systems. CONTRACTOR MUST ENSURE THAT WALLS ARE ADEQUATELY BRACED DURING CONSTRUCTION.

- 23. ALL COLUMNS AND FOOTINGS MUST BE CENTERED ON GRIDLINES IN EACH DIRECTION, UNLESS NOTED OTHERWISE.

- 24. THE STRUCTURE HAS BEEN DESIGNED FOR THE IN-SERVICE LOADS ONLY. THE METHODS, PROCEDURES, AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. SUPPORTING FORMWORK FOR THE CONCRETE CONSTRUCTION MUST NOT BE REMOVED BEFORE THE CONCRETE HAS GAINED SUFFICIENT STRENGTH TO SAFELY SUPPORT THE DEAD AND SUPERIMPOSED LOADS WHICH WOULD BE SUBSEQUENTLY APPLIED. THE CONTRACTOR MUST TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.

- 25. METHODS, PROCEDURES, AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR MUST TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.

- 26. THE CONTRACTOR MUST FABRICATE AND ERECT STEEL IN ACCORDANCE WITH OSHA'S SAFETY REQUIREMENTS, 29 CFR 1926 SAFETY STANDARDS FOR STEEL ERECTION; FINAL RULE.

- 27. A REGISTERED ENGINEER MUST DESIGN ALL NON-PRIMARY STRUCTURAL ELEMENTS SUCH AS, BUT NOT LIMITED TO THE FOLLOWING: MULLIONS, RAILINGS AND COLD-FORMED STEEL FRAMING TO MEET THE REQUIREMENTS OF THE LOCAL GOVERNING JURISDICTION. SUBMIT CALCULATIONS AND SHOP DRAWINGS WITH THE RESPONSIBLE ENGINEER'S SEAL AND SIGNATURE FOR THE STATE WHERE THE CONSTRUCTION TAKES PLACE.
28. DELEGATED DESIGN ITEMS IDENTIFIED IN THE CONSTRUCTION DRAWINGS MUST BE ENGINEERED BY A REGISTERED ENGINEER TO MEET THE REQUIREMENTS OF THE LOCAL GOVERNING JURISDICTION. SUBMIT CALCULATIONS AND SHOP DRAWINGS WITH THE RESPONSIBLE ENGINEER'S SEAL AND SIGNATURE FOR THE STATE WHERE THE CONSTRUCTION TAKES PLACE.

B. FOUNDATIONS AND EARTHWORK

- 1. GEOTECHNICAL INFORMATION IS CONTAINED IN A REPORT PREPARED BY ECS SOUTHEAST, LLC, REPORT NUMBER 30:2598, DATED FEBRUARY 12, 2024. BORING LOGS AND LABORATORY TEST RESULTS ARE INCLUDED FOR REFERENCE IN THE PROJECT MANUAL.
2. THE CONTRACTOR MUST PROCURE AND MAINTAIN AN ECS GEOTECHNICAL REPRESENTATIVE ON-SITE FULL TIME DURING EARTHWORK AND FOUNDATION CONSTRUCTION ACTIVITIES. THE GEOTECHNICAL REPRESENTATIVE MUST VERIFY COMPLIANCE WITH THE PROJECT'S GEOTECHNICAL REPORT, CONFIRM ALLOWABLE BEARING PRESSURE, AND PROVIDE REMEDIAL ACTIONS, AS REQUIRED.
3. THE CONTRACTOR MUST COMPLY WITH THE RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL REPORT AND THE PROJECT SPECIFICATIONS WHEN PERFORMING EXCAVATIONS, FOOTING CONSTRUCTION, AND PREPARING THE SUBGRADE UNDER THE SLAB ON GRADE.
4. LOW CONSISTENCY AND MOISTURE SENSITIVE SOILS ARE EXPECTED TO BE ENCOUNTERED ON SITE. AREAS OF THE SITE WITH THESE SOILS MUST BE DENSIFIED PRIOR TO PLACEMENT OF STRUCTURAL FILL. BEFORE STRUCTURAL FILL IS PLACED, DENSIFIED AREAS MUST BE PROOFROLLED UNDER THE OBSERVATION OF AN ECS ENGINEER. IF DENSIFIED SOILS EXHIBIT EXCESSIVE MOVEMENT OR DEFLECTION DURING PROOFROLLING THAT THE SOIL MUST BE UNDERCUT AND REPLACED WITH STRUCTURAL FILL IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
5. CONTRACTOR MUST PROVIDE SITE DRAINAGE AND DEWATERING PLAN AT THE ONSET AND DURING CONSTRUCTION IN ACCORDANCE WITH THE GEOTECHNICAL REPORT AND PROJECT SPECIFICATIONS. CONSTRUCTION TRAFFIC MUST BE MINIMIZED ACROSS THE SITE DURING WET PERIODS TO MITIGATE STRENGTH LOSS OF SITE SOILS.
6. THE FOUNDATION DESIGN FOR THE STRUCTURE IS FOR AN ALLOWABLE SOIL BEARING CAPACITY OF 1,500 PSF. FOUNDATIONS EXPOSED TO FROST MUST BEAR A MINIMUM OF 24 INCHES BELOW FINISHED GRADE.
7. DO NOT OVER EXCAVATE THE FOOTPRINT OF SPREAD FOOTINGS, STRIP FOOTINGS, OR OTHER SOIL FORMED FOUNDATION ELEMENTS. IF OVER EXCAVATION IS REQUIRED BEYOND THE 4 INCH CONSTRUCTION TOLERANCE, FORM THE SIDES OF THE FOOTING.
8. CLEAN AND TAMP SHALLOW FOUNDATION EXCAVATIONS TO A UNIFORM SURFACE. PLACE FOUNDATION CONCRETE THE SAME DAY THAT PROPER EXCAVATION AND BEARING PRESSURE CAPACITY IS ACHIEVED AND APPROVED BY THE GEOTECHNICAL REPRESENTATIVE. IF THE FOUNDATION EXCAVATION MUST REMAIN OPEN OVERNIGHT, OR IS SUBJECT TO RAIN, PROVIDE A 3" THICK MUD MAT OF LEAN CONCRETE OVERTOP THE EXPOSED BEARING SOILS. REMOVE ANY WATER FROM FOUNDATION EXCAVATIONS PRIOR TO PLACEMENT OF FOUNDATION CONCRETE.
9. PLACE CONCRETE FOR WALL FOOTINGS MONOLITHICALLY WITH COLUMN FOOTINGS. CONSTRUCTION JOINTS IN WALL FOOTINGS MUST BE MADE ONLY MIDWAY BETWEEN COLUMN FOOTINGS UNLESS OTHERWISE NOTED OR DIRECTED BY GEOTECHNICAL REPRESENTATIVE. PROVIDE A CONSTRUCTION JOINT, IN ACCORDANCE WITH DETAIL ON S-501, WHERE CONTINUOUS FOOTINGS TRANSITION FROM EXISTING SOIL TO NEWLY PLACED AND COMPACTED FILL. FINAL LOCATIONS OF ALL CONSTRUCTION JOINTS MUST BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO PLACEMENT.
10. ALWAYS PROVIDE POSITIVE SURFACE WATER DRAINAGE AWAY FROM THE STRUCTURE.

C. SOIL SUPPORTED SLABS

- 1. FILL THAT SUPPORTS SLABS MUST HAVE A MAXIMUM PLASTICITY INDEX OF <10. PREPARE, PLACE AND COMPACT AS RECOMMENDED IN THE GEOTECHNICAL REPORT.
2. FOR SOIL SUPPORTED SLABS, PROPERLY COMPACT AND PROOFROLL SUBGRADE AS RECOMMENDED IN THE GEOTECHNICAL REPORT. MAINTAIN THE SUBGRADE FREE OF STANDING WATER, MUD AND FROZEN SOIL. PROVIDE A MINIMUM OF 6 INCHES OF GRANULAR PER THE GEOTECHNICAL REPORT. COVER GRANULAR MATERIAL WITH A VAPOR RETARDER BEFORE PLACING THE SLAB.
3. FOR SLAB ON GRADE REINFORCEMENT, PROVIDE WELDED WIRE FABRIC IN FLAT SHEETS.
4. SUPPORT SLAB ON GRADE REINFORCEMENT WITH STANDARD CORROSION RESISTANT HIGH CHAIRS WITH SAND BEARING PLATES.
5. PROVIDE SLAB CONTROL JOINTS AT EACH COLUMN LINE IN EACH DIRECTION FOR SLABS ON GRADE. SPACE ADDITIONAL JOINTS AT A MAXIMUM OF 30 TIMES THE SLAB THICKNESS OR 15 FEET, WHICHEVER IS LESS. PLACE JOINTS SO THAT PANEL LENGTH TO WIDTH RATIOS ARE LESS THAN 1.5.
6. FREE WATER ON THE SLAB SURFACE DURING FINISHING OPERATIONS IS PROHIBITED. SOFT CUT CONTROL JOINTS AS SOON AS POSSIBLE - GENERALLY WITHIN 6 HOURS AFTER FINISHING.
7. WHERE SLAB CONTROL JOINTS ARE SHOWN ON THE DRAWINGS, CONSTRUCTION JOINTS MAY BE SUBSTITUTED TO ACCOMMODATE THE CONTRACTOR'S PLACEMENT STRATEGY, UNLESS OTHERWISE NOTED ON THE DRAWINGS, PROVIDE 3/4" DIAMETER BY 1'-4" SMOOTH DOWELS AT 12 INCHES ON CENTER AT SLAB ON GRADE CONSTRUCTION JOINTS. GREASE ONE END OF DOWELS.
8. WHERE SHEAR TRANSFER IS REQUIRED ACROSS SLAB CONTROL JOINTS, PROVIDE DOWEL BASKET ASSEMBLIES. AREA OF STEEL MUST MEET THE REQUIREMENTS OF ACI 360 TABLE 6.1 FOR ROUND DOWELS OR SQUARE BARS OR AS REQUIRED BY MANUFACTURER FOR PLATE DOWELS.

D. CONCRETE

- 1. ALL CONCRETE WORK MUST BE IN ACCORDANCE WITH ACI 301, ACI 318 AND ACI 302.
2. PROVIDE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH (fc) AT 28 DAYS, A MAXIMUM WATER/CEMENT RATIO, AND DURABILITY EXPOSURE CATEGORIES (FREEZE/THAW, SULFATE, WATER CONTACT, AND CORROSION PROTECTION, RESPECTIVELY) AS FOLLOWS:

Table with columns for ELEMENT, COMPRESSIVE STRENGTH, WATER/CEMENT RATIO, and DURABILITY. Rows include FOOTINGS, SLABS ON GRADE, PIERS & STEM WALLS, and EXTERIOR CONCRETE.

USE NORMAL WEIGHT AGGREGATES CONFORMING TO ASTM C33 AND TYPE III PORTLAND CEMENT CONFORMING TO ASTM C150.

- 3. FLY ASH CONFORMING TO ASTM C618, TYPE C OR F MAY BE USED AS TO REPLACE A PORTION OF THE PORTLAND CEMENT IN A CONCRETE MIX. THE AMOUNT OF PORTLAND CEMENT CONTENT MUST NOT BE LESS THAN 70 PERCENT OF THE TOTAL AMOUNT OF CEMENTITIOUS MATERIAL IN THE MIX.
4. GROUND GRANULATED BLAST-FURNACE SLAG CONFORMING TO ASTM C989, MAY BE USED AS TO REPLACE A PORTION OF THE PORTLAND CEMENT IN A CONCRETE MIX. THE AMOUNT OF PORTLAND CEMENT CONTENT MUST NOT BE LESS THAN 70 PERCENT OF THE TOTAL AMOUNT OF CEMENTITIOUS MATERIAL IN THE MIX.
5. CONCRETE REINFORCEMENT BARS MUST CONFORM TO ASTM A615, GRADE 60. REINFORCEMENT BARS MUST NOT BE TACK WELDED, WELDED, HEATED OR CUT UNLESS INDICATED ON THE CONTRACT DOCUMENTS OR APPROVED BY THE STRUCTURAL ENGINEER.
6. PROVIDE WELDED WIRE FABRIC CONFORMING TO ASTM A1064 IN FLAT SHEETS. LAP FABRIC TWO MESHES AT SPLICES.
7. GROUT UNDER ALL COLUMN BASE PLATES AND BEAM BEARING PLATES WITH NON-SHRINK, NON-METALLIC GROUT WHICH CONFORMS TO CORPS OF ENGINEERS SPECIFICATION CRD-C 621-82 OR ASTM C1107.
8. DETAILING OF CONCRETE REINFORCEMENT BARS AND ACCESSORIES MUST CONFORM TO THE RECOMMENDATIONS OF ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT" AND ACI SP-66 "DETAILING MANUAL". PLACING OF REINFORCING BARS MUST CONFORM TO THE RECOMMENDATIONS OF ACI 315R "MANUAL OF ENGINEERING AND PLACING DRAWINGS FOR REINFORCED CONCRETE STRUCTURES" AND CRSI "MANUAL OF STANDARD PRACTICE".
9. MIX, TRANSPORT, AND PLACE CONCRETE PER THE RECOMMENDATIONS OF ACI 301.
10. PROVIDE CONCRETE COVER PROTECTION OF REINFORCEMENT PER ACI 318 SECTION 20.6.1.3 WITH STANDARD BAR CHAIRS AND SPACERS REQUIRED TO MAINTAIN MINIMUM CONCRETE PROTECTION. TYPICAL COMMON MINIMUM CONCRETE COVER APPLYING TO THIS PROJECT INCLUDES, BUT IS NOT LIMITED TO THE FOLLOWING:

Table with columns for cover requirements (CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH, #6 BARS AND LARGER, #5 BARS AND SMALLER, etc.) and their values (3 INCHES, 2 INCHES, 1 1/2 INCHES, etc.).

- 11. WELDING OF REINFORCEMENT BARS IS NOT PERMITTED.
12. REINFORCEMENT DESIGNATED AS "CONTINUOUS" MUST LAP 48 BAR DIAMETERS AT SPLICES UNLESS NOTED OTHERWISE. REINFORCEMENT BAR SPLICES IN GRADE BEAMS MUST BE LOCATED AT THE CENTERLINE OF SUPPORTS FOR BOTTOM BARS AND AT MIDSPAN FOR TOP BARS. PROVIDE STANDARD ACI HOOKS FOR TOP AND BOTTOM BARS AT DISCONTINUOUS ENDS OF ALL GRADE BEAMS.
13. HORIZONTAL FOOTING AND HORIZONTAL WALL REINFORCEMENT MUST BE CONTINUOUS AND MUST HAVE 90-DEGREE BENDS AND EXTENSIONS, OR CORNER BARS OF EQUIVALENT SIZE LAPPED 48 BAR DIAMETERS, AT CORNERS AND INTERSECTIONS.
14. PROVIDE CORNER BARS AT ALL WALL INTERSECTIONS WITH SIZE AND SPACING TO MATCH HORIZONTAL WALL REINFORCEMENT.
15. TIE DOWELS IN PLACE BEFORE PLACING CONCRETE. DO NOT STAB OR "WET-SET" DOWELS.
16. HORIZONTAL JOINTS WILL NOT BE PERMITTED IN CONCRETE CONSTRUCTION EXCEPT AS SHOWN ON THE CONTRACT DOCUMENTS. VERTICAL JOINTS MUST OCCUR AT CENTER OF SPANS AT LOCATIONS APPROVED BY THE STRUCTURAL ENGINEER.
17. PROVIDE HORIZONTAL CONSTRUCTION JOINTS ONLY WHERE SHOWN IN THE CONTRACT DOCUMENTS. AT HORIZONTAL JOINTS, ROUGHEN THE LOWER CONTACT SURFACE WITH ABOUT A 1/4 INCH AMPLITUDE. REMOVE ANY LAITANCE FROM THE HARDENED CONTACT SURFACE AND MAINTAIN A CLEAN CONTACT SURFACE FOR THE ADJOINING POUR.
18. CHAMFER EXPOSED EDGES OF CONCRETE 3/4" UNLESS OTHERWISE NOTED.

D. CONCRETE CONTINUED

- 19. SHORING UNDER FORMS MUST NOT BE REMOVED UNTIL THE CONCRETE IT SUPPORTS IS CAPABLE OF SUPPORTING ITSELF AND ALL SUPERIMPOSED LOADS.
20. PROTECT AND CURE ALL CONCRETE SURFACES. BEGIN CURING WALLS IMMEDIATELY AFTER STRIPPING FORMS AND FLATWORK IMMEDIATELY AFTER FINISHING.
21. INSTALL AND SECURE EMBEDMENTS SUCH AS ANCHOR BOLTS AND EMBEDMENT PLATES WITHIN SPECIFIED TOLERANCES BEFORE CONCRETE PLACEMENT.
22. DO NOT SLEEVE BEAMS OR COLUMNS WHERE NOT SHOWN ON THE STRUCTURAL DRAWINGS WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
23. DO NOT PLACE CONDUIT OR PIPES IN ANY CONCRETE ELEMENTS INCLUDING SLABS, BEAMS, WALLS OR COLUMNS UNLESS INDICATED IN THE STRUCTURAL DOCUMENTS.

E. STRUCTURAL COLD-FORMED STEEL

- 1. ALL GALVANIZED STUDS AND JOISTS 16 GAGE AND HEAVIER MUST BE FORMED FROM STEEL THAT CONFORMS TO THE REQUIREMENTS OF ASTM A1003, GRADE ST50H, WITH A MINIMUM YIELD STRENGTH OF 50,000 PSI. ALL 18 GAGE STUDS MUST CONFORM TO ASTM A1003 GRADE ST33H, WITH A MINIMUM YIELD STRENGTH OF 33,000 PSI.
2. ALL GALVANIZED TRACK, BRIDGING, END CLOSURES AND ACCESSORIES MUST BE FORMED FROM STEEL THAT CONFORMS TO THE REQUIREMENTS OF ASTM A1003, GRADE ST33H, WITH A MINIMUM YIELD STRENGTH OF 33,000 PSI.
3. ALL GALVANIZED STUDS AND ACCESSORIES MUST BE FORMED FROM STEEL HAVING A GALVANIZED COATING WHICH CONFORMS TO ASTM A525 G-60.
4. ALL PAINTED STUDS AND JOISTS 16 GAGE AND HEAVIER MUST BE FORMED FROM STEEL THAT CONFORMS TO ASTM A570 GRADE 50, MINIMUM YIELD STRENGTH 50,000 PSI. ALL PAINTED STUDS AND JOISTS 18 GAGE AND LIGHTER MUST BE FORMED FROM STEEL THAT CONFORMS TO ASTM A611 GRADE C, MINIMUM YIELD STRENGTH 33,000 PSI.
5. ALL PAINTED TRACK, BRIDGING, END CLOSURES AND ACCESSORIES MUST BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF ASTM A611, GRADE C WITH A MINIMUM YIELD STRENGTH OF 33,000 PSI.
6. WELDING MUST CONFORM TO AWS D1.1, AWS D1.3 AND AISI MANUAL SECTION 4.2.
7. WELDING MUST BE BY FUSION WELDING UTILIZING ELECTRODES WITH ASTM CLASSIFICATION E60. ALL WELDERS MUST BE QUALIFIED BY TESTING IN ACCORDANCE WITH THE "CODE FOR WELDING IN BUILDING CONSTRUCTION", AWS D1.1 OF THE AMERICAN WELDING SOCIETY.
8. BOLTING AND SELF DRILLING/SELF TAPPING SCREWS MAY BE EMPLOYED. THE CONTRACTOR MUST SUBMIT MECHANICAL FASTENER DATA FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER.
9. STUD MEMBERS MAY BE PUNCHED. JOIST AND HEADER MEMBERS MUST NOT BE PUNCHED.
10. ALL PAINTED MATERIAL AND ACCESSORIES MUST BE PRIMED WITH RUST INHIBITIVE PAINT MEETING THE PERFORMANCE REQUIREMENTS OF TT-P-636C.
11. METAL STUDS MUST BE EITHER PAINTED OR GALVANIZED AS INDICATED ABOVE WITH THE EXCEPTION OF STUDS, HEADERS AND TRACK LOCATED AT EXTERIOR WALLS WHICH MUST BE GALVANIZED.
12. MILL CERTIFICATION OF THE STEEL PROPERTIES IS REQUIRED.
13. PRIOR TO PREFABRICATION OF FRAMING, THE CONTRACTOR MUST SUBMIT FABRICATION AND ERECTION DRAWINGS TO THE ARCHITECT AND/OR STRUCTURAL ENGINEER FOR REVIEW.
14. ALL FRAMING COMPONENTS MUST BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS, OR AS REQUIRED FOR AN ANGULAR FIT AGAINST ABUTTING MEMBER. MEMBERS MUST BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED.
15. AXIALLY LOADED STUDS MUST BE INSTALLED IN A MANNER WHICH WILL ASSURE THAT ENDS OF THE STUDS ARE POSITIONED AGAINST THE INSIDE TRACK WEB, PRIOR TO STUD AND TRACK ATTACHMENT.
16. WIRE TYING IS NOT PERMITTED.
17. ALL LOAD BEARING PANELS AND HEADER MEMBERS WITH STIFFENERS MUST BE PRE-LOADED WITH CLAMPING DEVICE TO INSURE FULL BEARING BETWEEN STUDS AND TRACKS. HEADERS MUST BE REVIEWED BY THE STRUCTURAL ENGINEER OR AUTHORIZED REPRESENTATIVE PRIOR TO INSTALLATION.
18. METAL STUD ERECTOR IS RESPONSIBLE FOR INSTALLING WIND BRACING WHERE INDICATED ON THE PLANS. FLOOR PLATES WILL BE INSTALLED BY GENERAL CONTRACTOR.
19. COLD FORMED HEADERS MUST HAVE STIFFENERS COMPRISED OF 14 GAGE TRACK PLACED VERTICALLY BETWEEN THE FLANGES. CONNECTION BETWEEN THE 2 MEMBERS COMPRISING THE LINTEL, END SPACERS AND STIFFENERS MUST BE WELDED OR SCREWED AT FIFTH POINTS.
20. STUDS MUST BE ATTACHED TO TRACK ON TWO SIDES.
23. METAL STUD ERECTOR MUST BE RESPONSIBLE FOR LOCATION AND ATTACHING CONNECTION PLATES TO HOT ROLLED STEEL SECTIONS.
24. HEADER WITH INTERIOR STIFFENERS MUST HAVE INDIVIDUAL STIFFENERS INSPECTED PRIOR TO ASSEMBLING THE TWO JOIST MEMBERS.
25. AT TRACK BUTT JOINTS, ABUTTING PIECES OF TRACK MUST BE SECURELY ANCHORED TO A COMMON STRUCTURAL ELEMENT, OR THEY MUST BE BUTT-WELDED OR SPLICED TOGETHER.
26. STUDS MUST BE PLUMBED, ALIGNED AND SECURELY ATTACHED TO THE FLANGE OR WEBS OF BOTH UPPER AND LOWER TRACKS.
27. WALL STUD BRIDGING MUST BE ATTACHED IN A MANNER TO PREVENT STUD ROTATION. BRIDGING ROWS MUST BE SPACED AT THIRD POINTS AND BRIDGING MUST BE 16 GAGE, 1-1/2" WIDE WITH 9/16" FLANGES.
28. SPLICES IN AXIALLY LOADED STUDS ARE NOT PERMITTED.
29. WINDOW SILLS MUST BE THE SAME SIZE AND GAGE AS THE OTHER MEMBERS DESIGNATED IN THE WALL PANEL UNLESS OTHERWISE NOTED.
30. CFS TRUSSES MUST BE LOCATED DIRECTLY OVER BEARING STUDS.

F. CONCRETE MASONRY

- 1. MASONRY CONSTRUCTION MUST BE PERFORMED IN ACCORDANCE WITH TMS 402 "BUILDING CODE FOR MASONRY STRUCTURES" AND TMS 602, "SPECIFICATION FOR MASONRY STRUCTURES."
2. CONCRETE MASONRY UNITS MUST CONFORM TO ASTM C90, GRADE N-1 UNLESS OTHERWISE NOTED. COMPRESSIVE STRENGTH ON NET CROSS SECTIONAL AREA OF INDIVIDUAL MASONRY UNITS MUST BE 2,000 PSI. NET AREA COMPRESSIVE STRENGTH OF MASONRY (fm) MUST BE 1,500 PSI.
3. MASONRY MUST BE LAID IN ASTM C270, TYPE "S" MORTAR, UNLESS NOTED OTHERWISE AND MUST HAVE FULL MORTAR COVERAGE OF THE FACE SHELLS IN BOTH HORIZONTAL AND VERTICAL JOINTS.
4. GROUT FOR REINFORCED MASONRY MUST CONFORM TO ASTM C476.
5. GROUT FOR REINFORCED MASONRY MUST HAVE A COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS AND MUST CONFORM TO THE FOLLOWING:
a. GROUT IN SPACES 3" X 4" AND GREATER MUST HAVE A SLUMP OF 9" TO 11" WITH 3/8" MAXIMUM AGGREGATE SIZE.
b. GROUT IN SPACES 2" X 4" AND SMALLER MUST HAVE A SLUMP OF 5" AND CONFORM TO ASTM C476.
6. GALVANIZED HORIZONTAL JOINT REINFORCEMENT MUST BE PLACED IMMEDIATELY ABOVE AND BELOW ALL OPENINGS AND AT 16 INCHES ON CENTER VERTICALLY. REINFORCEMENT MUST BE LADDER, TRUSS TYPE, AND WHERE SPLICED, MUST LAP A MINIMUM OF 6 INCHES. REINFORCEMENT MUST CONFORM TO ASTM A82.
7. EXCEPT FOR WALL PILLASTERS, VERTICAL MASONRY REINFORCEMENT MUST BE FIELD CUT FOR 4'-0" LIFTS AND LAPPED SPLICED A MINIMUM OF 48 BAR DIAMETERS. MASONRY CORES CONTAINING VERTICAL REINFORCEMENT MUST BE GROUTED SOLID.
8. CELLS TO BE GROUTED MUST BE CLEAN AND FREE OF EXCESS MORTAR AND FOREIGN MATERIALS.
9. GROUT ALL CELLS THAT INCLUDE REINFORCEMENT, ANCHORS OR STRUCTURAL EMBEDMENTS. PLACE GROUT IN 4'-0" LIFTS. CONSOLIDATE ALL GROUT PLACEMENTS BY MECHANICAL VIBRATION. PROVIDE CLEANOUTS FOR TOTAL GROUT PLACEMENT HEIGHT OVER 5'-0".
10. PROVIDE MASONRY ACCESSORIES TO SECURE VERTICAL REINFORCEMENT IN PLACE AND CORRECTLY POSITIONED. VERTICAL REINFORCEMENT IS TO BE CENTERED IN THE MASONRY CELLS UNLESS INDICATED OTHERWISE.
11. REINFORCEMENT MUST CONFORM TO THE STANDARDS SPECIFIED IN THE CONCRETE NOTES. REINFORCEMENT MUST BE LAP SPLICED A MINIMUM OF 48 BAR DIAMETERS UNLESS NOTED OTHERWISE.
12. THE MASONRY CONTRACTOR MUST PROVIDE AND PLACE SUCH SPECIAL UNITS AS MAY BE REQUIRED TO FORM ALL CORNERS, RETURNS, AND OFFSETS WHILE MAINTAINING THE PROPER BOND.
13. WHERE INTERIOR CONCRETE MASONRY PARTITIONS INTERSECT WITH OTHER INTERIOR PARTITIONS OR EXTERIOR WALLS, A MASONRY BOND, OR THE EQUIVALENT IN APPROVED METAL TIES, MUST BE PROVIDED UNLESS NOTED OTHERWISE ON THE DRAWINGS.
14. REINFORCE ALL JAMB CELLS, CORNER CELLS, TEE CELLS, END CELLS AND AT EACH SIDE OF CONTROL JOINTS FULL HEIGHT - MATCH TYPICAL WALL REINFORCING UNLESS OTHERWISE NOTED.
15. FACE SHELL BEDDING MUST BE USED WITH COMPLETE COVERAGE OF FACE SHELLS. FURROWING OF THE MORTAR MUST NOT BE PERMITTED.
16. MORTAR JOINTS MUST BE 3/8" THICK WITH FULL MORTAR COVERAGE ON VERTICAL AND HORIZONTAL FACE SHELLS.
17. SECURE MASONRY VENEER TO SUPPORTING WALLS OR COLUMNS AT 16" ON CENTER VERTICAL AND HORIZONTAL WITH APPROVED TIES AND/OR ANCHORS.

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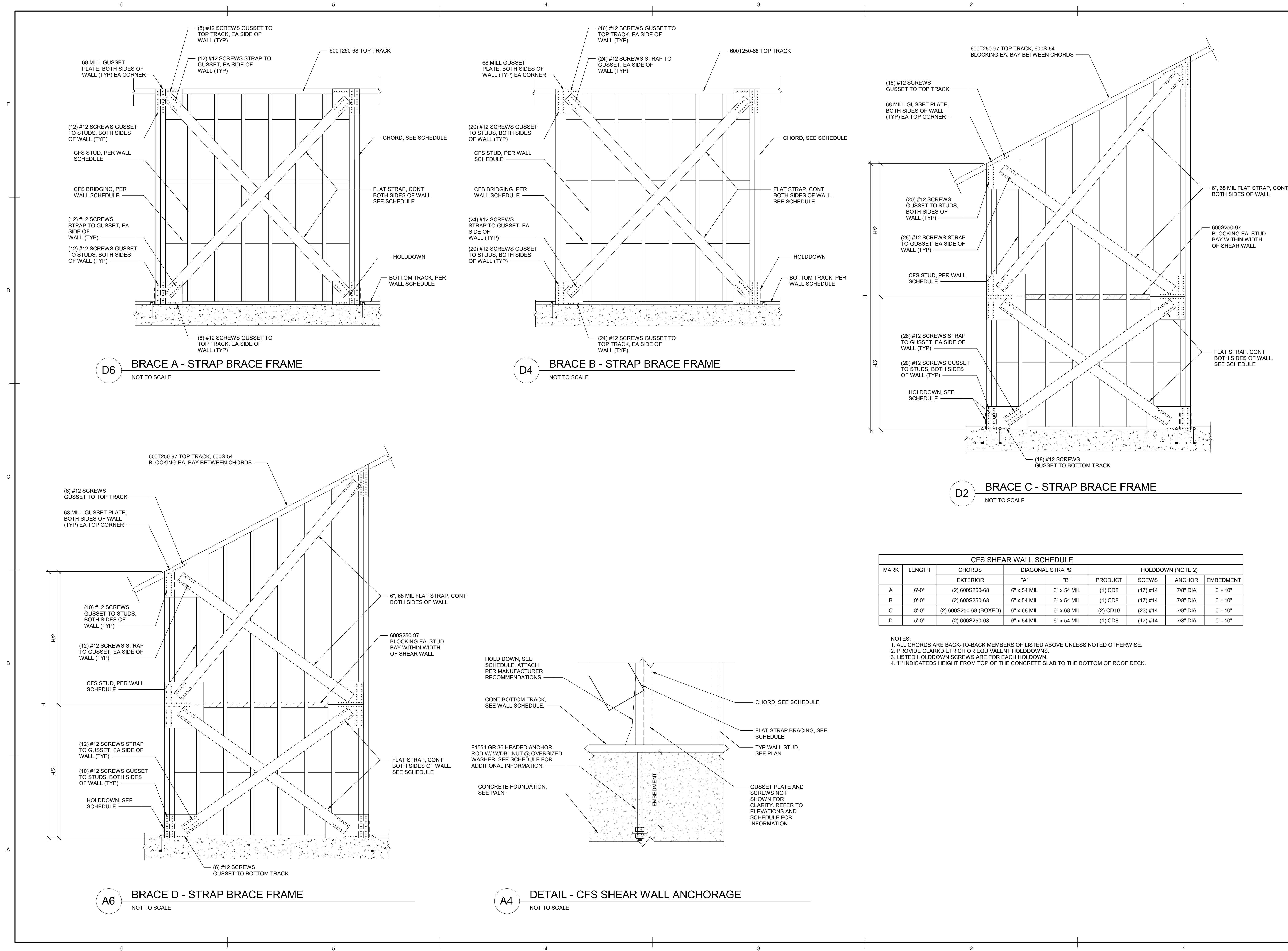
Professional Engineer Seal for Robert L. Murr, State of Alabama, License No. 20770, Expiration 5/2/2024.

FISH & WILDLIFE SERVICES BON SECOUR NAT'L WILDLIFE REFUGE. BON SECOUR VISITOR CENTER & ADMINISTRATION. BON SECOUR MWR, AL.

Table with columns for MARK, DATE, and REVISION DESCRIPTION.

Project information: COMM NO: 230182.00, DATE: 05/02/2024, DRAWN: JH, DESIGN: JH, CHECK: JR, SHEET TITLE: STRUCTURAL GENERAL NOTES, SHT. NO: S-001, REV. NO.

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D6 BRACE A - STRAP BRACE FRAME
NOT TO SCALE

D4 BRACE B - STRAP BRACE FRAME
NOT TO SCALE




D2 BRACE C - STRAP BRACE FRAME
NOT TO SCALE

A6 BRACE D - STRAP BRACE FRAME
NOT TO SCALE

A4 DETAIL - CFS SHEAR WALL ANCHORAGE
NOT TO SCALE

MARK	LENGTH	CHORDS		DIAGONAL STRAPS		HOLDDOWN (NOTE 2)		
		EXTERIOR	"A"	"B"	PRODUCT	SCEWS	ANCHOR	EMBEDMENT
A	6'-0"	(2) 600S250-68	6" x 54 MIL	6" x 54 MIL	(1) CD8	(17) #14	7/8" DIA	0' - 10"
B	9'-0"	(2) 600S250-68	6" x 54 MIL	6" x 54 MIL	(1) CD8	(17) #14	7/8" DIA	0' - 10"
C	8'-0"	(2) 600S250-68 (BOXED)	6" x 68 MIL	6" x 68 MIL	(2) CD10	(23) #14	7/8" DIA	0' - 10"
D	5'-0"	(2) 600S250-68	6" x 54 MIL	6" x 54 MIL	(1) CD8	(17) #14	7/8" DIA	0' - 10"

- NOTES:
 1. ALL CHORDS ARE BACK-TO-BACK MEMBERS OF LISTED ABOVE UNLESS NOTED OTHERWISE.
 2. PROVIDE CLARKDIETRICH OR EQUIVALENT HOLDDOWNS.
 3. LISTED HOLDDOWN SCREWS ARE FOR EACH HOLDDOWN.
 4. 'H' INDICATEDS HEIGHT FROM TOP OF THE CONCRETE SLAB TO THE BOTTOM OF ROOF DECK.

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MARK	DATE	REVISION DESCRIPTION

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DATE: 05/02/2024

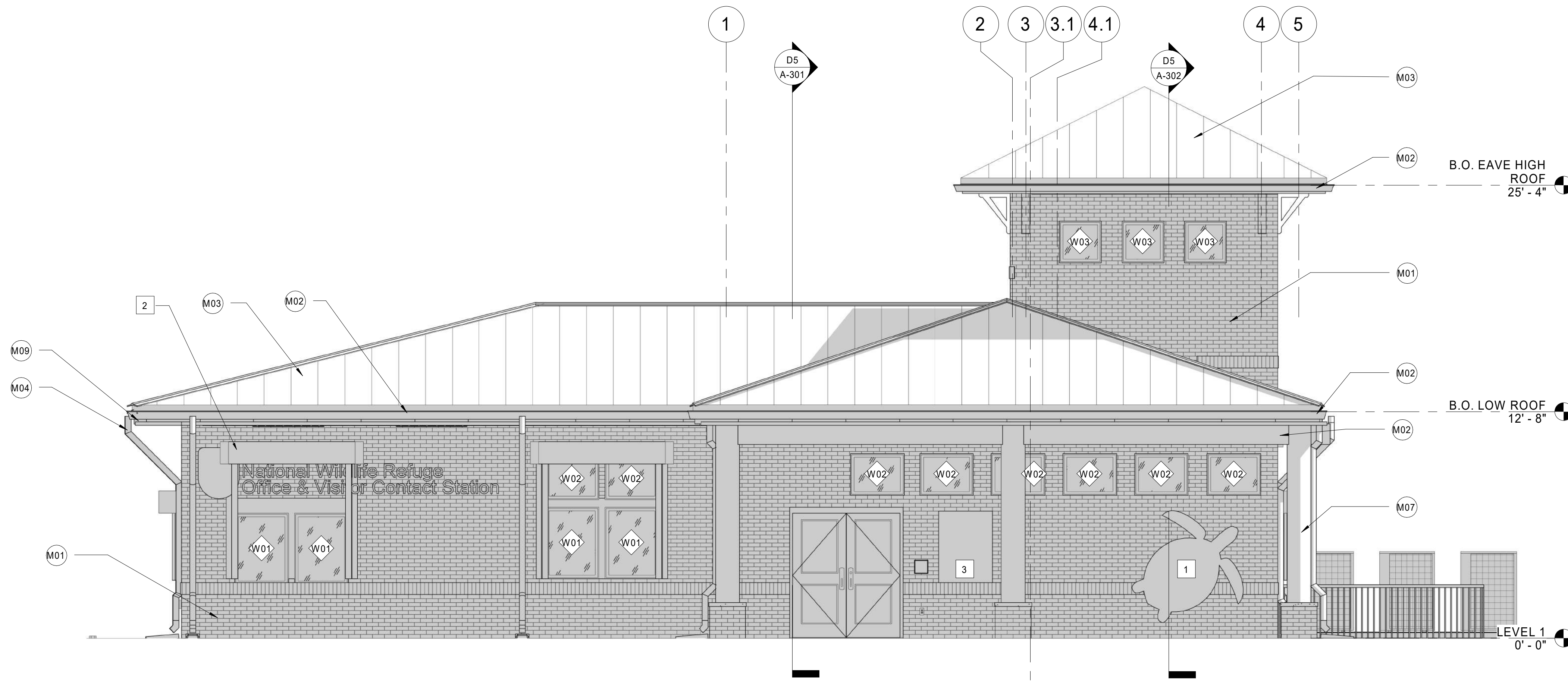
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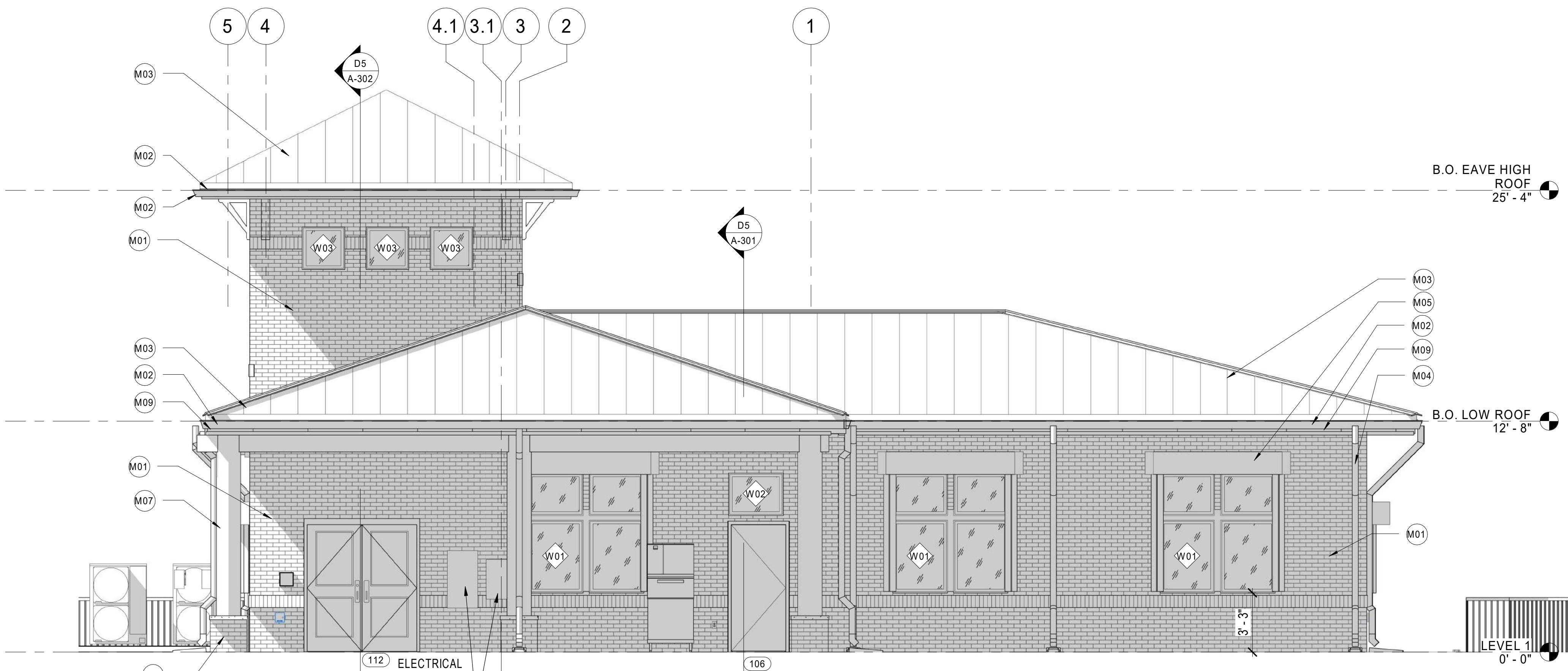
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CFS STRAP BRACE ELEVATIONS

SHT. NO.	REV. NO.
S-202	



C6 PLAN ELEVATION - SOUTH
SCALE: 1/4" = 1'-0"



A6 PLAN ELEVATION - NORTH
SCALE: 1/4" = 1'-0"

GENERAL SHEET NOTES

- 1 WHERE COLOR IS SHOWN AS BEING SPECIFIC TO ONE MANUFACTURER, AN EQUIVALENT COLOR BY ANOTHER MANUFACTURER MAY BE SUBMITTED FOR APPROVAL. MANUFACTURERS AND MATERIALS SPECIFIED ARE NOT INTENDED TO LIMIT THE SELECTION OF EQUAL COLORS FROM OTHER MANUFACTURERS. PRIOR TO PROCURING MATERIAL THE FINAL COLOR SELECTION MUST BE PER USFWS APPROVAL.
- 2 REFER TO HORIZONTAL AND VERTICAL ASSEMBLY DETAILS ON SHEETS A-601 THRU A-603 FOR EXTERIOR WALL AND ROOF ASSEMBLY.
- 3 REFER TO DOOR SCHEDULE FOR DOOR TYPES, FRAME TYPES, FINISH AND STYLE.
- 4 REFER TO WINDOW SCHEDULE FOR WINDOW TYPES, FRAME TYPES, FINISH AND STYLE.
- 5 REFER TO WALL SECTION SHEETS FOR ADDITIONAL INFORMATION AND DIMENSIONS.
- 6 CONTROL JOINT LOCATIONS INDICATED ARE APPROXIMATE. COORDINATE CONTROL JOINT LOCATIONS IN FIELD TO AVOID CONFLICT WITH LINTELS. MOVEMENT JOINTS AT CHANGES IN WALL DIRECTION, WALL OPENINGS, CHANGES IN WALL HEIGHTS, CHANGES IN WALL MATERIALS, AND BELOW SELF ANGLE SUPPORTS, TYPICALLY.
- 7 ALL CAULK/SEALANT ADJACENT TO WINDOWS/DOOR FRAMES MUST MATCH THEIR RESPECTIVE COLORS UNLESS SPECIFIED OTHERWISE.
- 8 REFER TO AND COORDINATE WITH THE MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR THE LOCATIONS, SIZES AND QUANTITIES OF EXTERIOR LIGHTS, LOUVERS AND VENTS.

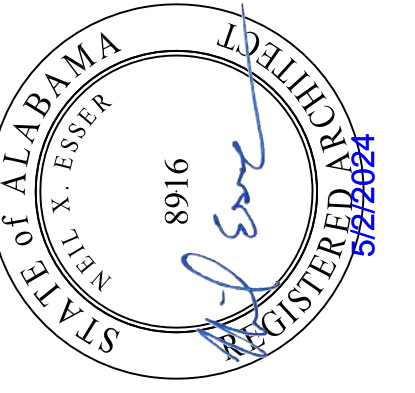
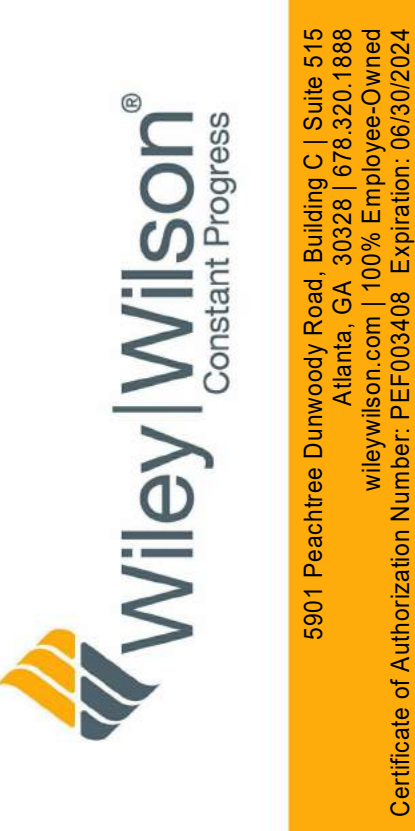
KEYNOTES

- 1 INSTALL WALL MOUNTED ART FURNISHED BY GOVERNMENT. COORDINATE INSTALLATION WITH GOVERNMENT AND PROVIDE REQUIRED BLOCKING, ANCHORING PER LOADING REQUIREMENTS AS REQUIRED ASSUME DEAD LOAD OF 200 LBS.
- 2 INSTALL AND PROCURE CAST METAL LETTERING & USFWS LOGO. CONFIRM FINAL SIGNAGE VERBIAGE WITH USFWS PRIOR TO ORDERING AND INSTALLING.
- 3 INSTALL AND PROCURE WALL MOUNTED INFORMATION KIOS. COORDINATE INSTALLATION WITH GOVERNMENT AND PROVIDE REQUIRED BLOCKING, ANCHORING PER LOADING REQUIREMENTS AS REQUIRED ASSUME DEAD LOAD OF 200 LBS.

ELEVATION FINISH SCHEDULE

MARK	BOD MFR	BOD PRODUCT	COLOR
M01	SOUTH ALABAMA BRICK COMPANY	COMMON BRICK MASONRY	SEASPRAY KLAYCOAT
M02	BASIS OF DESIGN MANUFACTURER	---	---
M03	PAC-CLAD	1 3/4" HIGH STANDING SEAM SNAP-CLAD PANEL	SILVER SMITH
M04	PAC-CLAD	PAC-TITE GOLD GUTTER & DOWNSPOUTS	MATCH M03
M05	COOKSON	STORM DEFENDER	MATCH WINDOW TRIM
M06	ARCHITECTURAL MALL, INC	FIBERGLASS BEAM WRAP	WHITE
M07	ARCHITECTURAL MALL, INC	FRP COLUMN WRAP	WHITE
M08	BASIS OF DESIGN MANUFACTURER	---	---

GRAPHIC SCALE(S)



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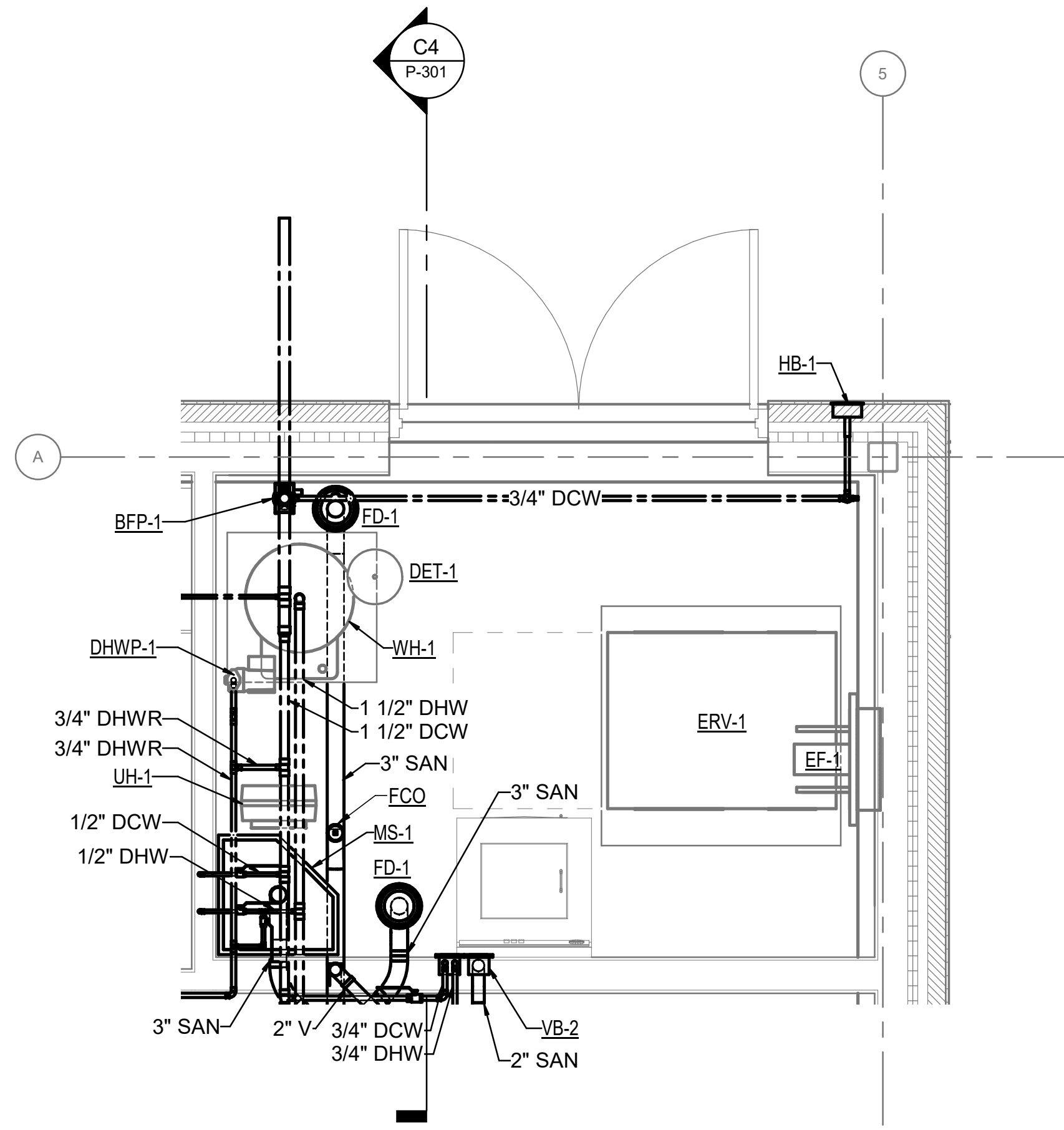
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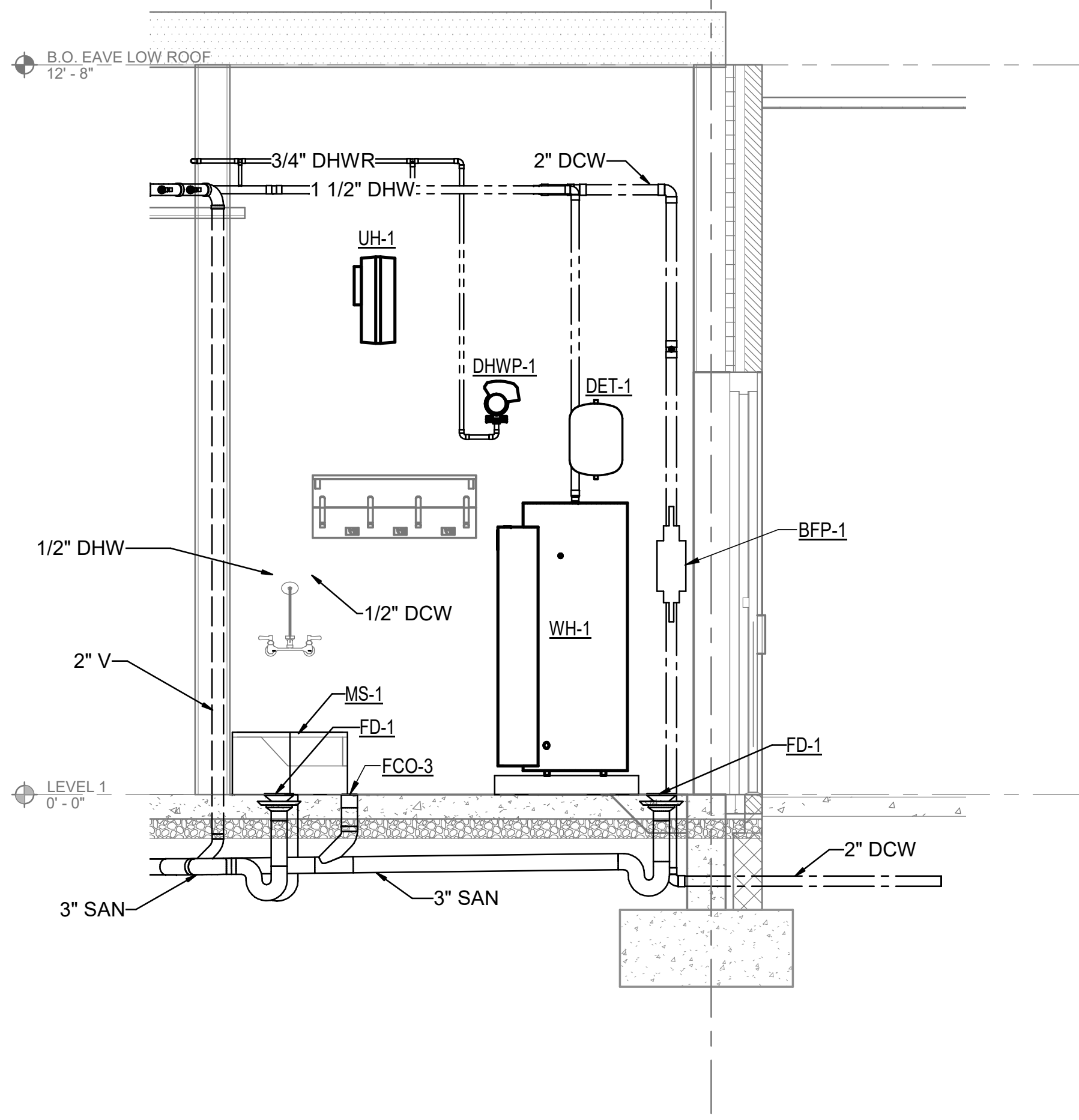
OVERALL BUILDING ELEVATIONS

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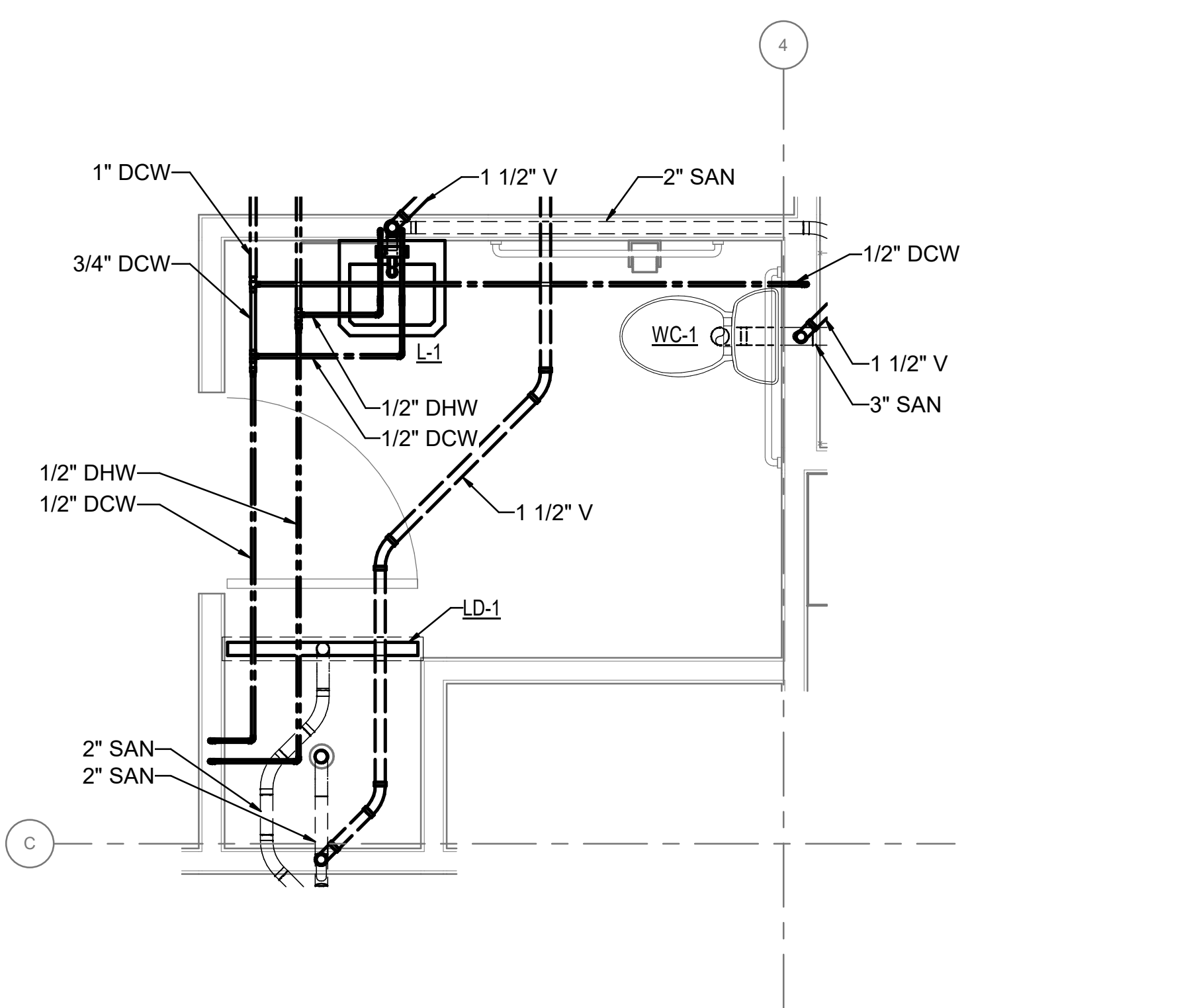
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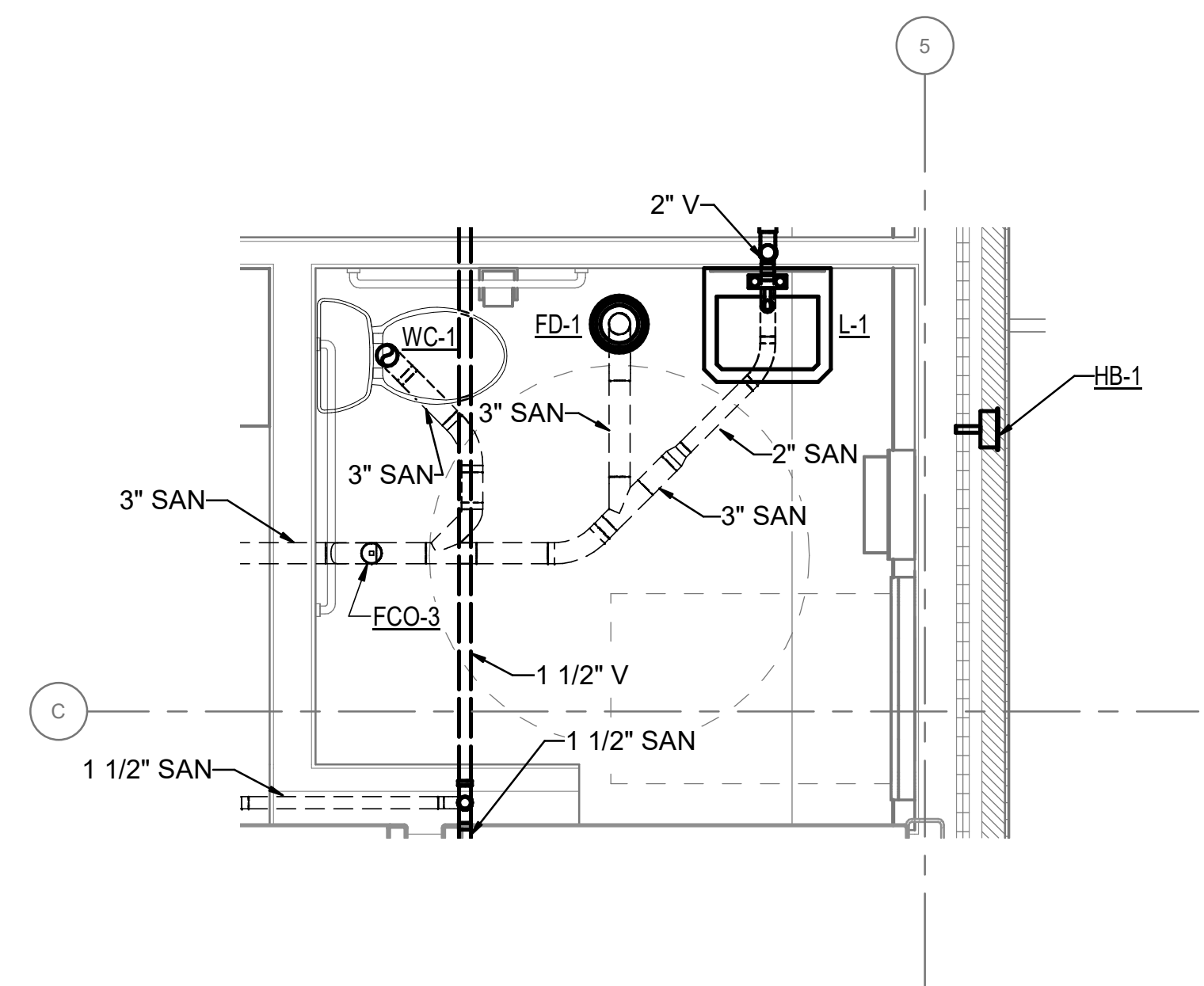
C6 ENLARGED PLAN - MECHANICAL ROOM - PLUMBING
 SCALE: 1/2" = 1'-0"
 0' 1' 2' 4'



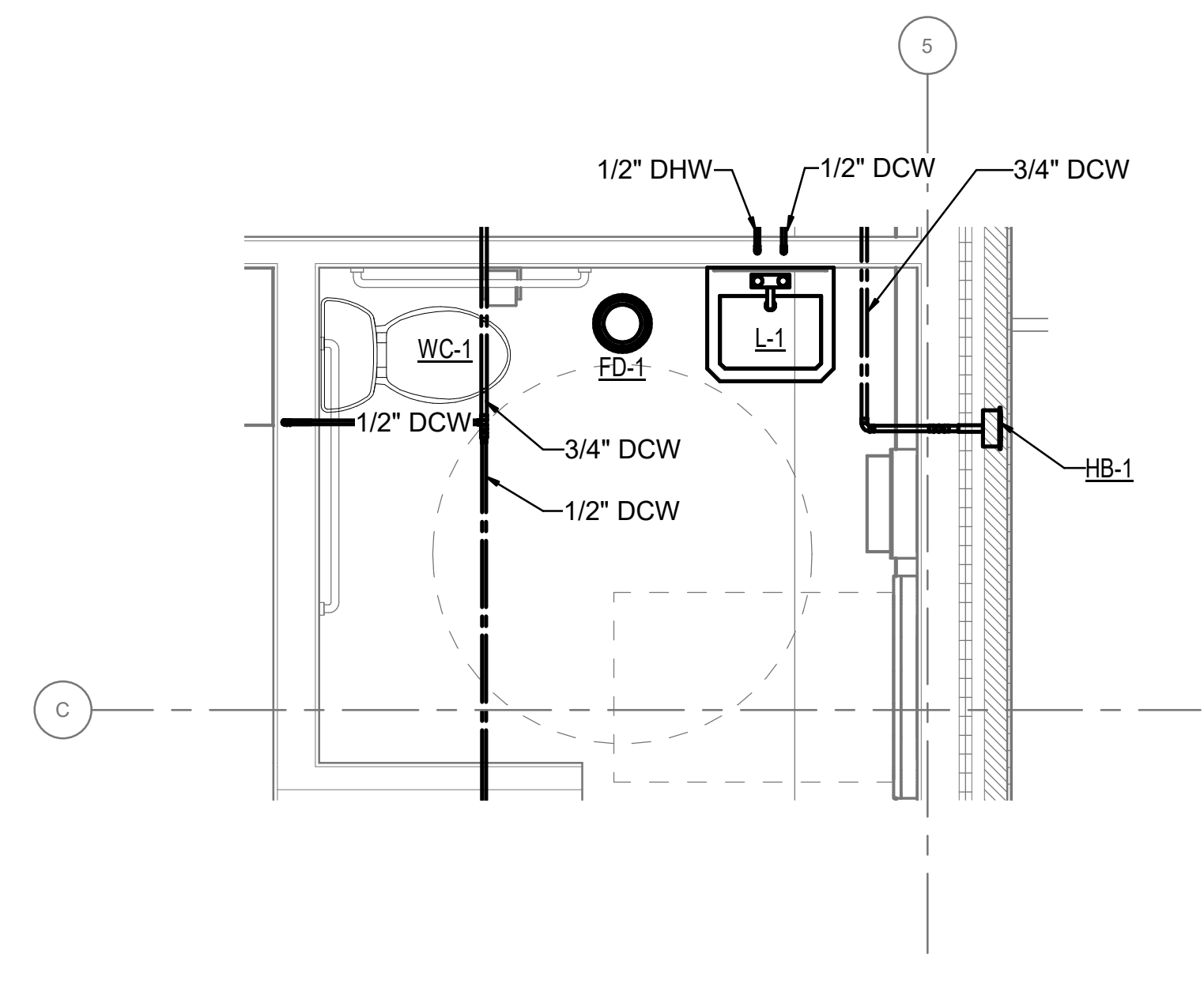
C4 PLUMBING SECTION - DOMESTIC WATER ENTRY
 SCALE: 1/2" = 1'-0"
 0' 1' 2' 4'



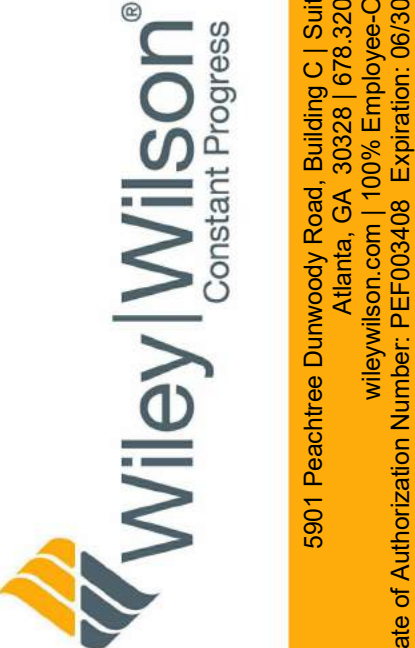
A6 ENLARGED PLAN - EMPLOYEE RESTROOM
 SCALE: 1/2" = 1'-0"
 0' 1' 2' 4'



A4 ENLARGED PLAN - PUBLIC RESTROOM - SANITARY PIPING
 SCALE: 1/2" = 1'-0"
 0' 1' 2' 4'



A2 ENLARGED PLAN - PUBLIC RESTROOM - DOMESTIC PIPING
 SCALE: 1/2" = 1'-0"
 0' 1' 2' 4'



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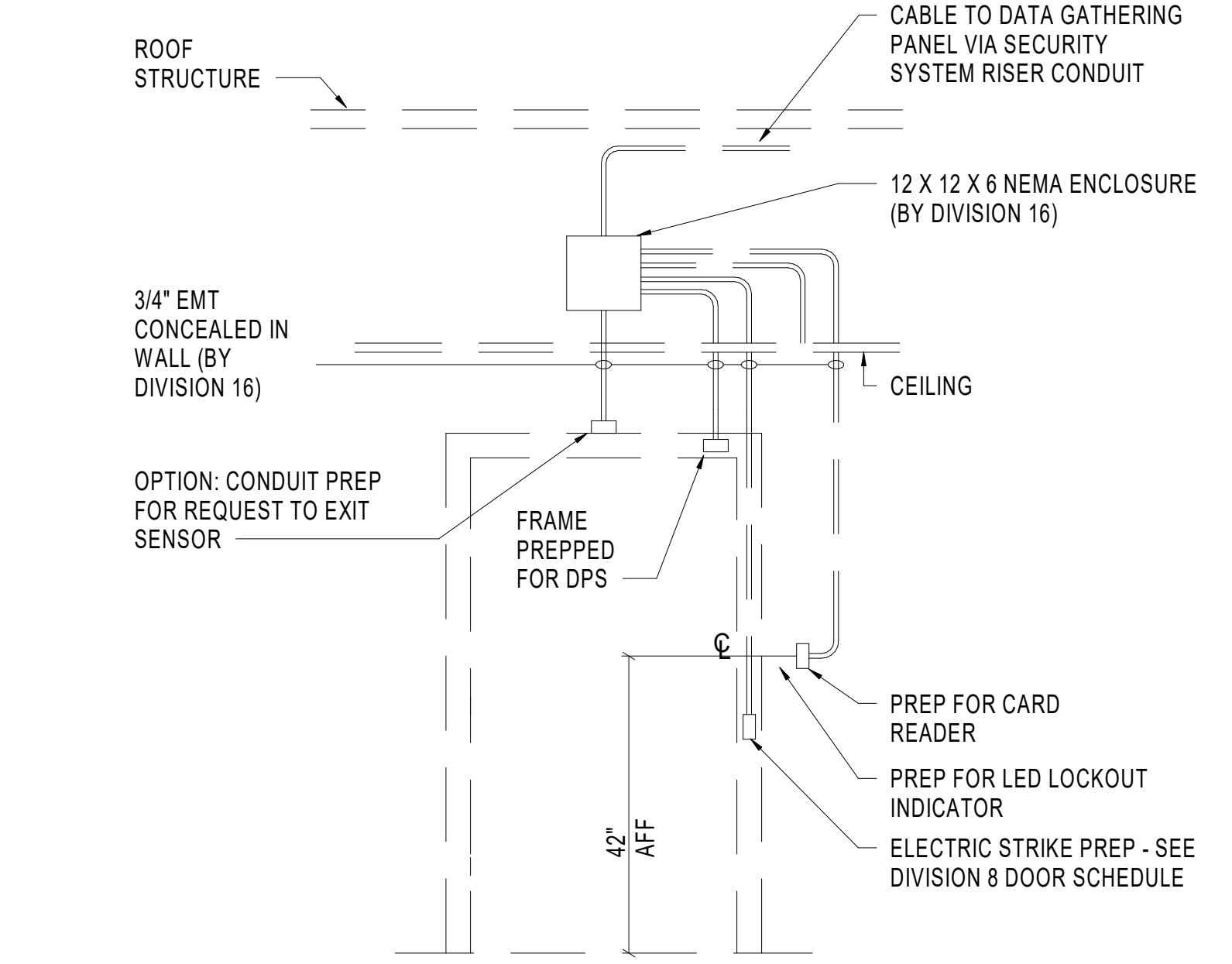
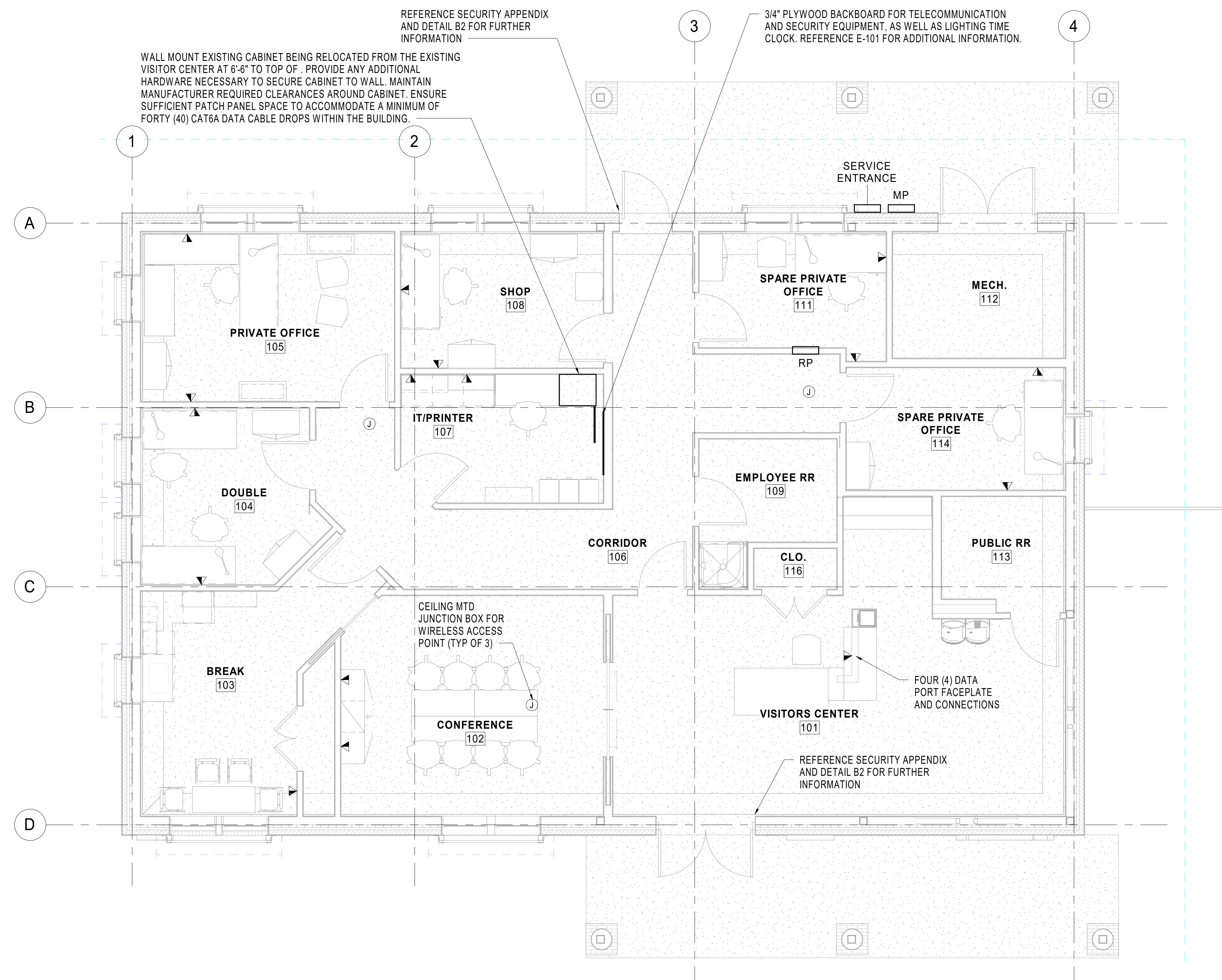
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ENLARGED PLANS AND SECTIONS

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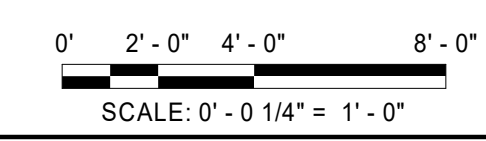
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B6 FIRST FLOOR - TELECOMMUNICATION & TECHNICAL SECURITY PLAN
SCALE: 1/4" = 1'-0"

B2 DETAIL - TYPICAL DOOR ELECTRONIC SECURITY
SCALE: NONE



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DESIGN:	EAG
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SHEET TITLE	
TELECOMMUNICATION & TECHNICAL SECURITY PLAN	
SHT. NO.	REV. NO.
E-104	

