LATHAN • McKEE
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ADDENDUM NO. 2

BEVILL JASPER INDUSTRIAL COURT ALABAMA ENERGY INFRASTRUCTURE TRAINING CENTER ADDITION PACKAGE C: INTERIOR FIT OUT

Architect Job No. 24-40C

February 27, 2026

ACCS No. 2024 070 BSCC

BIDS DUE:
Thursday, March 12, 2026, until
2:00 p.m., local time, held at
Bevill State Community College
Wade Math and Science Building
President's Conference Room
805 14th Street East
Jasper, AL 35501

The Plans and Specifications are here by amended. The following supersedes all contrary and/or conflicting information and is made part of the contract documents.

SPECIFICATIONS

1. **SECTION 10436 – AUTOMATIC EXTERNAL DEFIBRULATOR**: Add in its entirety

2. **SECTION 12150 – MISCELLANEOUS FURNISHINGS AND FIXTURES**: Revised in its entirety.

DRAWINGS

1. **Sheet A6.1** – INTERIOR ELEVATIONS: Revised interior elevations.

2. **Sheet A8.1** – FINISH FLOOR PLAN AND LEGEND – AREA A: Revised finish pattern legend

3. **Sheet A8.2** – FINISH FLOOR PLAN, LEGEND, AND DETAILS – AREA B: Revised finish pattern legend.

4. **Sheet A8.3** – FINISH SCHEDULE AND LEGEND: Revised finish schedule and materials legend and added the finish pattern legend to the sheet.

CLARIFICATIONS

1. The following documents were provided as part of Package A and B and are included in this addendum for **reference only**, see attached:
   - **Report of Subsurface Exploration and Geotechnical Engineering Evaluation** *REFERENCE ONLY*
   - **Aggregate Piers – Shop Drawings** *REFERENCE ONLY*
   - **Pre-Engineered Steel Building – Shop Drawings and Design Analysis** *REFERENCE ONLY*
   - **Alabama Infrastructure Training Center, Pkg. A Sitework Plans** *REFERENCE ONLY*

PART 1 - GENERAL

1.1 SUMMARY

A. Section Includes:

1. Automated External Defibrillators (AED).
2. Automated External Defibrillator (AED) Cabinets.

1.2 ACTION SUBMITTALS

A. Product Data: For each type of product indicated. Include construction details, material descriptions, dimensions of individual components and profiles, and finishes for AED cabinets. 1. Include roughing-in dimensions, details showing mounting methods, relationships of box and trim to surrounding construction, door hardware, cabinet type, trim style, and panel style. Include construction details, material descriptions, dimensions of individual components and profiles, and finishes for Power Heart AED G3 or a comparable product approved by the Architect.

1. Include rated capacities, operating characteristics, and furnished specialties and accessories.

B. Samples: For each type of cabinet indicated

C. Written statement that the specified AED will fit in the submitted AED cabinet.

1.3 INFORMATIONAL SUBMITTALS

A. Coordination Drawings: Coordinate sizes and locations of AED cabinets with wall depths.

B. Sample Warranty: For manufacturer's warranty.

1.4 CLOSEOUT SUBMITTALS

A. Maintenance Data: For AED and AED cabinets.

1.5 QUALITY ASSURANCE

A. Electrical Components, Devices, and Accessories: Listed and labeled as defined in NFPA 70, by a qualified testing agency, and marked for intended location and application.

1. Review methods and procedures related to AED cabinets including, but not limited to, the following:
   a. Schedules and coordination requirements.

PART 2 - PRODUCTS

2.1 MANUFACTURERS

A. AUTOMATIC EXTERNAL DEFIBRILLATOR (AED)

1. Basis-of-Design Product: Subject to compliance with requirements, provide Lifeline Power View AED (#DCF-A2310) as manufactured by Cardiac Science Corporation or comparable product approved by the Architect.

B. AED CABINET

1. Cabinet Type: Suitable for holding basis of design AED;
   a. Cabinet Construction: Nonrated.
   b. All cabinet components and equipment shall be accessible, removable and replaceable with the cabinet door in a 90 degree position.
   c. Cabinet Material: Aluminum sheet.
   d. Semirecessed Cabinet: One-piece combination trim and perimeter door frame overlapping surrounding wall surface with exposed trim face and wall return at outer edge (backbend).
      1) Rolled-Edge Trim: 2-1/2-inchbackbend depth.
   e. Cabinet Trim Material: Aluminum sheet.
   f. Size: 17.5”W x 17.5”H x 7”D
   g. Door Material: Aluminum sheet.
   h. Door Style: Standard visibility window.
   i. Door Glazing: Tempered float glass.
   j. Door Hardware: Manufacturer's standard door-operating hardware of proper type for cabinet type, trim style, and door material and style indicated.
      1) Provide manufacturer's standard.
      2) Provide continuous hinge, of same material and finish as trim, permitting door to open 180 degrees.

2. Accessories:
   a. Door Lock: Cam lock that allows door to be opened during emergency by pulling sharply on door handle.
   b. Identification: Manufacturer's standard.

3. Finishes: Aluminum – clear anodized

2.2 FABRICATION

A. AED Cabinets: Provide manufacturer's standard box (tub) with trim, frame, door, and hardware to suit cabinet type, trim style, and door style indicated.

1. Weld joints and grind smooth.
2. Provide factory-drilled mounting holes.

B. Cabinet Doors: Fabricate doors according to manufacturer's standards, from materials indicated and coordinated with cabinet types and trim styles selected.

C. Cabinet Trim: Fabricate cabinet trim in one piece with corners mitered, welded, and ground smooth.

2.3 GENERAL FINISH REQUIREMENTS

A. Protect mechanical finishes on exposed surfaces from damage by applying a strippable, temporary protective covering before shipping.

B. Finish products after assembly.

C. Aluminum: Clear Anodized finish.

PART 3 - EXECUTION

3.1 EXAMINATION

A. Examine walls and partitions for suitable framing depth and blocking where recessed cabinets will be installed.

B. Proceed with installation only after unsatisfactory conditions have been corrected.

3.2 INSTALLATION

A. General: Install AED cabinets in locations as shown on the Drawings and at mounting heights, at heights acceptable to the Authority Having Jurisdiction (AHJ)

B. AED Cabinets: Fasten cabinets to structure, square and plumb.

3.3 ADJUSTING AND CLEANING

A. Remove temporary protective coverings and strippable films, if any, as cabinets are installed unless otherwise indicated in manufacturer's written installation instructions.

B. Adjust cabinet doors to operate easily without binding.

C. On completion of cabinet installation, clean interior and exterior surfaces as recommended by manufacturer.

D. Touch up marred finishes or replace cabinets that cannot be restored to factory-finished appearance. Use only materials and procedures recommended or furnished by AED cabinet and mounting bracket manufacturers.

E. Replace AED cabinets that have been damaged or have deteriorated beyond successful repair by finish touchup or similar minor repair procedures.

END OF SECTION

1.0 - GENERAL

1.1 Scope

The work of this section consists of furnishing and installing complete, all miscellaneous furnishings, fixtures, and signage items as indicated.

1.2 Existing Conditions

A. It is the general contractor's responsibility to field verify existing signage before a bid and provide signage that shall match all existing signage types and styles currently installed to provide a continuity of design to the owner as required.

1.3 Submittals

Shop drawings shall be submitted.

2.0 - PRODUCTS

2.1 Building Letters

A. Cast aluminum letters, equal to Leeds Architectural Letters, Inc., Select from all available fonts Size: As indicated on drawings, lay-out as indicated. Colors as selected by Architect. Provide flush concealed stud mounting.

2.2 Building Plaque

A. Dedication plaque shall be of cast aluminum. Furnish and install a 24" x 42" plaque with approximately 500 raised letters and raised border. Field shall have stipple finish. Face of letters and borders shall have ground satin finish surface.

B. Plaque layout and designation shall be furnished by the Architect.

2.3 Appliances

A. **Display Refrigerator Merchandiser** - True GDM-23-HC~TSL01
   1. Manufacturer - True
   2. Model GDM-23-HC~TSL01
   3. See Electrical/Plumbing drawings and provide equipment with all required electrical and plumbing rough-ins, hook-ups, and installations.
   4. Warranty: Provide manufacturer's 7-year warranty
   5. Provide Operations/Maintenance Demonstration for Owner.

B. **Ice and Water Dispenser** - Scotsman HID312 Meridian Series Ice and Water Dispenser
   1. Manufacturer - Scotsman
   2. Model - HID312 Meridian Series
   3. Provide manufacturer's recommended machine stands.
   4. See Electrical/Plumbing drawings and provide equipment with all required electrical and plumbing rough-ins, hook-ups, and installations.
   5. Warranty: Provide manufacturer's 3-year warranty
   6. Provide Operations/Maintenance Demonstration for Owner.
   7. Provide manufacturer's recommended machine stands.

C. **Refrigerator -** GE Appliances Model # GDE25EYKFS
   1. See Electrical/Plumbing drawings and provide equipment with all required electrical and plumbing rough-ins, hook-ups, and installations.
   2. Warranty: Provide manufacturer's 1-year warranty.
   3. Provide Operation/Maintenance Demonstration for Owner.

2.4 KnoxBox
Provide one Standard Capacity Model 3274 KnoxBox 3200 - Location as directed by the Architect
Color: Black
Mount Type: Standard Mount
Tamper Switch Type: None

~~2.5 Chemical Toilet~~
~~Provide Serene Life Portable Toilet (s) SLCATL320 or pre-approved equal as required in the Storm Shelter. Provide Maintenance and Instructions with Closeouts.~~

2.5 Wall Corner Protective Guards (Op.2)

A. General
Furnish and install 8'-0" high (or cut to fit) corner guards as indicated on the drawings. Surface mounted corner guards VA Series as manufactured by CS Acrovyn.

B. Corner guards to be surface mounted with self adhesive tape backing. Provide 2-1/2" CS Acrovyn # VA-250 or pre-approved equal. Color to be selected.

2.6 Project Sign - Specification requirements are listed in Section 01030.

3.0 - EXECUTION

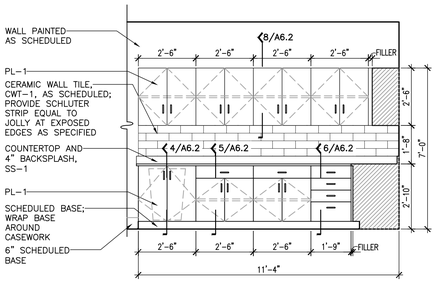
3.1 Installation
Installation of all items shall be in full conformity with manufacturer's specifications, recommendations, and approved details.

3.2 Installation of Building Letters
Install building letters on surfaces and at heights as directed. Install in accordance with manufacturer’s recommendations.

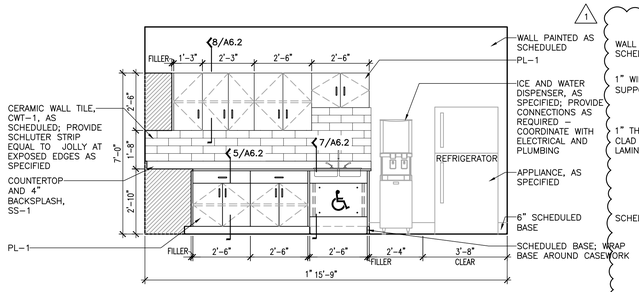
3.3 Installation of Plaque(s)
Install plaque(s) where directed.

3.4 Installation of Appliances
Install appliances as directed. Install in accordance with manufacturer’s recommendations.

END OF SECTION



1 INTERIOR ELEVATION @ BREAK ROOM A103
SCALE: 3/8" = 1'-0"



2 INTERIOR ELEVATION @ BREAK ROOM A103
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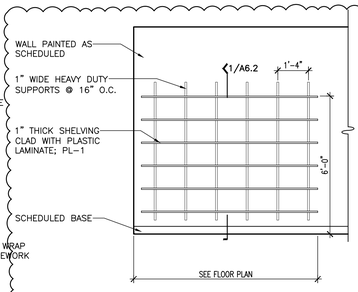

3 INTERIOR ELEVATION @ STORAGE A105
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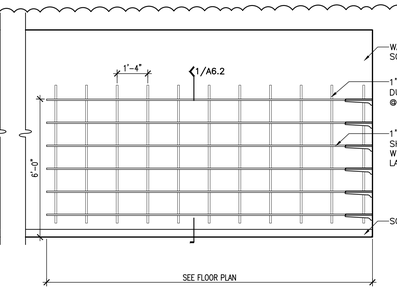

4 INTERIOR ELEVATION @ STORAGE A105
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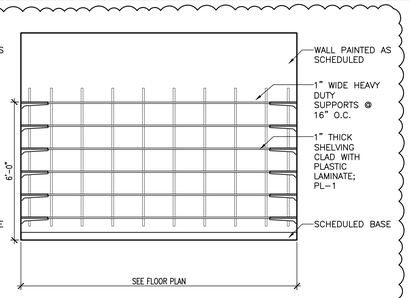

5 INTERIOR ELEVATION @ STORAGE A105
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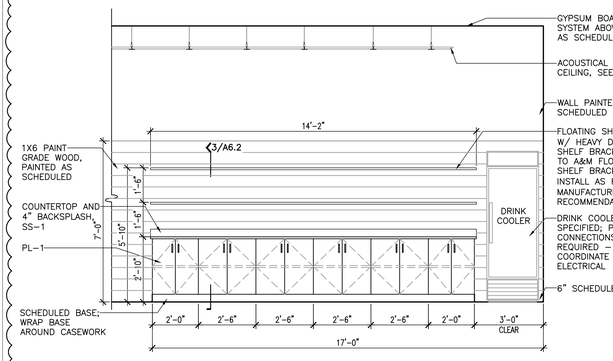
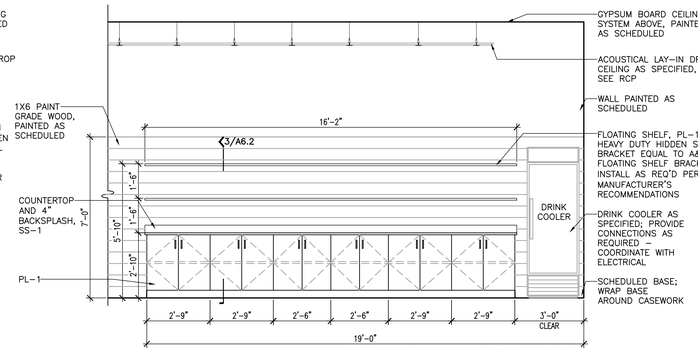
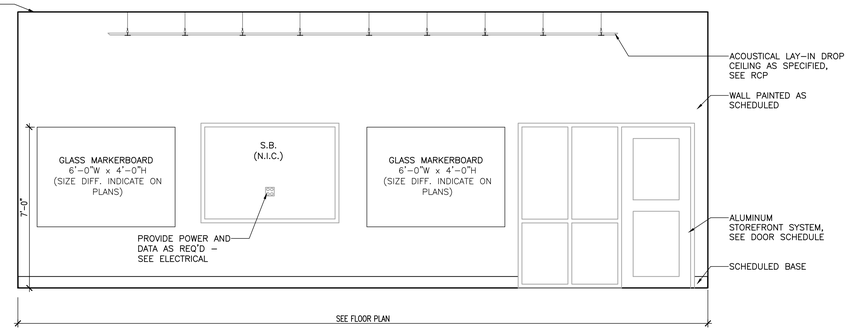


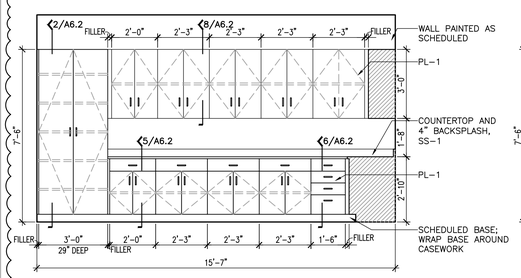
6 INTERIOR ELEVATION @ SOLAR CLASSROOM A111, EV CLASSROOM A117, EV CHARGER CLASSROOM B100, AND CONNECTIVITY CLASSROOM B106
SCALE: 3/8" = 1'-0"



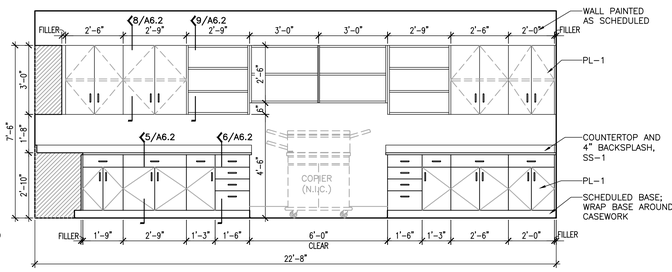
7 INTERIOR ELEVATION @ DUAL ENROLLMENT SOLAR CLASSROOM A113, DUAL ENROLLMENT EV CLASSROOM A119, AND DUAL ENROLLMENT EV CHARGER CLASSROOM B102
SCALE: 3/8" = 1'-0"



8 INTERIOR ELEVATION @ TYPICAL CLASSROOM GLASS MAKERBOARD/TV
SCALE: 3/8" = 1'-0"



9 INTERIOR ELEVATION @ COPIER/OFFICE SUPPLIES B124
SCALE: 3/8" = 1'-0"



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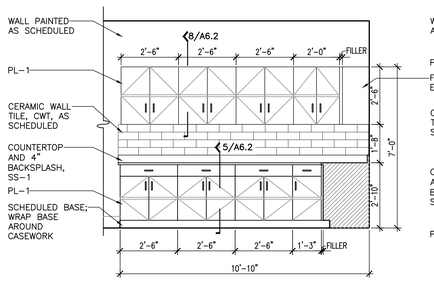
SCALE: 3/8" = 1'-0"



11 INTERIOR ELEVATION @ KITCHENETTE B125
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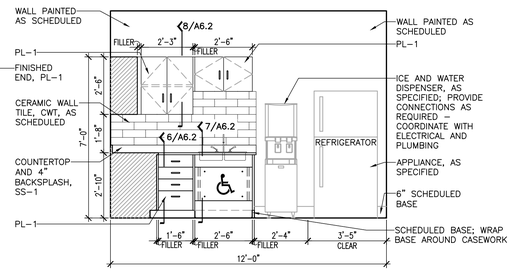


12 INTERIOR ELEVATION @ KITCHENETTE B125
SCALE: 3/8" = 1'-0"

LATHAN ▪ McKEE ARCHITECTS

ALABAMA ENERGY INFRASTRUCTURE TRAINING CENTER

PACKAGE C: INTERIOR FIT OUT
3711 INDUSTRIAL COURT, JASPER, ALABAMA 35501
BEVILL STATE COMMUNITY COLLEGE

SHEET TITLE: INTERIOR ELEVATIONS

STATE OF ALABAMA
No. 7007
RYAN D. VERNON
REGISTERED ARCHITECT

PROJ. MGR.: S. CALMA
DRAWN: K.JOINER
DATE: 12.19.25
REVISIONS
1 02.26.26 ADD #2

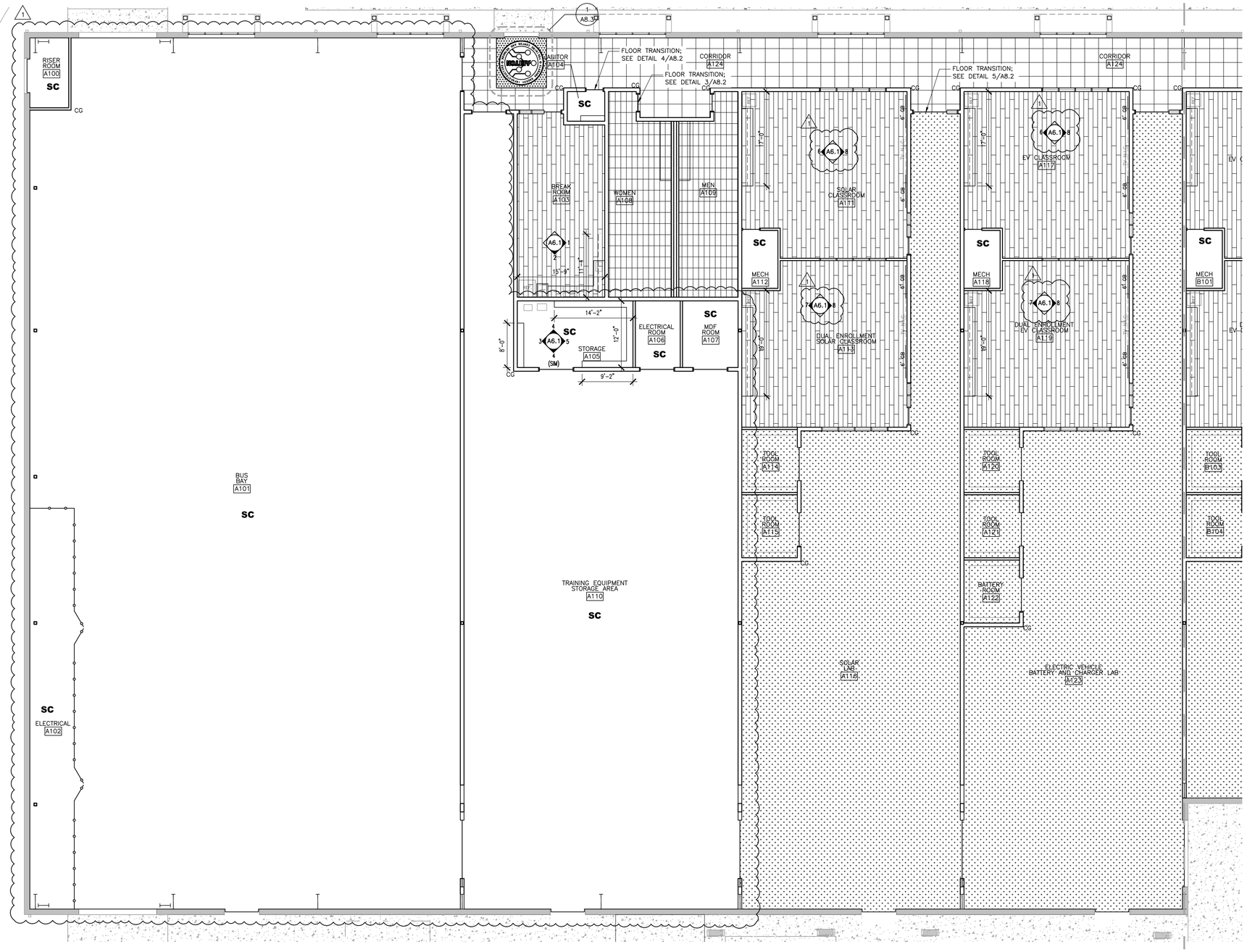
JOB NO. 24-40C
SHEET NO:
A6.1
16 OF 24
0 1" 2"

FINISH PATTERN LEGEND

SC — SEALED CONCRETE	LVT–1 — LUXURY VINYL TILE
ERF–1 — EPOXY RESIN FLOOR	PFT–1 — PORCELAIN FLOOR TILE
PFT–2 — PORCELAIN FLOOR TILE	CPT–1 — CARPET
CPT–2 — CARPET	CPT — CUSTOM CARPET LOGO

NOTE: CORNER GUARDS, CG, SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS ON ALL GYPSUM WALL CORNERS. SEE FINISH SCHEDULE FOR COLOR SELECTION

AREA–A A8.1 | AREA–B A8.2



1 FINISH FLOOR PLAN

SCALE: 1/8" = 1'–0" AREA A

LATHAN ▪ McKEE ARCHITECTS

ALABAMA ENERGY INFRASTRUCTURE TRAINING CENTER

PACKAGE C: INTERIOR FIT OUT

3711 INDUSTRIAL COURT, JASPER, ALABAMA 35501

BEVILL STATE COMMUNITY COLLEGE

SHEET TITLE: FINISH FLOOR PLAN AND LEGEND - AREA A

STATE OF ALABAMA
No. 7007
RYAN D. VERNON
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PROJ. MGR.:	S. CALMA
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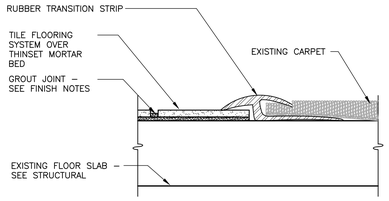
JOB NO. 24-40C

SHEET NO:

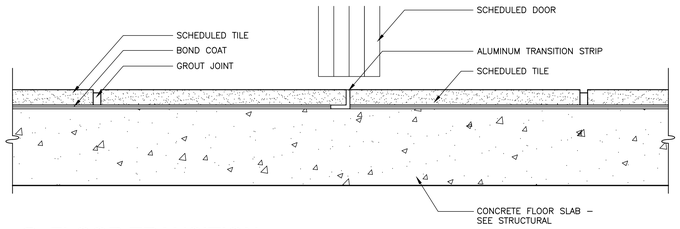
A8.1

20 OF 24

0 1" 2"



2 FLOOR TRANSITION @ EXISTING CARPET TO NEW TILE
NOT TO SCALE



3 FLOOR TRANSITION @ TILE TO TILE
NOT TO SCALE

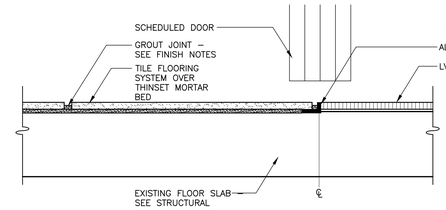
AREA–A A8.1 | AREA–B A8.2

1 FINISH FLOOR PLAN
SCALE: 1/8" = 1'–0" AREA B

FINISH PATTERN LEGEND

SC – SEALED CONCRETE	LVT–1 LUXURY VINYL TILE
ERF–1 EPOXY RESIN FLOOR	PFT–1 PORCELAIN FLOOR TILE
PFT–2 PORCELAIN FLOOR TILE	CPT–1 CARPET
CPT–2 CARPET	CPT CUSTOM CARPET LOGO

NOTE: CORNER GUARDS, CG, SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS ON ALL GYPSUM WALL CORNERS. SEE FINISH SCHEDULE FOR COLOR SELECTION



4 FLOOR TRANSITION @ TILE TO LVT
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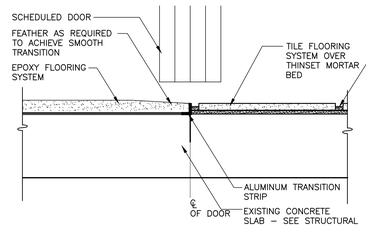
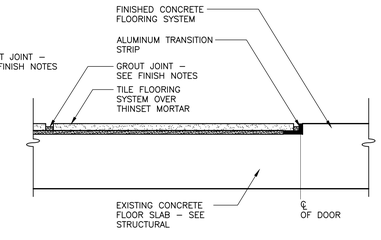


5 FLOOR TRANSITION @ TILE TO EPOXY
NOT TO SCALE



6 FLOOR TRANSITION @ TILE TO CONCRETE
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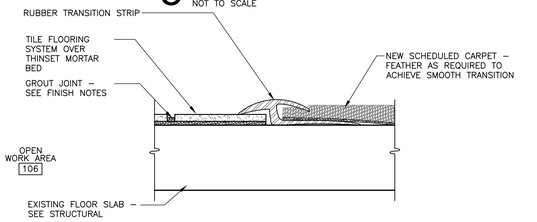
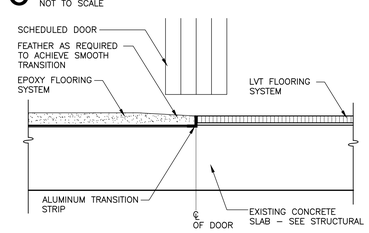


7 FLOOR TRANSITION @ TILE TO CARPET
NOT TO SCALE



8 FLOOR TRANSITION @ EPOXY TO LVT
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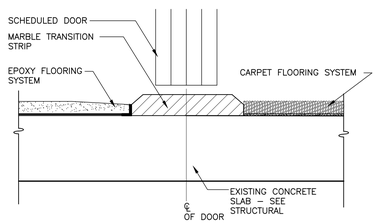
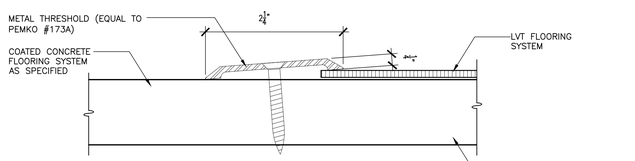


9 FLOOR TRANSITION @ EPOXY TO CARPET
NOT TO SCALE



10 FLOOR TRANSITION @ CONCRETE TO LVT
NOT TO SCALE

LATHAN ▪ McKEE ARCHITECTS

ALABAMA ENERGY INFRASTRUCTURE TRAINING CENTER
PACKAGE C: INTERIOR FIT OUT
3711 INDUSTRIAL COURT, JASPER, ALABAMA 35501
BEVILL STATE COMMUNITY COLLEGE

SHEET TITLE: FINISH FLOOR PLAN, LEGEND, AND DETAILS - AREA B

STATE OF ALABAMA
No. 7007
RYAN D. VERNON
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PROJ. MGR.:	S. CALMA
DRAWN:	K.JOINER
DATE:	12.19.25
REVISIONS	
1	02.26.26 ADD #2

JOB NO. 24-40C

SHEET NO:

A8.2

21 OF 24

0 1" 2"

FINISH SCHEDULE

ROOM NO.	ROOM NAME	FLOOR	BASE	MILLWORK FACE	MILLWORK TOP	WALLS NORTH	WALLS SOUTH	WALLS EAST	WALLS WEST	DOOR FRAME	CEILING/SOFFIT/ ACCENT PAINT	NOTES
AREA A												
A100	RISER ROOM	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A101	BUS BAY	SC	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		ALT. 3 - FLOORING SHALL BE ERF-1 WITH A COVE BASE
A102	ELECTRICAL	SC	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		ALT. 3 - FLOORING SHALL BE ERF-1 WITH A COVE BASE
A103	BREAK ROOM	LVT-1	RB-1/2	PL-1	SS-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
A104	JANITOR	SC	NO BASE	PL-1	PL-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		ALT. 3 - FLOORING SHALL BE ERF-1 WITH A COVE BASE
A105	STORAGE	SC	RB-1	PL-1	PL-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		ALT. 3 - FLOORING SHALL BE ERF-1 WITH A COVE BASE
A106	ELECTRICAL ROOM	SC	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		ALT. 3 - FLOORING SHALL BE ERF-1 WITH A COVE BASE
A107	MDF ROOM	SC	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		ALT. 3 - FLOORING SHALL BE ERF-1 WITH A COVE BASE
A108	WOMEN	PFT-2	PFT-2	PL-1	SS-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		EPOXY PAINT AT ALL WALLS
A109	MEN	PFT-2	PFT-2	PL-1	SS-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		EPOXY PAINT AT ALL WALLS
A110	TRAINING EQUIP. STOR. AREA	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A111	SOLAR CLASSROOM	LVT-1	RB-1/2	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
A112	MECHANICAL	SC	NO BASE	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A113	DUAL ENROLLMENT SOLAR	LVT-1	RB-1/2	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
A114	TOOL ROOM	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A115	TOOL ROOM	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A116	SOLAR LAB	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A117	EV CLASSROOM	LVT-1	RB-1/2	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
A118	MECHANICAL	SC	NO BASE	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A119	DUAL ENROLLMENT EV	LVT-1	RB-1/2	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
A120	TOOL ROOM	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A121	TOOL ROOM	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A122	BATTERY ROOM	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A123	EV BATTERY & CHARGER LAB	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
A124	CORRIDOR	PFT-1/CPT-2/3	PFT-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		CUSTOM INSET ENTRY CARPET, CPT-2 AND CTP-3
AREA B												
B100	EV CHARGER CLASSROOM	LVT-1	RB-1/2	PL-1	SS-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
B101	MECHANICAL	SC	NO BASE	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B102	DUAL ENROLL. EV CHARGER	LVT-1	RB-1/2	PL-1	SS-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
B103	TOOL ROOM	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B104	TOOL ROOM	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B105	ELECTRIC VEHICLE LAB	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B106	CONNECTIVITY CLASSROOM	LVT-1	RB-1/2	PL-1	SS-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
B107	MECHANICAL	SC	NO BASE	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B108	OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B109	OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B110	TOOL ROOM	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B111	IDF	ERF-1	ERB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B112	MECHANICAL	ERF-1	ERF-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	
B113	CONNECTIVITY LAB	ERF-1	ERF-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	
B114	CORRIDOR	PFT-1/CPT-2/3	PFT-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		CUSTOM INSET ENTRY CARPET, CPT-2 TO CPT-8
B115	LOBBY	PFT-1	PFT-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	
B116	RECEPTION	PFT-2	PFT-2	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B117	CORRIDOR	PFT-2	PFT-2	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	
B118	OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B119	OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B120	OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B121	OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B122	OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B123	OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B124	OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B125	TOILET	PFT-2	PWT-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		EPOXY PAINT AT ALL WALLS
B126	EXECUTIVE OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B127	EXECUTIVE OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B128	EXECUTIVE OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B129	EXECUTIVE OFFICE	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B130	ELECTRICAL	SC	NO BASE	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		
B131	KITCHENTTE	LVT-2	RB-1/2	PL-1	SS-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
B132	COPIER/OFFICE SUPPLIES	LVT-2	RB-1/2	PL-1	SS-1	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2		USE RB-2 (4" BASE) ONLY WHERE CASEWORK OCCURES
B133	CONFERENCE ROOM	CPT-1	RB-1	-	-	PNT-1	PNT-1	PNT-1	PNT-1	PNT-2	PNT-3	

BASE(RUBBER/CERAMIC/PORCELAIN/WOOD)

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
RB-1	MANNINGTON	COLOR: IRISH GREY 110 6" SCULPTURED WALL BASE, EDGE EFFECTS, SIMPLICITY EESM	SEE FINISH SCHEDULE
RB-2	MANNINGTON	COLOR: IRISH GREY 110 4" SCULPTURED WALL BASE, EDGE EFFECTS, SIMPLICITY EESM	SEE FINISH SCHEDULE; CASEWORK BASE ONLY
ERB-1	TORGINOL	6" BASE, MATCH ERF-1	SEE FINISH SCHEDULE

EPOXY RESIN FLOOR

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
ERF-1	TORGINAL	2" BROADCAST FLAKES CUSTOM COLOR MIX	AS INDICATED ON FINISH SCHEDULE

LUXURY VINYL TILE

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
LVT-1	MANNINGTON COMMERCIAL	COLLECTION:SPADA+ABSTRACT COLOR: CONSTELLATION GREY SSSA4702 SIZE: 12X28	SEE SCHEDULE
LVT-2	MANNINGTON COMMERCIAL	COLLECTION: THE GRIT+WOOD COLOR: DUTCH ELM TINSMITH 0104 SIZE: 7.25X48	SEE SCHEDULE

TILE

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
CWT-1	CREATIVE MATERIALS CORPORATION	COLLECTION: BRIXTON COLOR: WHITE SIZE: 3X12 GROUT:	BACKSPLASH IN BREAK ROOM A103 AND KITCHENETTE B131
PWT-1	DALTILE	COLLECTION: MODERNIST COLOR: FOG F103 SIZE: 12X24 GROUT:	ALL WOMEN'S
PFT-1	DALTILE	COLLECTION: MODERNIST COLOR: PROVE STEEL MD88 SIZE: 36X36 GROUT:	SEE SCHEDULE
PFT-2	DALTILE	COLLECTION:FIXTURE COLOR: GLACIER FX21 SIZE: 12X24 GROUT:	SEE SCHEDULE

ACOUSTICAL BAFFLES

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
AB-1	FRÄSCHI	COLLECTION: BAFL CLASSIC COLOR: MAHOGANY ON SILVER 13P-82	SEE RCP

WOOD DOOR

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
WD-1	MASONITE ARCHITECTURAL	SPECIES: WHITE BIRCH (PLAIN SPLICE) STAIN COLOR: COCOA BEAN	WOOD DOORS

PAINT

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
PNT-1	SHERWIN WILLIAMS	COLOR: AGREEABLE GRAY SW 7029	GENERAL WALLS
PNT-2	SHERWIN WILLIAMS	COLOR: DORIAN GRAY SW 7017	GENERAL TRIM
PNT-3	SHERWIN WILLIAMS	COLOR: CEILING BRIGHT WHITE SW 7007	GENERAL CEILING/SOFFIT

PLASTIC LAMINATE

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
PL-1	WILSONART	COLOR: FLORENCE WALNUT 7993 PREMIUM LAMINATE WITH MATCHING EDGEBANDING	SEE FINISH SCHEDULE

SOLID SURFACE

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
SS-1	DURASEIN BY BPI	COLOR: BLOSSOMING D5027	SEE FINISH SCHEDULE

CARPET

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
CPT-1	MANNINGTON COMMERCIAL	COLLECTION:BLUEPRINT – SCRIPT COLOR: TICKTOM SIZE: 24X24 INSTALLATION: ASHLAR	SEE SCHEDULE
CPT-2	INTERFACE	COLLECTION:VIVA COLORES COLOR: LOCURO 101177 INSTALLATION: CUSTOM; RECESSED IN TILE	CUSTOM LOGO
CPT-3	INTERFACE	COLLECTION:VIVA COLORES COLOR: ORO PERLA 101179 INSTALLATION: CUSTOM; RECESSED IN TILE	CUSTOM LOGO

TRIM

ITEM	MANUFACTURER	ITEM NUMBER/NAME	LOCATION
SCH-1	SCHLUTER SYSTEMS	COLLECTION:—SCHIENE PROFILE: RADIUS MATERIAL: ALUMINUM	CUSTOM LOGO – WHERE TILE MEETS CARPET AT ENTRY
SCH-2	SCHLUTER SYSTEMS	COLLECTION:—JOLLY PROFILE: RADIUS MATERIAL: ALUMINUM	AT ALL CORNERS AND TOPS OF EXPOSED TILE EDGES
CG	CONSTRUCTION SPECIALITIES	COLLECTION: VA SERIES COLOR: ACROVYN SOLID COLORS	AT ALL GYPSUM WALL BOARD CORNERS, SEE FINISH FLOOR PLAN

FINISH ABBREVIATION LEGEND

APF	ACOUSTIC PANEL FABRIC	IC	IMPRINTED CONCRETE	SC	SEALED CONCRETE
CC	COATED CONCRETE	LVT	LUXURY VINYL TILE	SCH	SCHLUTER STRIP
CM	CROWN MOLDING	NS	NATURAL STONE	STC	STAINED CONCRETE
CPT	CARPET	PL	PLASTIC LAMINATE	SS	SOLID SURFACE
CR	CHAIR RAIL	PNT	PAINT	ST	STAIN
DP	DIGITAL ACOUSTIC PANEL	PT	PORCELAIN TILE	SWB	STAINED WOOD BASE
		PTB	PORCELAIN TILE BASE	TP	TACKABLE ACOUSTIC PANEL
CG	CORNER GUARDS	PWB	PAINTED WOOD BASE		
CWT	CERAMIC WALL TILE	QT	QUARRY TILE	TS	TACKABLE SURFACE
ERB	EPOXY RESIN BASE	QTB	QUARRY TILE BASE	VCT	VINYL COMP. TILE
ERF	EPOXY RESIN FLOOR	QRB	QUARTZ RESIN BASE	WB	WOOD BASE
ESD	STATIC CONTROL TILE	QRF	QUARTZ RESIN FLOOR	WC	WALLCOVERING
GYP	GYPSUM BOARD	RB	RUBBER BASE	WF	WOOD FLOORING
		RF	RUBBER FLOOR	WP	WOOD PANELING
				WV	WOOD VENEER

FINISH NOTES

ALL WALLS TO BE PAINTED PNT-1 UNLESS NOTED OTHERWISE.

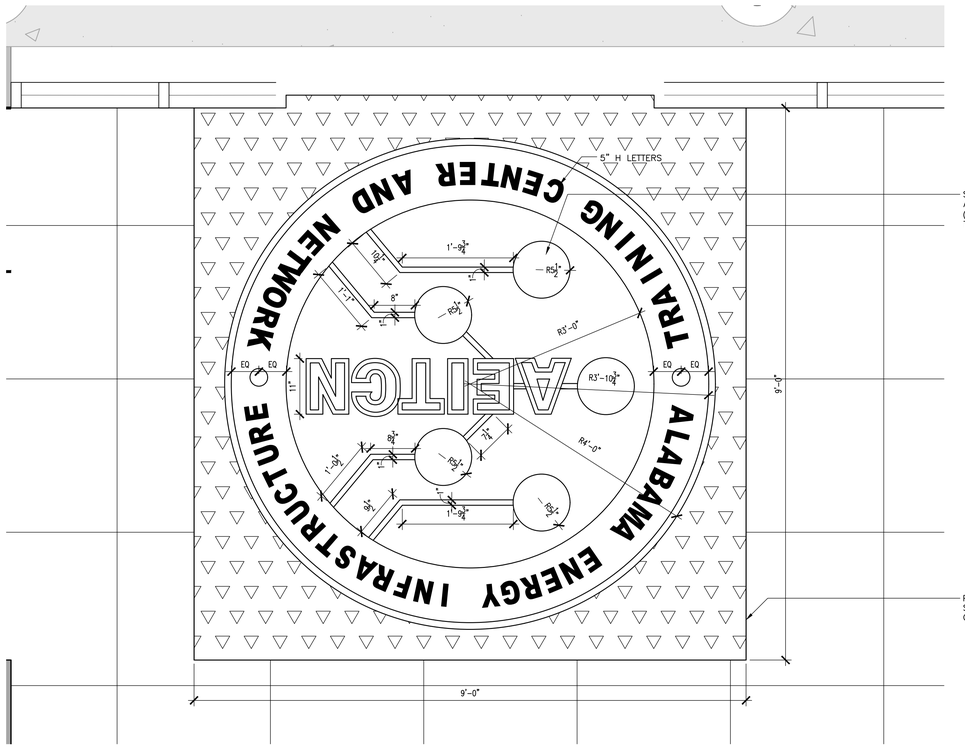
ALL EXPOSED STRUCTURE SHALL BE PAINTED BLACK; REFER TO ELEVATIONS

ALL SLOPED CEILINGS SHALL BE PAINTED PNT-3.

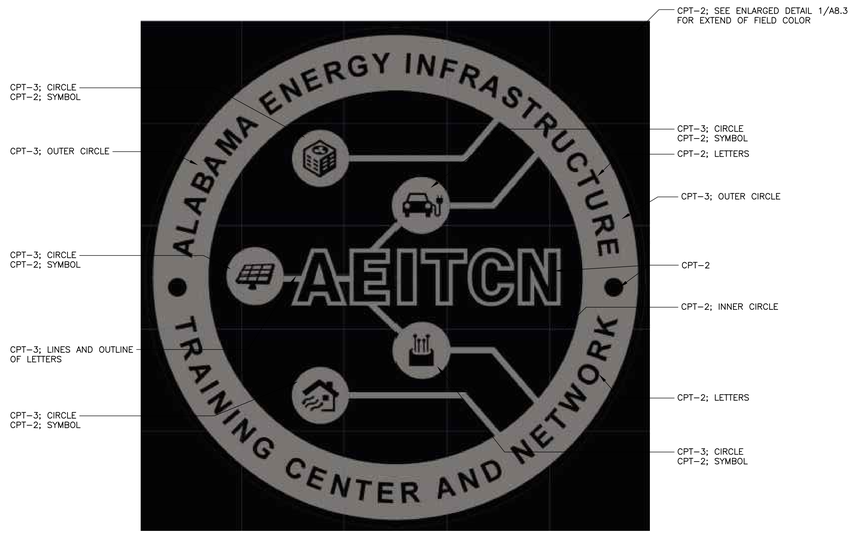
FINISH PATTERN LEGEND

- SC — SEALED CONCRETE
- LVT-1 — LUXURY VINYL TILE
- ERF-1 — EPOXY RESIN FLOOR
- PFT-1 — PORCELAIN FLOOR TILE
- PFT-2 — PORCELAIN FLOOR TILE
- CPT-1 — CARPET
- CPT-2 — CARPET
- 1 CPT — CUSTOM CARPET LOGO

NOTE: CORNER GUARDS, CG, SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS ON ALL GYPSUM WALL CORNERS. SEE FINISH SCHEDULE FOR COLOR SELECTION



1 CUSTOM INSET ENTRY CARPET
SCALE: 1" = 1'-0"
NOTE: ARCHITECT TO PROVIDE CAD/VECTOR FILE



2 CUSTOM INSET ENTRY CARPET
NOT TO SCALE
NOTE: ARCHITECT TO PROVIDE CAD/VECTOR FILE

LATHAN ▪ McKEE ARCHITECTS

ALABAMA ENERGY INFRASTRUCTURE TRAINING CENTER
PACKAGE C: INTERIOR FIT OUT
3711 INDUSTRIAL COURT, JASPER, ALABAMA 35501
BEVILL STATE COMMUNITY COLLEGE

SHEET TITLE: FINISH SCHEDULE AND LEGEND

STATE OF ALABAMA
No. 7007
RYAN D. VERNON
REGISTERED ARCHITECT

PROJ. MGR.:	S. CALMA
DRAWN:	K.JOINER
DATE:	12.19.25
REVISIONS	
1	02.26.26 ADD #2

JOB NO. 24-40C
SHEET NO:
A8.3
22 OF 24
0 1" 2"

MAY 17, 2024

REPORT OF SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING EVALUATION FOR

Expansion of Alabama Power HVAC Training Center Building

Jasper, Alabama

BECC Project Number: 224039

PREPARED FOR:

McGehee Engineering

450 19th Street West

Jasper, AL 35501-3431

GEOTECHNICAL, MATERIALS, AND ENVIRONMENTAL ENGINEERS

360 Industrial Lane, Birmingham, AL 35211 - (205) 941-1119 - www.beccinc.com

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LISA K. MORRISON
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MARTIN T. BURFORD, P.E.
President

RICHARD A. RHINEHART, P.E.
Principal Geotechnical Engineer

May 17, 2024

Mr. Zac McGehee, PE
McGehee Engineering
450 19th Street West
Jasper, AL 35501-3431

Subject: Report of Subsurface Exploration and
Geotechnical Engineering Evaluation for
Expansion to Alabama Power HVAC Training Center Building
Beville State Community College
Jasper, Alabama
BECC Project Number: 224039

Dear Mr. McGehee:

BECC has completed the authorized Subsurface Exploration and Geotechnical Engineering Evaluation for the subject project. This work was conducted in accordance with our proposal number Q1-24052 dated April 25, 2024.

The purpose of our work was to evaluate the soil boring data, provide laboratory testing and provide geotechnical engineering recommendations for the subject project. This report outlines the exploration procedures used, exhibits the data obtained, and presents our conclusions and recommendations.

We sincerely appreciate the opportunity to have worked with you on this project. If you have any questions, or if we may be of further service to you, please call us.

Respectfully submitted,
BECC, Inc.

Richard A. Rhinehart

Richard A. Rhinehart, P.E.
Principal Geotechnical Engineer

EXECUTIVE SUMMARY

The proposed project includes Expansion of Alabama Power HVAC Training Center Building in Jasper, Alabama. The following is a brief summary of the exploration including our findings, conclusion and recommendations. Refer to subsequent sections within the text for detailed discussions of these topics.

- Seventeen (17) soil test borings were drilled for the project to depths ranging from 5 to 21 feet.

- Existing fill soils were typically encountered below the surface materials and extended to maximum depths of 21 feet. The fill soils consisted of soft to very stiff, silty clay with traces of shale fragments. Residual soils were generally encountered by the soil borings where the existing fill was not as thick. The residual soils consisted of medium stiff to very hard silty clay with some shale fragments. Auger refusal (rock) varied from 15 to 21 feet by the deeper borings. Perched groundwater was encountered at depths 3 to 15 feet.

- The soil borings indicated a thick layer of existing fill soils in the proposed building expansion area. The fill consistency was highly variable ranging from soft to stiff. These fill soils had characteristics of fill soils that were not systematically placed in standard lift thicknesses and adequately compacted. In our opinion, conventional shallow spread foundations should not be supported by the existing fill soils due to the risk of excessive settlements.

- Mass undercutting of the existing fill soils and replacement with new controlled engineered fill may not be a practical solution due to the vicinity of the existing building and the thickness of the existing fill. We recommend that the new building foundations either be supported by rammed aggregate piers (RAP) or concrete drilled shafts.

- The RAP would be constructed by augering 24 to 36-inch diameter holes to depths below the existing fill soils in the foundation areas then backfilling the holes with thin lifts of compacted aggregate. Conventional shallow foundations supported on rammed aggregate piers can be designed based on a net allowable bearing pressure of 6 ksf.

- In our opinion, rock supported concrete drilled piers can be used as the foundation system to support the building foundations. With this option, drilled piers would be augured through the existing fill and residual soils to bear on the underlying bedrock. Drilled piers bearing on sound, continuous, shale or sandstone can be designed for a maximum allowable bearing pressure of 40 ksf.

- The existing fill soils can provide floor slab support provided the upper 2 feet are compacted to a minimum of 98% of the Standard Proctor maximum dry density (ASTM D698). After compaction, we also recommend that the existing fill soils be proofrolled with a tandem dump truck (50,000 lbs. min.).

NOTE: *It should be noted that this executive summary presents selected elements of our findings and recommendations* ***only.*** *It* ***does not*** *present crucial details needed for the proper application of our findings and recommendations. Our findings, recommendations, and application are related* ***only through the full report****, and are best evaluated with the active participation of the geotechnical engineer who developed them.*

Table of Contents

SECTION 1: SCOPE OF WORK

The purpose of the geotechnical exploration was to evaluate general soil conditions for the Expansion of Alabama Power HVAC Training Center Building in Jasper, Alabama. This work was conducted in accordance with our proposal number Q1-24052 dated April 25, 2024. The following items were included as part of our scope of work:

1. Review the 17 field soil test boring log data drilled by Earthcore, LLC for the project. Prepare final boring logs after BECC's engineer observe and visually classify the soil samples furnished by Earthcore.

2. Laboratory tests were to include natural moisture content (ASTM D2216), Atterberg limits (ASTM D4318), and percent passing #200 sieve (ASTM D1140).

3. A geotechnical engineering report is to include the following items:

   - A site plan illustrating the boring locations
   - Boring logs describing the subsurface and groundwater conditions encountered
   - Description of field and laboratory procedures
   - Discussion of site geology
   - Results of the laboratory tests performed
   - Presence of unsuitable soils within the borings
   - Recommendations for undercutting unsuitable soils
   - General earthwork recommendations (compaction)
   - Recommendations for foundation design alternatives
   - Floor slab recommendations

NOTE: BECC's work scope did not include exploration of the site for environmental contaminants in the subsurface soils or groundwater.

SECTION 2: PROJECT DESCRIPTION

The project will consist of an addition to the Alabama Power HVAC training center building located at 3711 Industrial Court in Jasper, Alabama. The addition will be located on the south side of the existing building. The new addition will be approximately 160' wide and 320' long. We understand that the existing building had foundation problems after construction that were subsequently repaired.

SECTION 3: SITE GEOLOGY

Published geologic maps by the United States Geological Survey (USGS) indicate that the site is underlain by the Pottsville Formation. The Pottsville Formation consists of a thick-bedded coarse-grained, locally conglomeratic basal sandstone overlain by a sequence of alternating thin-bedded shale containing coal beds and thin to medium bedded sandstone.

The sandstone weathers to a sandy soil that has a maximum thickness of about 15 feet. The rocks in this geologic formation are prone to differential weathering, which often results in alternating hard and soft layers. The shales are more susceptible to weathering, and advanced weathering results in the formation of silt and clay strata. In many instances, competent layers of sandstone can be found to be underlain by clay seams derived from weathered shale beds. Wet springs and perennial springs are common in the Pottsville Formation.

In addition to differential weathering, the rocks of the Pottsville Formation have been subjected to intense folding, often creating tightly folded beds. Due to intense folding, the rock beds are sometimes folded to a near vertical orientation, which permits water infiltration and advanced weathering. Anomalous localized conditions are frequently associated with the differential folding which is common to the Pottsville Formation. Previous surfacing mining and underground mining for coal has occurred in the Pottsville Formation. The evaluation of the site for coal mining was not part of BECC's work scope.

3.1 Seismic Design Parameters

The subsurface conditions at the site consist of soils underlain by bedrock. Based on the borings performed at the site and our knowledge of the site geology, we recommend a seismic site class definition of "C" be used in design calculations (International Building Code). Site Coefficient values for spectral response acceleration taken from ASCE 7 Hazard Tool are given below.

Site Location: 33.816671° N Latitude -87.230079° W Longitude

Risk Category	S_S	S_1	S_{MS}	S_{M1}	S_{DS}	S_{D1}	PGA_M	T_L	V_{S30}
II	0.37	0.13	0.37	0.17	0.25	0.11	0.18	12	530

SECTION 4: SUBSURFACE EXPLORATION

BECC received 17 field soil test boring logs and soil samples drilled by Earthcore. The boring locations are shown on the "Boring Location Map" in the Appendix. The soil samples were visually classified by BECC's engineers and the data is presented on the boring logs in the Appendix.

SECTION 5: LABORATORY TESTS

A laboratory testing program was conducted to ascertain additional engineering characteristics of the soils obtaineed. To supplement the visual classification of the soil samples, the following tests were performed. The results of the tests are shown on the "Geotechnical Lab Summary" and the individual test reports in the Appendix.

5.1 Description of Soils (Visual-Manual Procedure) (ASTM D2488)

The soil samples were visually examined by our engineer and soil descriptions were provided. Representative samples were then selected and tested to determine soil classification as described above. This data was used to correlate our visual descriptions with the Unified Soil Classification.

5.2 Natural Moisture Content (ASTM D2216)

Natural moisture contents (M%) were determined on selected samples. The natural moisture content is the ratio, expressed as a percentage, of the weight of water in a given amount of soil to the weight of solid particles.

5.3 Percent Passing #200 Sieve (ASTM D1140)

Wash #200 tests were performed on the selected samples to determine the amount of "fines" in the represented soil. "Fines" are defined as particles with a grain size equal to or less than a diameter of 0.075 millimeters. These particles are typically found in silts, clays, and silty clays, as well as silty or clayey sands.

5.4 Atterberg Limits Tests (ASTM D4318)

Atterberg Limit tests were performed on the selected samples to determine how the characteristics change upon variation in moisture stage. The limits are bracketed by the Liquid Limit (LL) and the Plastic Limit (PL). The Liquid Limit is the moisture content at which the soil will flow as a heavy viscous fluid. The plastic Limit is the moisture content at which the soil is between the "plastic" and semi-solid stage. The soil's Plasticity Index (PI) is the difference between the Liquid Limit and the Plastic Limit. The PI is often used as the indicator of the soil's expansive tendencies. The greater the range between the LL and the PL, the more potentially expansive the soil can be.

SECTION 6: SURFACE & SUBSURFACE CONDITIONS

Details of the subsurface conditions encountered by the test borings are shown on the boring log sheets in the Appendix. The general subsurface conditions encountered, and their pertinent characteristics are described in the following paragraphs. The stratification lines indicated on the logs of boring represent approximate boundaries between soil types. However, the actual transition may be gradual. Conditions represented by the test borings should be considered applicable only at the test boring locations on the dates shown, and it should be assumed that the conditions may be different at other locations at other times.

6.1 Surface Materials

The soil borings typically encountered asphalt paving or topsoil at the ground surface. The asphalt was typically 2 inches thick underlain by approximately 2 inches of crushed stone base. When encountered the topsoil was approximately 1 inch thick.

6.2 Existing Fill Soils

Existing fill soils were typically encountered below the surface materials and extended to maximum depths of 21 feet. The fill soils consisted of soft to very stiff, silty clay with traces of shale fragments. Standard penetration resistance tests (N-values) within the fill soils ranged from 1 and 26 blows per foot (bpf) typically less than 10 bpf. The following table indicates the existing the approximate fill thickness at the boring locations.

BORING NO.	EXISTING FILL THICKNESS (feet)
B-2	3
B-3	5+
B-4	5+
B-6	5+
B-7	5+
B-8	5+
B-9	21
B-10	13
B-11	17
B-11b	8
B-12	20
B-13	16
B-14	16
B-15	20

6.3 Residual Soils

Residual soils are those that have been formed by the in-place weathering of the parent rock. Residual soils were generally encountered by the soil borings where the existing fill was not as thick. The residual soils consisted of medium stiff to very hard silty clay with some shale fragments. N-values in the residual soils ranged from 7 to 100 bpf.

6.4 Auger Refusal

Auger refusal is the drilling depth at which advancement of the borehole can no longer be accomplished by standard soil drilling; rock coring must be employed for further penetration. Auger refusal encountered in the borings ranged from 15 to 21 feet. Auger refusal at the site was due to resistant shale/sandstone bedrock.

BORING NO.	AUGER REFUSAL DEPTH (feet)
B-9	21
B-10	21
B-11	17
B-11b	17
B-12	20
B-12b	15
B-13	16
B-14	16
B-15	20

6.5 Groundwater

Groundwater was encountered in the following soil borings at the time of drilling. The groundwater levels are not true groundwater levels, but rather "perched" or trapped groundwater in the existing fill soils and or just above the rock surface.

BORING NO.	GROUNDWATER DEPTH AT TIME OF DRILLING (feet)
B-3	3
B-9	8
B-10	11
B-11	10
B-12	12
B-13	11
B-14	8
B-15	15

The presence or absence of water in the boreholes at the time of drilling does not necessarily mean that groundwater will or will not be present at other times. Groundwater levels fluctuate seasonally and are related to the amount of rainfall received in months prior to the observations. Perched or trapped groundwater is common near the interfaces of fill/residual soils/rock.

SECTION 7: FOUNDATION RECOMMENDATIONS

7.1 General

The soil borings indicated a thick layer of existing fill soils in the proposed building expansion area. The fill consistency was highly variable ranging from soft to stiff. These fill soils had characteristics of fill soils that were not systematically placed in standard lift thicknesses and adequately compacted. In our opinion, conventional shallow spread foundations should not be supported by the existing fill soils due to the risk of excessive settlements.

Mass undercutting of the existing fill soils and replacement with new controlled engineered fill may not be a practical solution due to the vicinity of the existing building and the thickness of the existing fill. We recommend that the new building foundations either be supported by rammed aggregate piers or concrete drilled shafts. The details are presented in the following sections.

7.2 Rammed Aggregate Piers

The proposed structure can be supported by strip and spread footings resting on existing soil provided it is reinforced by Rammed Aggregate Piers (RAP). Such a foundation system is typically a design-build product performed by a specialty contractor such as Geopier. The piers are constructed by augering 24 to 36-inch diameter holes to extend below the existing fill soils then backfilling the holes with thin lifts of compacted aggregate. Compaction densifies the aggregate and increases lateral stress in the soil matrix. The system serves to reduce settlement by replacing the random fill (compressible) soils with a stiffer composite soil matrix.

RAP elements typically cover approximately 30 percent of the footing footprint area. We recommend a net allowable bearing pressure for shallow spread footings of 6 ksf at the project site where RAP elements are used to modify the subsurface conditions. This bearing pressure can be increased one-third, in accordance with the local practice to resist transient loading conditions associated with seismic events or wind gusts.

Final settlement calculations should be performed by RAP designer. Typically total and differential settlements foundation settlements are estimated to be 1 inch and ½ inch, respectively.

7.2.1 RAP Installation Recommendations

If an aggregate pier-enhanced foundation system is selected, we recommend that the following issues be considered prior to construction.

- The RAP foundation design and construction should be performed by a specialty contractor that has a proven track record of successful projects using RAPs.

- The existing fill may contain isolated rock or boulders which the auger used to install the rammed aggregate piers cannot remove. Other methods of boulder removal may be required before the pier installation (typically trackhoe used for removal of isolated boulders).

- One demonstration pier should be installed with the Contractor's standard procedures and then load-tested to determine the modulus. The load testing setup and procedures should be selected by the Specialty RAP Contractor and submitted for review to the project geotechnical engineer. The demonstration pier should be installed at the foundation grade level.

- All of the RAP element's installation operations should be conducted under the observation of the geotechnical engineer's representative.

- After the foundation soils have been reinforced with RAP elements, the treated ground surface should be cleared and cleaned to the satisfaction of the geotechnical engineer, and shallow foundations installed at grade.

7.3 CONCRETE DRILLED PIERS

In our opinion, rock supported concrete drilled piers can be used as the foundation system to support the building foundations. With this option, drilled piers would be augured through the existing fill and residual soils to bear on the underlying bedrock. **Drilled piers bearing on sound, continuous, shale or sandstone can be designed for a maximum allowable bearing pressure of 40 ksf.** The use of this bearing pressure will require that sound, level, continuous rock is present beneath the bearing surface of the drilled pier. Based on our drilling data, it appears that a typical drilled pier would encounter rock at approximately 15 to 25 feet below the existing ground surface. Some rock excavation should be expected in order to remove weathered/fractured upper rock to reach competent bearing rock.

The minimum drilled pier diameter should be 30 inches so that adequate cleaning and inspection can be accomplished. Each drilled pier should be inspected by a qualified geotechnical engineer by examining the bottom of the shaft to check for rock soundness and the presence of voids and seams. Given the rock condition at the site, we anticipate that at least one test hole (2" diameter) drilled to at least twice the drilled pier diameter will be required to adequately evaluate the condition of the bearing rock. The drilling of any additional test holes should be left to the discretion of the inspecting engineer.

7.3.1 Dewatering

The results of our exploration and experience in this geologic formation indicate that dewatering will be required during drilled pier installation. The anticipated drilled pier depth, and the potential for high groundwater flows make it mandatory that the contractor be prepared to adequately seat casing and have facilities available to handle water in-flows.

We note that it is sometimes necessary to extend drilled piers through what may be adequate bearing material simply to restrict the flow of water into the drilled pier. Occasionally vertical slots are present in the bedrock resulting in significant inflows of water. In this case, measures will be required to control soil inflow and removal during dewatering to minimize potential for future ground loss.

7.3.2 Concrete Placement

The procedures used in installing drilled piers and the nature of the materials penetrated in this area are such that concrete quantities can be underestimated if the "neat" volume of the design drilled pier diameter is used. The drilled pier is typically started one or more sizes larger than design to facilitate "telescoping" as the drilled pier is extended. Since the depth of suitable rock is never a certainty, the contractor must be conservative in terms of being able to be at or above the design diameter when suitable rock is reached.

Another factor contributing to apparent "overages" in concrete quantities is the creation of voids adjacent to the drilled pier due to removal of boulders or the occurrence of actual cavities within the rock or soil mass. These two factors normally result in the actual quantity of concrete used exceeding the "neat" volume of the design shaft by as much as 30% to 70%. These percentages obviously would tend to increase as the depth of the drilled pier increases. For this project we recommend that an allowance for at least 50% overage be included in the foundation concrete budget.

Our experience and current research in the field indicate that drilled piers can be concreted by the "free fall" method without affecting the strength and quality of the in-place concrete. Concrete should free fall without hitting the sides of the excavation or reinforcing. The use of a hopper or other suitable device is recommended to control concrete placement. The placement of concrete in the shaft should proceed until the concrete level is above the external fluid level and should be maintained above this level throughout casing removal.

SECTION 8: FLOOR SLAB CONSIDERATIONS

The existing fill soils can provide floor slab support provided the upper 2 feet are compacted to a minimum of 98% of the Standard Proctor maximum dry density (ASTM D698). After compaction, we also recommend that the existing fill soils be proofrolled with a tandem dump truck (50,000 lbs. min.). Any soft or unstable areas will require undercutting or recompaction. The geotechnical engineer should observe the proofrolling operations and perform density tests on the recompacted fill soils.

In addition, we suggest that all ground supported slabs be founded on a minimum of 4 inches of open graded stone (#57) or 6 inches of dense graded stone (#610 or #825). The granular materials will provide more uniform support, and to act as a capillary break. We recommend that slab joints and construction be in accordance with the guidelines of the American Concrete Institute (ACI) and the Portland Cement Association (PCA).

On most projects, there is a significant time lag between initial grading and the time when the contractor is ready to construct the slab-on-grade. Even though the soils may have been placed and compacted adequately during initial grading, exposure to weather, construction traffic, etc., can destroy the integrity of subgrade soils. On many projects, this becomes a point of controversy when remedial work is required for proper slab support.

SECTION 9: SITE PREPARATION & GRADING CONSIDERATIONS

9.1 Existing Structures and Surface Materials

All existing structures (including above and below ground construction) within the areas to be developed should be removed. Removal should include all vegetation, topsoil, underground pipes and lines, etc., that might interfere with construction. If abandoned underground utilities are to be removed prior to initiation of construction, provisions should be made in the construction specifications and budget to restore the subgrade to a stable condition. Restoration should include backfilling and compaction of the excavated areas.

9.2 Controlled Structural Fill

BECC recommends that the controlled structural fill meet the following recommendations:

- Liquid Limit (LL) less than 50.
- Plasticity Index (PI) less than 30.
- Free of organics.
- Maximum rock size of 4 inches.
- Maximum dry density greater than 100 pcf.

General fill should be compacted to a minimum 98% of maximum dry density, as determined by the Modified Proctor ASTM D1557. A sufficient number of field density tests should be performed to evaluate the grading contractor's performance during filling. During mass grading, lift thickness for fill should be limited to a maximum of 8 inches loose measure. Backfilling in limited access areas such as utility trenches should have a lift thickness limited to 4 inches loose measure.

We also recommend that all structural fill be placed within 2% of optimum moisture content. The grading contractor should acknowledge the importance of proper fill moisture conditioning. **We suggest that the project specifications address that both fill compaction and acceptable fill moisture content will be required for the acceptance of structural fills.** It will be particularly important to have a water truck available if filling takes place during dry months.

9.3 Backfilling of Utility Trenches

Backfilling of storm drain and utility trenches is often accomplished in an uncontrolled manner, leading to subsequent settlement of the fill and cracking of pavements. We recommend that utility trenches be backfilled with acceptable fill in 4-inch lifts and compacted with pneumatic-piston tampers to the project requirements. Should seepage occur in utility trenches, it may be necessary to "floor" the trench with open-graded gravel to provide a dry working surface.

9.4 Surface Drainage and Protection of Soils During Grading

The soils at the site are moisture sensitive and can become easily disturbed causing loss of strength. Proper surface drainage will be very important during grading at the site. If the soils become wet after being exposed it may become necessary to undercut or recondition. On many projects reworking of disturbed soils becomes a point of controversy. We recommend that the specifications for this project provide performance guidelines for protection of exposed soils and correction of disturbed areas.

SECTION 10: CONSTRUCTION MONITORING

We strongly recommend that **BECC, Inc.** be retained to provide a comprehensive construction-monitoring program when the project proceeds. This program would assist the owner in determining that the work is being carried out in general conformance with the plans and specifications and help avoid the potential of change orders and cost overruns. Construction monitoring includes testing of construction materials such as compacted fill, asphalt, and concrete. Also included is engineering observation during the site preparation, paving phases of the project.

Monitoring/testing during the earthwork and foundation construction phases is particularly important since assumptions (and recommendations) have been made based on the soil boring data. Confirmation that actual subsurface conditions are comparable to the assumed conditions is an essential part of the subsurface exploration process.

10.1 Subgrade, Observations, Proofrolling

The purpose of proofrolling will be to densify the exposed near-surface soils and also to reveal soft pockets of soil that will require remedial measures. Areas that pump or rut during the proofrolling operations should be undercut or reconditioned. The geotechnical engineer can determine the depth and extent of areas that will require undercutting.

10.2 Fill Monitoring

We recommend that in-place density tests should be performed in the field by an engineering technician to evaluate the contractor's performance regarding meeting the project specifications for fill placement. A commonly used testing frequency is one (1) test per eight (8) inch lift to fill placed per 2,500 square feet of fill area. The engineering technician can assist the grading contractor in soil moisture content evaluation by performing on-site fill moisture content tests.

10.3 Placement of Pavement & Dense-Grade Base

BECC recommends the placement of dense-grade aggregate base and concrete pavement be observed and tested by a BECC geotechnical engineer or engineering technician. Such testing is necessary to determine the appropriateness of the compaction of aggregate material and compressive and/or flexural strength of concrete material to conform to the specifications and recommendations set forth herein.

10.4 Foundation Excavations

It is recommended that the excavations for foundations be observed and tested by a BECC representative. Such testing is necessary to determine the appropriateness of the bearing level, the adequacy of the bearing materials and the conformity of the foundation to the specification with respect to depth, planned dimensions, cleanliness, etc.

SECTION 11: GENERAL REMARKS/REPORT LIMITATIONS

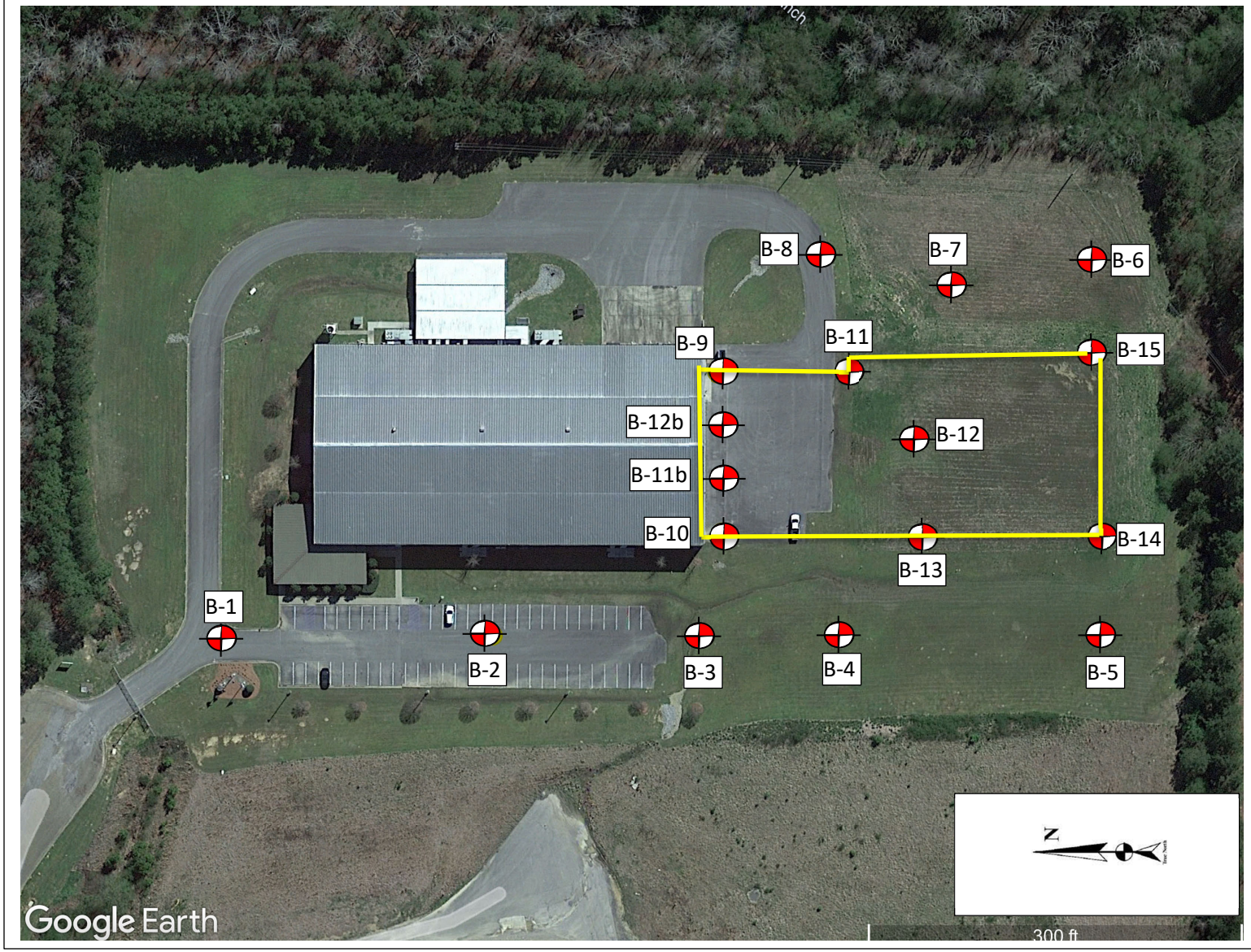
This report has been prepared for the exclusive use of **McGehee Engineering** for specific application to the subject project. All recommendations contained in this report have been made in accordance with generally accepted soil and pavement engineering practices. No other warranties are implied or expressed. In addition, the analysis and recommendations submitted in this report are based in part upon the data obtained from the test locations. The nature and extent of variations between the test locations may not become evident until construction. If variations then appear evident, it may be necessary to re-evaluate the recommendations of this report.

We emphasize that this report was for design purposes only and is not sufficient to prepare an accurate bid. Contractors reviewing this report should acknowledge that the recommendations and discussions herein are for design purposes.

If significant changes are made in the character of the proposed development, a consultation should be arranged to review them with respect to prevailing subsurface conditions. At that time, it may be necessary to submit supplementary recommendations.

It is imperative that the geotechnical engineer be provided the opportunity to review the final plans and specifications to verify that the recommendations in this report are properly interpreted and incorporated in the design. It will be the client's responsibility to furnish the final grading and foundation plans to BECC for the necessary review. If the geotechnical engineer is not accorded the privilege of making this recommended review, we can assume no responsibility for misinterpretation of our recommendations.

APPENDIX



BECC
Birmingham Engineering and Construction Consultants, Inc.
Geotechnical, Materials, and Environmental Engineers

Expansion of Alabama Power HVAC Training Center Building

Jasper, Alabama

Boring Location Map

Scale: 1 in = 125 ft

BECC Project No: 224039

Date: 5/16/2024

Map Sheet

Sheet 1 of 1

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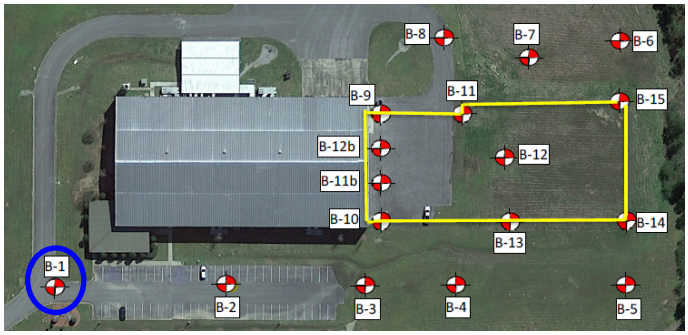

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-1
BORING LOCATION 33.818196, -87.230960
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 351.61'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value (0 50 100)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
350	0			RESIDUAL: Very Stiff, Tan, Very Silty, CLAY; trace of shale fragments		S-1		4-10-16 (26)							Surface elevation estimated from Google Earth.
						S-2		50/2" (100)							No groundwater encountered at time of drilling.
345	5			5.0 Boring Terminated at 5'											
340	10														
335	15														
330	20														

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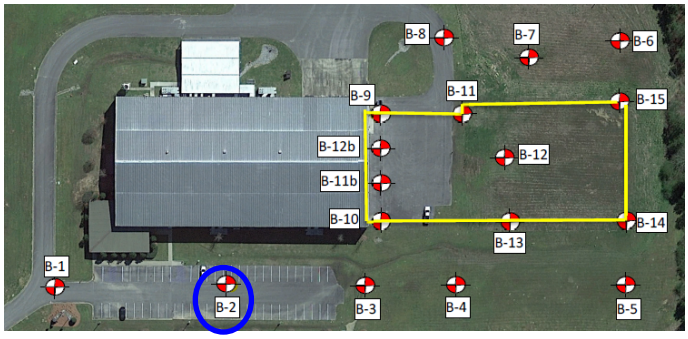

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-2
BORING LOCATION 33.817621, -87.230948
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 347.70'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
345				FILL: Very Stiff, Gray-Tan, Silty CLAY; trace of shale fragments		S-1		4-8-12 (20)		10.3					Surface elevation estimated from Google Earth.
				3.0											
	5		CL	RESIDUAL: Very Stiff, Tan-Gray, Very Silty, CLAY; trace of shale fragments		S-2		8-10-15 (25)		14.9	35	12	84		No groundwater encountered at time of drilling.
340				5.0											
				Boring Terminated at 5'											
	10														
335															
	15														
330															
	20														
325															

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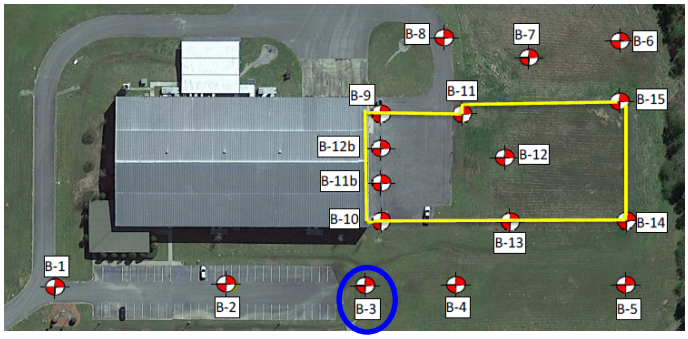

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-3
BORING LOCATION 33.817158, -87.230936
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 346.00'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
345				FILL: Soft to Medium Stiff, Gray-Brown, Silty CLAY; trace of shale fragments; very moist		S-1		WOH-2-2 (4)						▽	Surface elevation estimated from Google Earth.
															Groundwater encountered at 3' at time of drilling.
						S-2		4-3-2 (5)							
340	5			5.0											
				Boring Terminated at 5'											
335	10														
330	15														
325	20														

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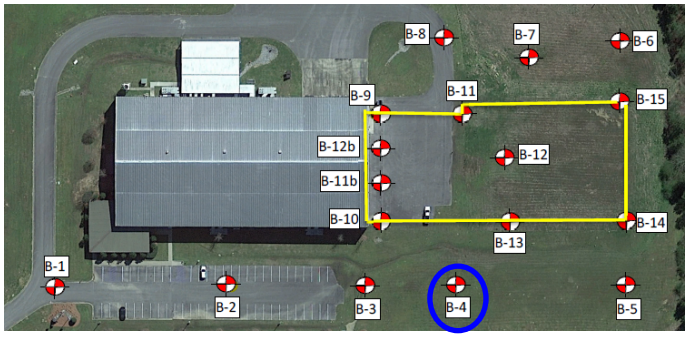

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-4
BORING LOCATION 33.816828, -87.230926
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 345.94'
METHOD Auger **HOLE DIAMETER** 6
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
345	0			FILL: Very Stiff to Stiff, Gray-Brown, Silty CLAY; trace of shale fragments		S-1		4-8-10 (18)							Surface elevation estimated from Google Earth.
															No groundwater encountered at time of drilling.
						S-2		5-7-7 (14)							
340	5			5.0											
				Boring Terminated at 5'											
335	10														
330	15														
325	20														

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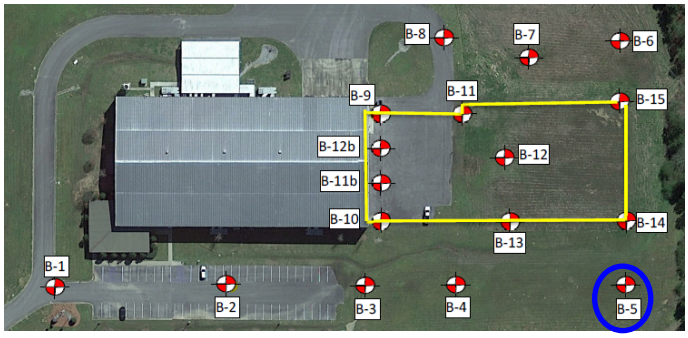

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-5
BORING LOCATION 33.816231, -87.230907
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 344.24'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
340	0			RESIDUAL: Very Stiff, Tan, Very Silty, CLAY		S-1		8-9-10 (19)							Surface elevation estimated from Google Earth.
	5			5.0		S-2		10-12-15 (27)							No groundwater encountered at time of drilling.
335				Boring Terminated at 5'											
	10														
330															
	15														
325															
	20														
320															

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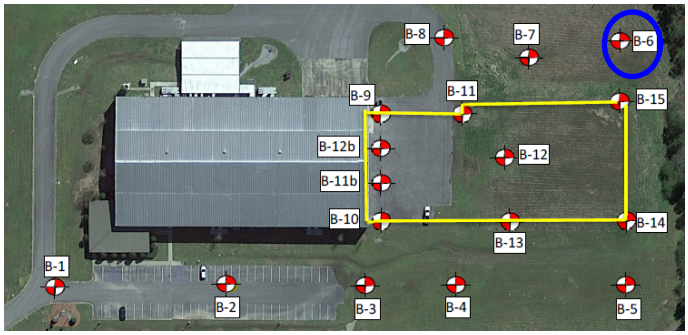

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-6
BORING LOCATION 33.816264, -87.229866
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 337.99'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
335	0			FILL: Stiff to Medium Stiff, Gray-Reddish Brown, Silty CLAY; trace of shale fragments		S-1		2-4-5 (9)							Surface elevation estimated from Google Earth.
															No groundwater encountered at time of drilling.
						S-2		3-4-4 (8)							
	5			5.0											
330				Boring Terminated at 5'											
	10														
325															
	15														
320															
	20														
315															

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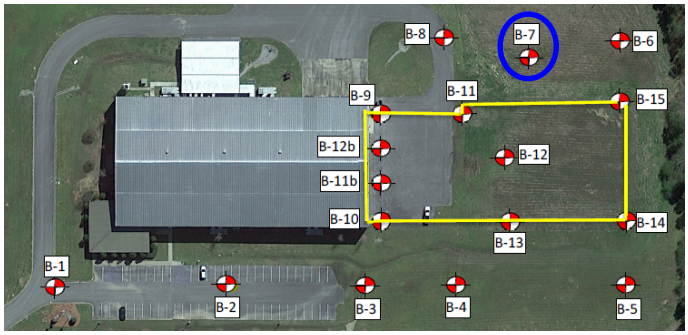

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-7
BORING LOCATION 33.816588, -87.229931
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 339.93'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
335	0			FILL: Medium Stiff, Gray-Brown, Silty CLAY; trace of shale fragments		S-1		3-3-4 (7)							Surface elevation estimated from Google Earth.
															No groundwater encountered at time of drilling.
						S-2		4-3-4 (7)							
	5			5.0 Boring Terminated at 5'											
330	10														
325	15														
320	20														
315															

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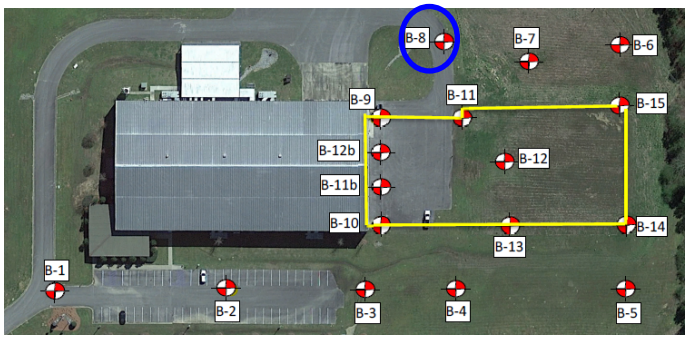

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-8
BORING LOCATION 33.816835, -87.229849
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 340.23'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
340	0			FILL: Medium Stiff, Brown-Gray, Silty CLAY; trace of shale fragments; very moist		S-1		3-3-2 (5)	▲						Surface elevation estimated from Google Earth.
				3.0											
				RESIDUAL: Stiff, Brownish Red-Gray, Silty, CLAY		S-2		4-4-5 (9)	▲						No groundwater encountered at time of drilling.
335	5			5.0											
				Boring Terminated at 5'											
330	10														
325	15														
320	20														

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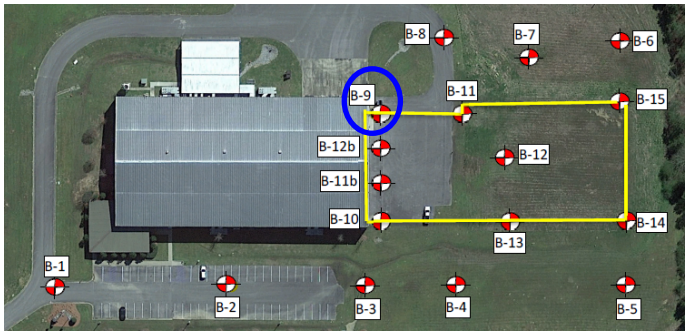

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-9
BORING LOCATION 33.817125, -87.230206
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 347.52'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
345	0			Asphalt (2") Crushed Stone Base (2") 0.3											Surface elevation estimated from Google Earth.
				FILL: Medium Stiff to Soft, Gray-Tan-Red, Silty CLAY; trace of shale fragments		S-1		3-3-3 (6)							
						S-2		3-3-4 (7)							
340	5					S-3		3-2-3 (5)						▽	Groundwater encountered at 8' at time of drilling.
				Wet from 8' to 21'		S-4		3-3-3 (6)							
335	10					S-5		WOH-2-1 (3)							
330	15														
325	20					S-6		9-50/3" (100)							
				21.0 Auger Refusal at 21'											

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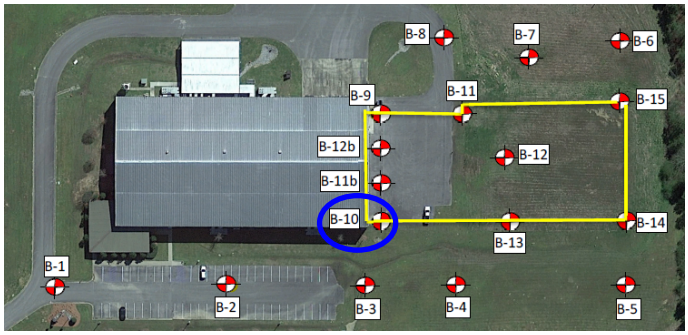

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-10
BORING LOCATION 33.817100, -87.230644
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 346.35'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
345	0		CL	Asphalt (2") Crushed Stone Base (2") 0.3 FILL: Soft, Gray-Brown, Silty CLAY; wet		S-1		WOH-WOH-1 (1)		18.1	30	9			Surface elevation estimated from Google Earth.
						S-2		3-2-2 (4)		15.0					
340	5					S-3		2-3-2 (5)		15.3					
						S-4		WOH-2-2 (4)		17.5					
335	10													▽	Groundwater encountered at 11' at time of drilling.
				13.0 RESIDUAL: Hard, Brown-Gray, Silty CLAY		S-5		11-18-24 (42)		12.2					
330	15			19.0											
				RESIDUAL: Very Hard, Brown-Gray, Silty CLAY with shale fragments 21.0		S-6		50/3" (100)							
325	20			Auger Refusal at 21'											

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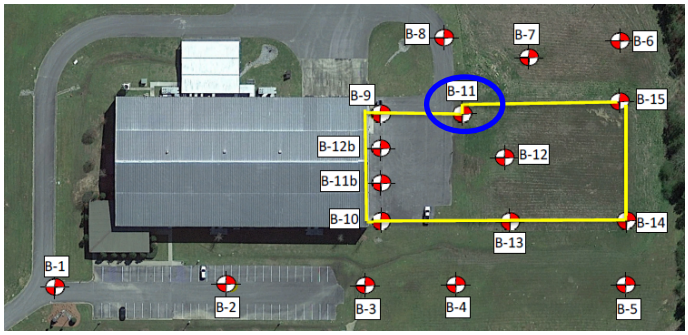

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-11
BORING LOCATION 33.816821, -87.230190
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 346.81'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
345	0			Topsoil (1") 0.1											Surface elevation estimated from Google Earth.
				FILL: Very Stiff to Stiff, Brown-Gray, Silty CLAY; trace of shale fragments		S-1		6-7-10 (17)							
						S-2		5-7-7 (14)							
340	5					S-3		4-5-6 (11)							
				8.0											
				FILL: Medium Stiff, Brown-Gray-Red, Silty CLAY; trace of shale fragments		S-4		3-2-3 (5)							Groundwater encountered at 10' at time of drilling.
335	10			12.0											
				FILL: Medium Stiff, Gray, Silty CLAY; trace of organics; wet		S-5		2-3-3 (6)							
330	15			17.0											
				Auger Refusal at 17'											
325	20														

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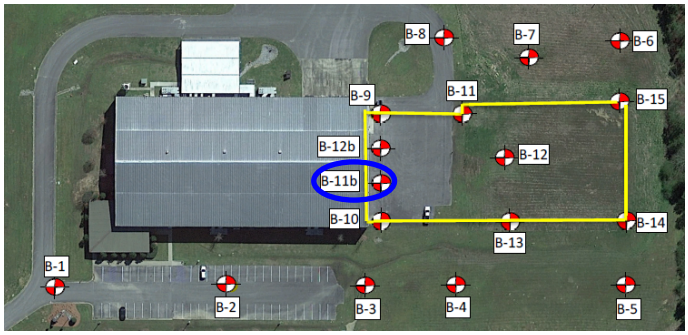

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-11b
BORING LOCATION 33.817110, -87.230499
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 346.76'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
345				Asphalt (2") Crushed Aggregate Base (2") 0.3											Surface elevation estimated from Google Earth.
				FILL: Stiff to Medium Stiff, Gray-Tan, Silty CLAY; trace of shale fragments		S-1		4-6-7 (13)							
						S-2		6-5-4 (9)							
340	5					S-3		4-3-4 (7)							No groundwater encountered at time of drilling.
				8.0											
				RESIDUAL: Medium Stiff, Tan-Gray, Silty CLAY		S-4		3-3-4 (7)							
335	10			13.0											
				RESIDUAL: Very Hard, Brown-Gray Shale fragments		S-5		50/4" (100)							
330	15			17.0											
				Auger Refusal at 17'											
325	20														

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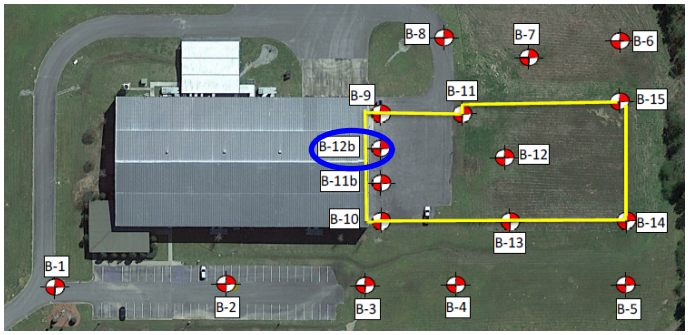

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-12b
BORING LOCATION 33.817115, -87.230340
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 347.40'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
345	0			Asphalt (2") Crushed Stone Base (2") 0.3		S-1		5-2-1 (3)							Surface elevation estimated from Google Earth.
				FILL: Soft to Stiff, Gray-Tan, Silty CLAY; trace of shale fragments		S-2		2-4-6 (10)							
340	5			6.0 RESIDUAL: Stiff, Tan-Gray, Silty CLAY		S-3		6-7-8 (15)							No groundwater encountered at time of drilling.
						S-4		6-7-8 (15)							
335	10														
						S-5		50/3" (100)							
330	15			15.0 Auger Refusal at 15'											
325	20														

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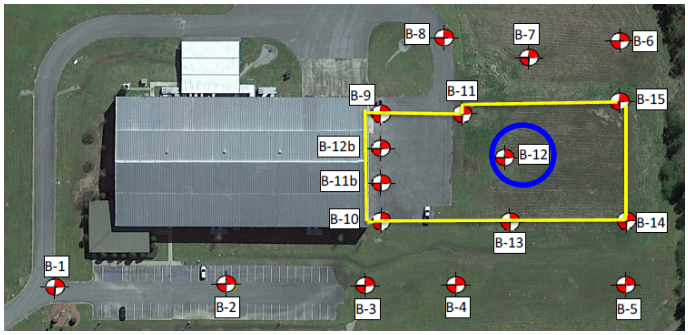

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-12
BORING LOCATION 33.816671, -87.230379
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** N/A
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
	0			Topsoil (1") 0.1											Surface elevation estimated from Google Earth.
				FILL: Very Stiff, Gray-Brown, Silty CLAY; trace of shale fragments 3.0	X	S-1		8-12-15 (27)							
	5			FILL: Medium Stiff to Stiff, Gray-Brown, Silty CLAY; trace of shale fragments	X	S-2		4-3-2 (5)							
					X	S-3		4-4-5 (9)							
	10				X	S-4		4-5-6 (11)							
				12.0										▽	Groundwater encountered at 12' at time of drilling.
				FILL: Soft, Gray-Brown, Silty CLAY; trace of shale fragments; wet	X	S-5		2-1-2 (3)							
	15														
					X	S-6		50/1" (100)							
	20			20.0 Auger Refusal at 20'											

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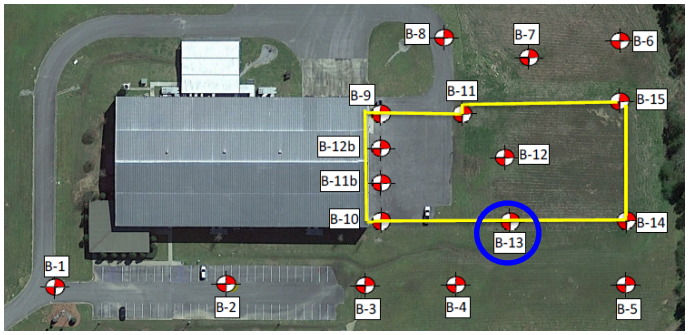

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-13
BORING LOCATION 33.816649, -87.230644
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 346.45'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
345	0			FILL: Stiff, Brown-Gray, Silty CLAY; trace of shale fragments											
						S-1		5-5-7 (12)							
						S-2		5-7-9 (16)							
340	5					S-3		5-5-8 (13)							
						S-4		4-7-5 (12)							
335	10													▽	Groundwater encountered at 11' at time of drilling.
						S-5		3-50/3" (100)							
330	15			16.0 Auger Refusal at 16'											
	20														
325															

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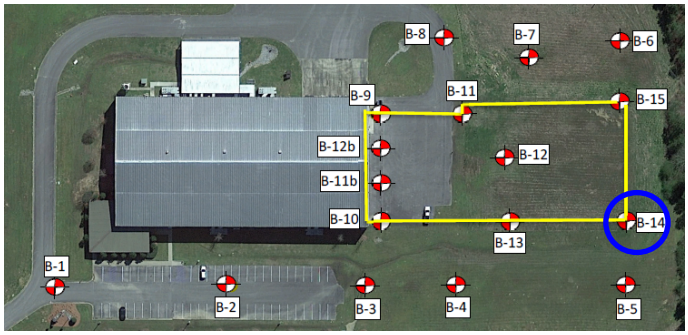

PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-14
BORING LOCATION 33.816232, -87.230630
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 343.69'
METHOD Auger **HOLE DIAMETER** 6
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
340	0			Topsoil (1") 0.1											Surface elevation estimated from Google Earth.
				FILL: Stiff, Brown-Gray, Silty CLAY with shale fragments		S-1		4-5-5 (10)							
						S-2		10-6-5 (11)							
	5			5.0											
335				FILL: Very Stiff, Brown-Gray, Silty CLAY with shale fragments; wet		S-3		10-10-12 (22)						▽	Groundwater encountered at 8' at time of drilling.
						S-4		10-12-14 (26)							
330	10														
						S-5		50/3" (100)							
	15			16.0											
325				Auger Refusal at 16'											
	20														
320															

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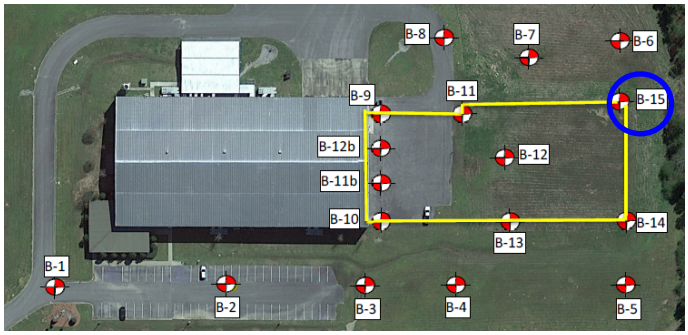


PROJECT Expansion of AL Power HVAC Training Center Building
BECC PROJECT NO. 224039 **BORING NUMBER** B-15
BORING LOCATION 33.816258, -87.230124
GEOLOGY Pottsville Formation
DATE COMPLETED 04/18/2024 **GROUND ELEVATION** 342.14'
METHOD Auger **HOLE DIAMETER** 6"
CONTRACTOR EARTHCORE
LOGGED BY EARTHCORE **REVIEWED BY** R. Rhinehart

ELEVATION (ft)	DEPTH (ft)	Graphic Log	UNIFIED CLASSIFICATION	MATERIAL DESCRIPTION	SAMPLE TYPE	NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	SPT N Value 0 50 100	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	FINES CONTENT (%)	WATER LEVEL	Remarks
				Topsoil (1") 0.1											
340				FILL: Stiff, Brown-Gray, Silty CLAY; some shale fragments		S-1		3-3-5 (8)		11.0					Surface elevation estimated from Google Earth.
	5					S-2		5-8-7 (15)		5.4					
335						S-3		4-6-7 (13)		6.8					
			CL			S-4		3-3-8 (11)		13.6	29	8	86		
	10														
330						S-5		3-3-5 (8)		24.7				▽	Groundwater encountered at 15' at time of drilling.
	15														
325															
				20.0		S-6		50/1" (100)		15.3					
	20			Auger Refusal at 20'											
320															

GEOTECHNICAL LAB SUMMARY

Project Name: Expansion of Alabama Power HVAC Training Center Building
BECC Project No.: 224039

Boring	Sample		Percent Passing #200 Sieve	Moisture (%)	Atterberg Limits			Unified Soil Classification
	Start Depth (ft.)	End Depth (ft.)			LL	PL	PI	
B-2	1.0	2.5		10.3			---	---
B-2	3.5	5.0	84.0	14.9	35	23	12	CL
B-10	1.0	2.5		18.1			---	---
B-10	3.5	5.0	63.1	15.0	30	21	9	CL
B-10	6.0	7.5		15.3			---	---
B-10	8.5	10.0		17.5			---	---
B-10	13.5	15.0		12.2			---	---
B-15	1.0	2.5		11.0			---	---
B-15	3.5	5.0		5.4			---	---
B-15	6.0	7.5		6.8			---	---
B-15	8.5	10.0	86.0	13.6	29	21	8	CL
B-15	13.5	15.0		24.7			---	---
B-15	18.5	20.0		15.3			---	---

STONE

Alabama Energy Infrastructure
Training Center
Submittal Review

SUBMITTAL NUMBER: 03166-001

DESCRIPTION: Aggregate Piers - Shop Drawings

Contractor Review Stone Building LLC	**STONE BUILDING, LLC** This submittal is reviewed for conformance with plans and specifications. Submittal review does not relieve subcontractor/supplier of meeting contract requirements. ■ REVIEWED FOR CONFORMANCE ☐ RELEASED WITHOUT COMMENTS ☐ RELEASED PER COMMENTS ☐ NOT RELEASED ☐ REVISE AND RESUMIT BY GE DATE 8/13/25 SUBMITTAL # 03166-001
Architect Review Lathan Associates Architects, P.C.	
ENGINEER REVIEW (IF APPLICABLE)	STRUCTURAL DESIGN GROUP, INC. JOB NO: 24-140 Review is for general compliance with the structural portions of the contract documents. No responsibility is assumed or implied for the correctness of dimensions or details. Received: 8-14-2025 Comments: NO EXCEPTIONS TAKEN Reviewed: 8-19-2025

SPECTEST ENGINEERING

substruct

August 13, 2025

Garrett Ellis
STONE BUILDING, LLC
8011 Liberty Parkway, Suite 201
Vestavia Hills, AL 35242

RE: AL Energy Infrastructure Training Center
Jasper, AL
Aggregate Pier Design

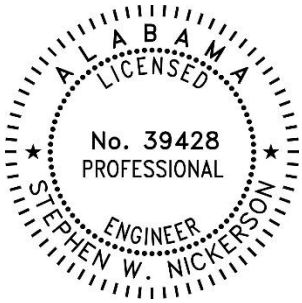
Garrett:

SubStruct LLC is pleased to submit this design for Aggregate Piers (AP's) for the above-mentioned project. The AP treatment will provide a bearing capacity of up to 6,000 psf and maximum long-term total settlement of less than 1 inch. This design submittal includes our design and verification procedures, layout drawing, and aggregate pier design summary.

SubStruct appreciates the opportunity to provide our services to you. Please feel free to contact us to discuss any questions you may have.

Sincerely,

Stephen W. Nickerson, PE



Stephen W. Nickerson, PE
SubStruct LLC
(843) 518-0701

SPECTEST ENGINEERING

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Aggregate Pier Design, Installation, and Verification Procedures

The following procedures supersede all sections of the aggregate pier specification section. Acceptance of this design submittal includes acceptance of this document.

1. Shop drawings reference structural drawings by SDG dated August 21, 2024.
2. Design is based on the data provided in the geotechnical report by BECC dated May 17, 2024.
3. Prior to any fill placement and aggregate pier installation, remove and replace any organic/deleterious materials.
4. Aggregate pier survey shall be performed by a licensed surveyor. Aggregate pier plan location may not deviate by more than 6 inches from the design location without the aggregate pier designer's approval.
5. Aggregate piers shall be installed to design depth unless refusal is encountered or the aggregate pier designer's approval. Refusal is determined by the aggregate pier design engineer.
6. Aggregate piers will be compacted by a proprietary hydraulic compactor system with a blow rate of over 300 blows per minute and 1,000 foot-pounds of energy. Compaction will be performed in one lift per location.
7. Aggregate pier diameter will vary due to variability of soil stiffness across the work area. Aggregate piers for foundations are 2 or 2.5 ft diameter.
8. A static load test shall be performed on one test aggregate pier with 2-foot diameter using Procedure A: Quick Test per ASTM D1143. Deviations from ASTM D1143 may be allowed as the standard was developed for the testing of piles, not aggregate piers. The plate utilized shall be 2 feet in diameter. The maximum load tested is 200% of the design bearing pressure (6,000 psf). A telltale will not be utilized.
9. Follow all site preparation recommendations as outlined in the report of geotechnical investigation.
10. Treated areas shall be compacted within footing with a tamper or jumping jack (by others) after AP installation and footing excavation.
11. Please review that all footings with required aggregate pier treatment have been captured in our design layout drawing.

Aggregate Pier (AP) Design Summary

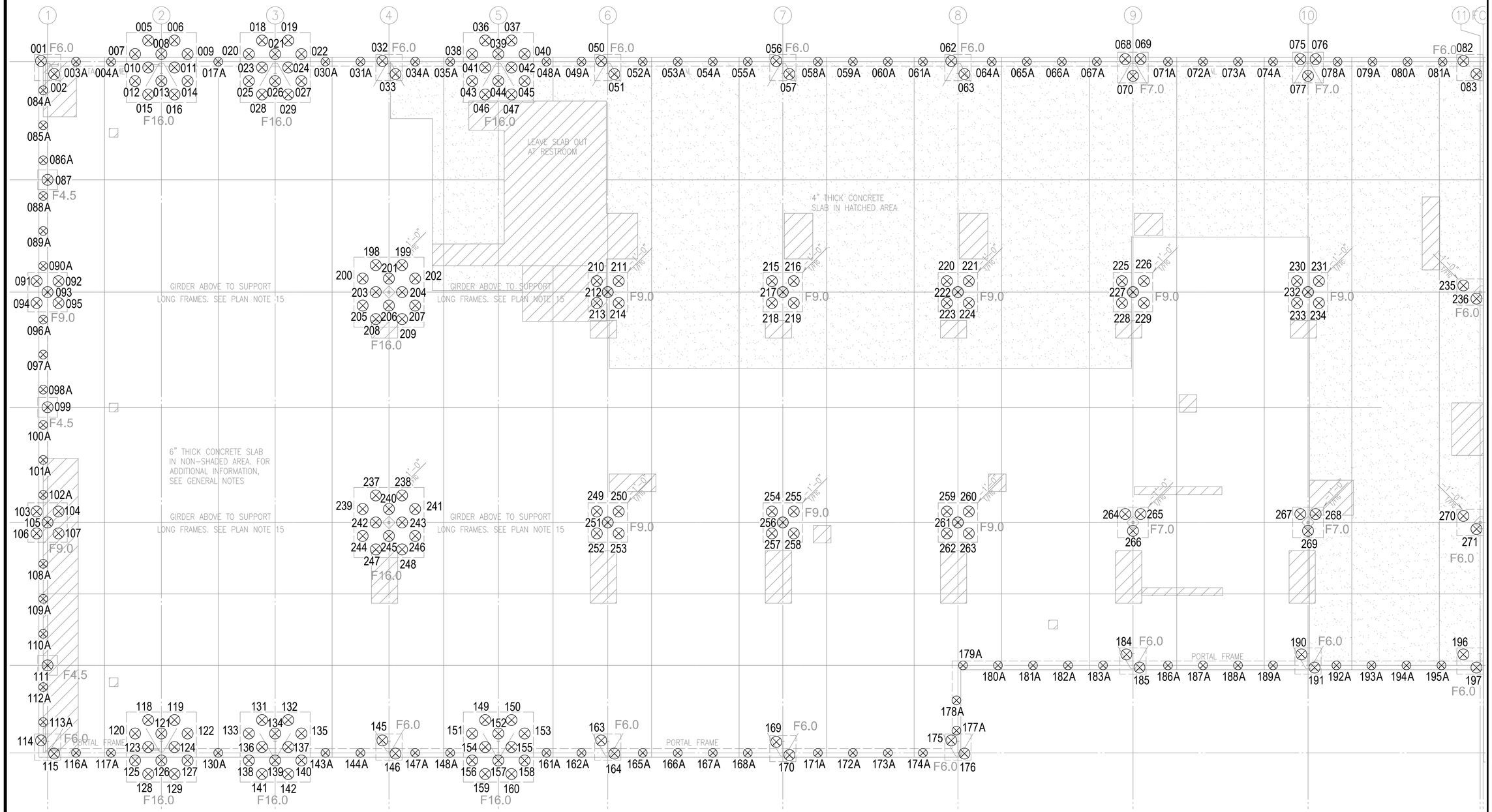
Project Name:	AL Energy Infrastructure Center
Location:	Jasper, AL
Project #:	25-22
Allow Bearing Capacity:	6,000 psf
Allow Total Settlement:	1 inch
Allow Differential Settlement:	1/2 inch
VSC Average Diameter:	2 or 2.5 feet
Groundwater Depth:	8 feet
Pier Stiffness:	120 pci (verified by modulus test)

SOIL PARAMETERS

Layer	Bottom of Layer Depth (ft)	Unit Weight (pcf)	Constrained Modulus (psf)	Poisson's Ratio
Layer 1	20	115	200,000	0.33

DESIGN TABLE

Footing Tag	Footing Width (ft)	Footing Length (ft)	AP's or Spacing	Area Replacement Ratio	VSC Design Length (ft) Below BOF	AP Diameter (ft)
WF2	2.0	-	8	-	4	2.0
F4.5	4.5	4.5	1	0.24	7	2.5
F6	6.0	6.0	2	0.27	7	2.5
F7	7.0	7.0	3	0.30	7	2.5
F9	9.0	9.0	5	0.30	7	2.5
F16	16.0	16.0	12	0.23	7	2.5



PIER LAYOUT
SCALE:3/32" = 1'-0"

ALABAMA LICENSED No. 39428 PROFESSIONAL ENGINEER STEPHEN W. NICKERSON

SUBSTRUCT, LLC
GROUND IMPROVEMENT
26001 E 111TH ST S
COWETA, OK 7449
(843) 518-0701
WWW.SUBSTRUCTLLC.COM

PIER LAYOUT

AL. ENERGY INFRASTRUCTURE TRAINING CENTER
BEVILL STATE COMMUNITY COLLEGE
3711 INDUSTRIAL COURT, JASPER, AL 35501

REVISIONS:

DRAWN BY:	ALS
CHECKED BY:	CLH
DATE:	08/12/25
SCALE:	AS NOTED
FILE:CS-PIER LAYOUT	

S1
SHEET

STONE

Alabama Energy Infrastructure
Training Center
Submittal Review

SUBMITTAL NUMBER: 13100-001

DESCRIPTION: Pre-Engineered Steel Building - Shop Drawings and Design Analysis

Contractor Review Stone Building LLC	**STONE BUILDING, LLC** This submittal is reviewed for conformance with plans and specifications. Submittal review does not relieve subcontractor/supplier of meeting contract requirements. ■ REVIEWED FOR CONFORMANCE ☐ RELEASED WITHOUT COMMENTS ☐ RELEASED PER COMMENTS ☐ NOT RELEASED ☐ REVISE AND RESUMIT BY GE DATE 9/4/25 SUBMITTAL # 13100-001
Architect Review Lathan Associates Architects, P.C.	
ENGINEER REVIEW (IF APPLICABLE)	STRUCTURAL DESIGN GROUP, INC. JOB NO: 24-140 Review is for general compliance with the structural portions of the contract documents. No responsibility is assumed or implied for the correctness of dimensions or details. Received: 09/03/2025 Comments: MAKE CORRECTIONS NOTED Reviewed: TPF 09/10/2025

ALLMETAL BUILDING SYSTEMS

1943 East Malvern Hwy
Dothan, AL 36305
(334) 792-6121

BUILDING LOADS / DESCRIPTION:

WIDTH: 158 LENGTH: 210.33 HEIGHT: 21 /21
(BUILDING DIMENSIONS ARE NOMINAL. REFER TO PLANS).

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY : IBC 21 .

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD:	4.00	PSF (ROOF PANELS & PURLINS)
COLLATERAL LOAD:	10.00	PSF
ROOF LIVE LOAD:	20.00	PSF
ROOF SNOW LOAD:	7.70	PSF
BASIC WIND SPEED:	115	MPH
SEISMIC ZONE:	C	
WIND EXPOSURE:	C	

IMPORTANCE FACTORS:

WIND LOAD:	___	1.00
SNOW LOAD	___	
SEISMIC LOAD	___	1.25

GENERAL NOTES:

1) MATERIALS :

	MINIMUM YIELD:	
HOT ROLLED BAR	Fy =	ksi MIN.
STRUCTURAL STEEL SHEET	Fy =	ksi MIN.
STRUCTURAL STEEL PLATE	Fy =	ksi MIN.
COLD FORMED SHAPES	Fy =	ksi MIN.
WALL SHEETING	Fy =	ksi MIN.
ROOF SHEETING	Fy =	ksi MIN.
BOLTS	A307 & A325	

THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.

2) BOLT TIGHTENING REQUIREMENTS:
ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE. HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE TURN OF THE NUT METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". A325 BOLTS SHALL BE INSTALLED WITH OUT WASHERS WHEN TIGHTENED BY THE "TURN OF THE NUT" METHOD. ALL BOLTED CONNECTIONS, FOR SHEAR/BEARING CONNECTION TYPE WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNUG TIGHT ONLY.

3) ALL STRUCTUAL STEEL TO RECEIVE A RUST INHIBITIVE PRIMER. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.

ROOF PANELS:

COLOR: Galvalume +

WALL PANELS:

COLOR: NEED SIG 200

TRIM COLORS:

CABLE: NEED SIG 200
CORNER: NEED SIG 200
EAVE: NEED SIG 200
FRAMED OPENINGS: NEED SIG 200

LINER PANELS:

COLOR: N/A

LINER TRIM:

COLOR: N/A

DEFLECTION LIMTS:

EW COL:	180
EW RAF LIVE:	360
EW RAF WIND:	240
WALL GIRT:	120
PURL LIVE:	360
PURL WIND:	240
WALL PANEL:	60
ROOF PANEL LIVE:	60
ROOF PANEL WIND:	60
RF HORIZONTAL:	200
RF VERTICAL:	360

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL
09-04-2025

BUILDER / CONTRACTOR RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUILDER/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE METAL BUILDING SYSTEM MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.
THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS REQUIRED.
APPROVAL OF THE METAL BUILDING SYSTEM MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE METAL BUILDING SYSTEM MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.2.1 AISC CODE OF STANDARD PRACTICES, 303-22)
WHERE DISCREPANCIES EXIST BETWEEN THE METAL BUILDING SYSTEM MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE 303-22)
DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE METAL BUILDING SYSTEM MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE METAL BUILDING SYSTEM MANUFACTURER'S ENGINEER UNLESS SPECIFICALLY INDICATED.
THE CONTRACTOR IS RESPONSIBILE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE METAL BUILDING SYSTEM MANUFACTURER "FOR CONSTRUCTION" DRAWINGS.
ALL BRACING AS SHOWN AND PROVIDED BY THE METAL BUILDING SYSTEM MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.
TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLIED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION, OR COLLISION. (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, 303-22)
WARNING: IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSION EFFECTS ON THE ALUMINUM ZINC ALLOY COATING WHEN THEY ARE USED IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE AVOIDED.

Do Not Use Abrasive Wheels To Cut Panels

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:
IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS BE MADE IN CONTRASTING INK (PREFERABLY RED INK), HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED, AND BE LEGIBLE AND UNAMBIGUOUS.
A SIGNATURE AND DATE IS REQUIRED ON ALL PAGES.
MANUFACTURER RESERVES THE RIGHT TO RE-SUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.
APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE METAL BUILDING SYSTEM MANUFAACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER.
ANY CHANGES NOTED ON THHE DRAWINGS NOT IN COMFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION. MANUFACTURER RECONGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTURAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILIAR INDICATIOIN OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERNATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

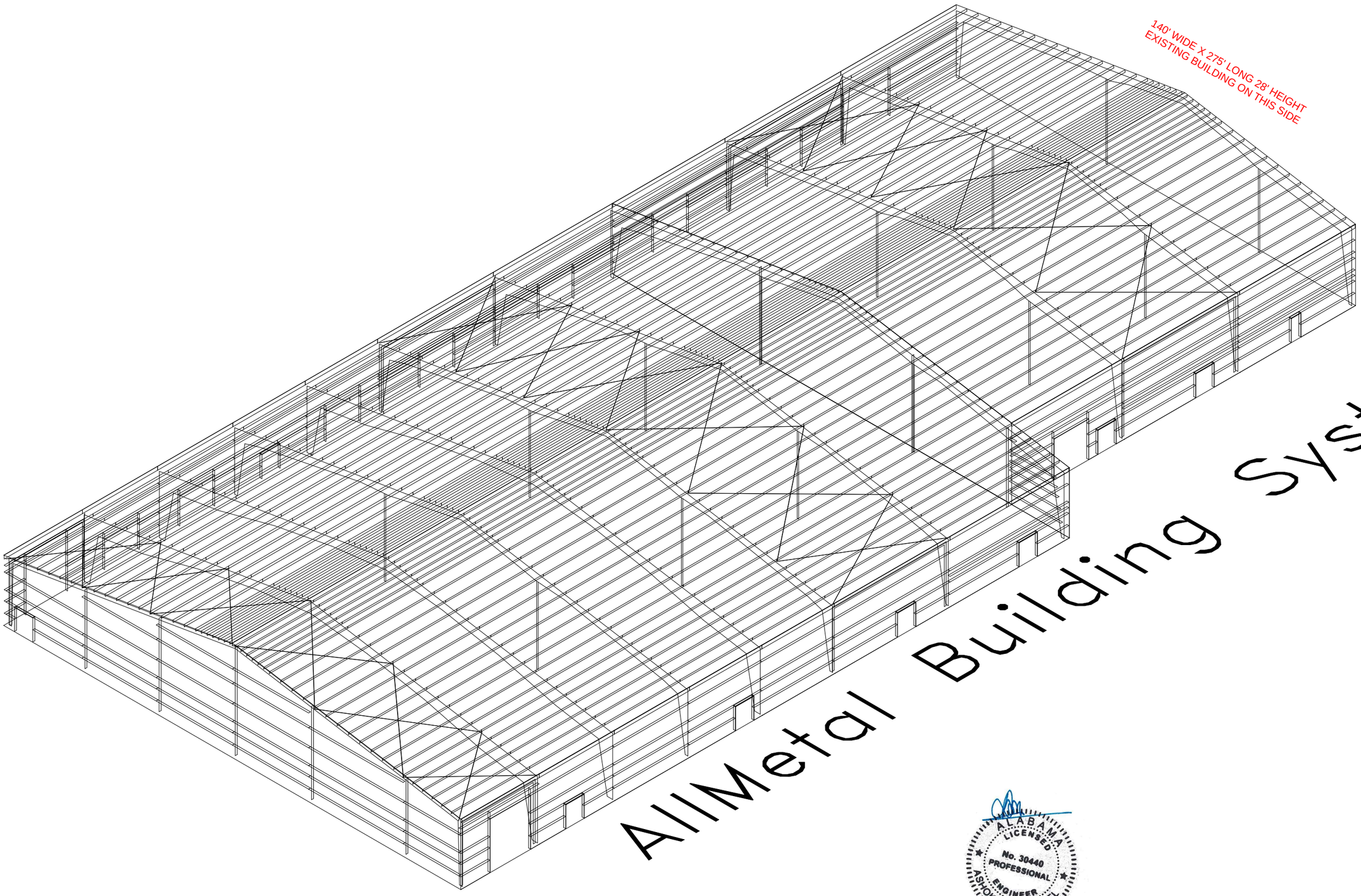
IMPORTANT NOTE: FINAL DETAILING, FABRICATION, AND DELIVERY DATE OF THIS PROJECT CANNOT BE COMPLETED UNTIL THE SIGNED APPROVALS ARE RETURNED TO THE METAL BUILDING MANUFACTURER.

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE.

Ashokbhai Patel
Digitally signed by Ashokbhai Patel
DN: C=US, E=ashokpatel@gmail.com, OU="", O=Jupiter Construciton LLC, CN=Ashokbhai Patel
Location: Valdosta
Reason: I have reviewed this document
Contact Info: (229)402-2640
Date: 2025.09.04 09:47:00-04'00'

REV.	DATE	REVISION
B	../../..	FOR CONSTRUCTION
A	../../..	FOR APPROVAL

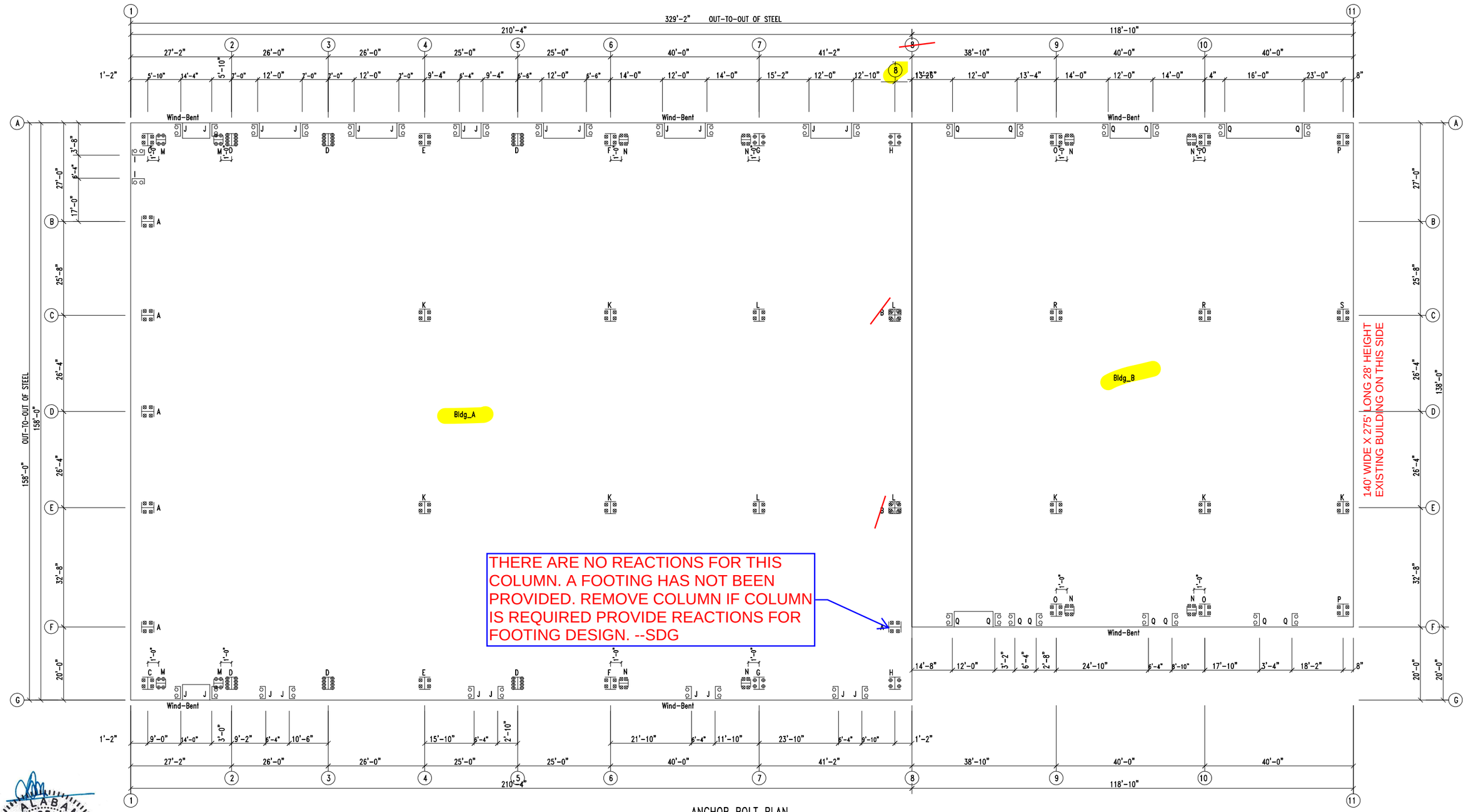
PURCHASER: Customer
PROJECT: Project Name
JOB NUMBER: Bldg_A



AllMetal Building Systems

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025



ANCHOR BOLT PLAN

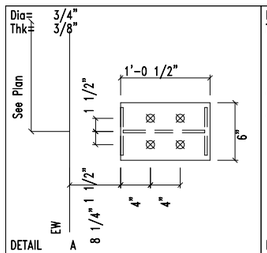
NOTE: All Base Plates @ 100'-0" (U.N.)

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

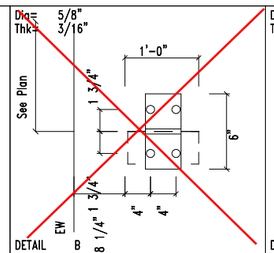
AllMetal Building Systems				
PROJECT	Project Name	ANCHOR BOLT PLAN		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

Dia= 3/4"
Thk= 3/8"



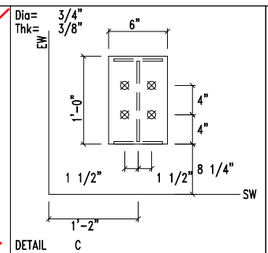
DETAIL A

Dia= 5/8"
Thk= 3/16"



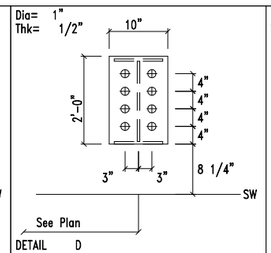
DETAIL B

Dia= 3/4"
Thk= 3/8"



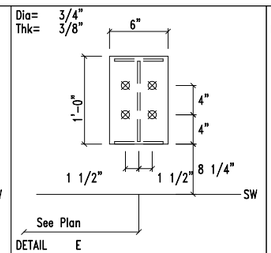
DETAIL C

Dia= 1"
Thk= 1/2"



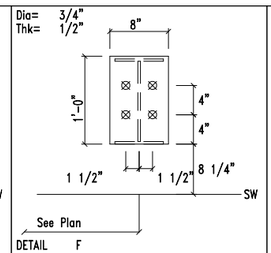
DETAIL D

Dia= 3/4"
Thk= 3/8"



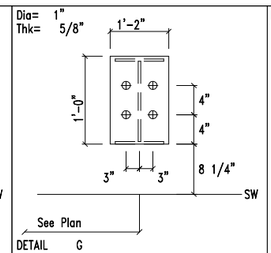
DETAIL E

Dia= 3/4"
Thk= 1/2"



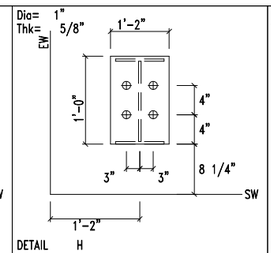
DETAIL F

Dia= 1"
Thk= 5/8"



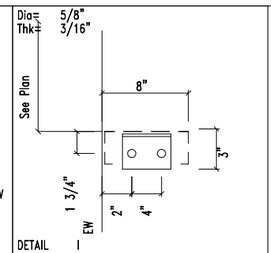
DETAIL G

Dia= 1"
Thk= 5/8"



DETAIL H

Dia= 5/8"
Thk= 3/16"



DETAIL I

Dia= 5/8"
Thk= 3/16"

DETAIL J

Dia= 3/4"
Thk= 1/2"

DETAIL K

Dia= 3/4"
Thk= 3/4"

DETAIL L

Dia= 1"
Thk= 3/4"

DETAIL M

Dia= 3/4"
Thk= 5/8"

DETAIL N

Dia= 3/4"
Thk= 1/2"

DETAIL O

Dia= 3/4"
Thk= 3/8"

DETAIL P

Dia= 5/8"
Thk= 3/16"

DETAIL Q

Dia= 3/4"
Thk= 5/8"

DETAIL R

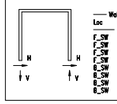
Dia= 3/4"
Thk= 5/8"

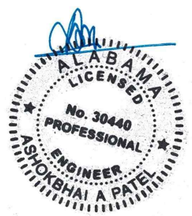
DETAIL S

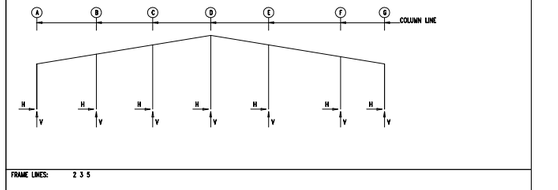
GENERAL NOTES

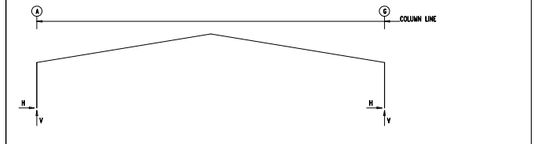
09-04-2025

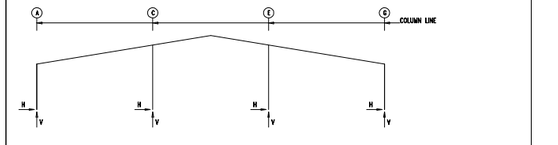
AllMetal Building Systems					
PROJECT	Project Name	ANCHOR BOLT DETAILS			
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:	
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25		SHEET	OF











NOTE: THE FRAMING AT LEFT ENDWALL IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THIS FRAME LINE REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

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NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	= 158.0
Length	(ft)	= 210.3
Eave Height	(ft)	= 21.0/ 21.0
Roof Slope	(rise/12)	= 2.00/ 2.00
Roof Dead Load	(psf)	= 4.0
Wall Dead Load		
Left Endwall	(psf)	= 2.0
Right Endwall	(psf)	= 2.0
Front Sidewall	(psf)	= 2.0
Back Sidewall	(psf)	= 2.0
Roof Live Load	(psf)	= 20.0
Frame Live Load	(psf)	= 12.0
Collateral Load	(psf)	= 10.0
Snow Load	(psf)	= 7.7
Minimum Snow	(psf)	= 11.0
Wind Speed	(mph)	= 115.0
Wind Code		= IBC 21
Exposure		= C
Closure		= Enclosed
Internal Wind Coeff		= −0.18, +0.18
Risk Category		= III− High
Importance − Wind		= 1.00
Importance − Seismic		= 1.25
Seismic Design Category		= C
Seismic Coeff	(Sms)	= 0.46

ID	Description
1	Dead+Collateral+Live
2	Dead+0.6Wind_Left1
3	Dead+0.6Wind_Right1
4	Dead+Collateral+0.75Live+0.45Wind_Long1R
5	Dead+Collateral+0.75Live+0.45Wind_Long2L
6	Dead+Collateral+0.75Live+0.45Wind_Long2R
7	Dead+Collateral+0.75Snow+0.45Wind_Left1
8	Dead+Collateral+0.75Snow+0.45Wind_Left1+0.75Snow_Drift
9	Dead+Collateral+0.75Snow+0.45Wind_Left1+0.75Slide_Snow
10	0.6Dead+0.6Wind_Left1
11	0.6Dead+0.6Wind_Right1
12	0.6Dead+0.6Wind_Left2
13	0.6Dead+0.6Wind_Right2
14	0.6Dead+0.6Wind_Long1L
15	0.6Dead+0.6Wind_Long1R
16	0.6Dead+0.6Wind_Long2L
17	0.6Dead+0.6Wind_Long2R
18	1.03Dead+1.03Collateral+0.75Live+0.53Seismic_Left
19	1.03Dead+1.03Collateral+0.75Live+0.53Seismic_Right
20	1.03Dead+1.03Collateral+0.52Seismic_LongR
21	Dead+Collateral+F1PAT_LL_2
22	Dead+Collateral+F1PAT_LL_3
23	Dead+Collateral+F1PAT_LL_4
24	Dead+Collateral+F3PAT_LL_1
25	Dead+Collateral+F3PAT_LL_2
26	Dead+Collateral+F3PAT_LL_3
27	Dead+Collateral+F4PAT_LL_1
28	Dead+Collateral+F4PAT_LL_2
29	Dead+Collateral+F4PAT_LL_3
30	Dead+Collateral+F5PAT_LL_1
31	Dead+Collateral+F5PAT_LL_2
32	Dead+Collateral+F5PAT_LL_3
33	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
34	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
35	Dead+Collateral+E1PAT_LL_1
36	0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
37	Dead+Collateral+E1PAT_LL_2
38	Dead+Collateral+E1PAT_LL_3
39	0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
40	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
41	Dead+Collateral+E1PAT_LL_4
42	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
43	Dead+Collateral+E1PAT_LL_5
44	0.6Dead+0.6Wind_Right2+0.6Wind_Suction

WIND BENT REACTIONS

—— Wall —— Loc	Line	Col Line	± Reactions Wind(k) Horz	Vert	Seismic(k) Horz	Vert	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick
F_SW	G	1	5.1	8.3	8.7	14.3	4	1.000	8.000	17.688	0.750
F_SW	G	2	5.1	8.3	8.7	14.3	4	1.000	8.000	17.688	0.750
F_SW	G	6	5.1	5.2	8.7	8.9	4	0.750	6.000	17.688	0.625
F_SW	G	7	5.1	5.2	8.7	8.9	4	0.750	6.000	17.688	0.625
B_SW	A	7	5.1	5.2	8.7	8.9	4	0.750	6.000	17.688	0.625
B_SW	A	6	5.1	5.2	8.7	8.9	4	0.750	6.000	17.688	0.625
B_SW	A	2	5.1	8.3	8.7	14.3	4	1.000	8.000	17.688	0.750
B_SW	A	1	5.1	8.3	8.7	14.3	4	1.000	8.000	17.688	0.750

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	------Dead----		--Collateral-		------Live----		------Snow----		--Wind_Left1-		-Wind_Right1-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	A	0.1	1.4	0.2	1.8	0.2	2.1	0.1	1.4	−3.0	−5.5	3.4	−1.7
1	G	−0.1	1.0	−0.2	1.2	−0.2	1.4	−0.1	0.9	−3.7	0.0	3.3	−4.8
1	B	0.0	2.5	0.0	4.0	0.0	4.7	0.0	3.0	0.0	−7.4	0.0	−7.6
1	C	0.0	2.5	0.0	3.7	0.0	4.4	0.0	2.8	0.0	−7.4	0.0	−5.3
1	D	0.0	2.6	0.0	3.6	0.0	4.3	0.0	2.8	0.0	−4.1	0.0	−3.8
1	E	0.0	3.0	0.0	4.4	0.0	5.3	0.0	3.4	0.0	−6.5	0.0	−9.0
1	F	0.0	2.5	0.0	4.0	0.0	4.7	0.0	3.0	0.0	−8.3	0.0	−6.9

Frame Line	Column Line	--Wind_Left2-		-Wind_Right2-		--Wind_Press-		--Wind_Suct--		--Wind_Long1-		--Wind_Long2-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	A	−4.3	−3.2	2.1	0.6	0.0	0.0	0.0	0.0	2.7	−5.3	0.5	−5.4
1	G	−2.4	1.7	4.6	−3.1	0.0	0.0	0.0	0.0	−0.1	−5.3	−3.1	−2.3
1	B	0.0	−4.0	0.0	−4.1	−6.9a	0.0	7.6a	0.0	0.0	−14.0	0.0	−5.6
1	C	0.0	−3.8	0.0	−1.8	−8.0a	0.0	8.9a	0.0	0.0	−12.1	0.0	−7.0
1	D	0.0	−2.0	0.0	−1.7	−9.4a	0.0	10.4a	0.0	0.0	−6.8	0.0	−6.7
1	E	0.0	−2.2	0.0	−4.8	−9.1a	0.0	10.1a	0.0	0.0	−8.3	0.0	−14.6
1	F	0.0	−4.8	0.0	−3.4	−6.5a	0.0	7.2a	0.0	0.0	−4.8	0.0	−14.9

Frame Line	Column Line	−Seismic_Left		Seismic_Right		−Seismic_Long		−MIN_SNOW--		F1PAT_LL_1-		F1PAT_LL_2-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	A	−2.1	−2.0	2.1	2.0	0.0	0.0	0.2	2.0	0.2	2.0	0.0	−0.2
1	G	−2.6	3.3	2.6	−3.3	0.0	0.0	−0.2	1.3	−0.2	0.2	0.0	−0.1
1	B	0.0	2.5	0.0	−2.5	0.1a	0.0	0.0	4.3	0.0	5.0	0.0	2.0
1	C	0.0	−0.6	0.0	0.6	0.1a	0.0	0.0	4.0	0.0	2.1	0.0	5.0
1	D	0.0	−0.1	0.0	0.1	0.1a	0.0	0.0	4.0	0.0	−0.3	0.0	2.2
1	E	0.0	0.5	0.0	−0.5	0.1a	0.0	0.0	4.8	0.0	0.1	0.0	−0.3
1	F	0.0	−3.6	0.0	3.6	0.1a	0.0	0.0	4.4	0.0	−0.2	0.0	0.1

Frame Line	Column Line	F1PAT_LL_3-		F1PAT_LL_4-		F1PAT_LL_5-		F1PAT_LL_6-		F1PAT_LL_7-		F1UNB_SL_L-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	A	0.0	0.1	−0.1	−0.1	0.0	0.0	0.2	2.3	0.0	−0.2	0.1	1.4
1	G	0.0	0.1	0.1	−0.6	0.0	1.0	−0.2	−0.4	−0.1	1.8	−0.1	0.4
1	B	0.0	−0.3	0.0	0.1	0.0	−0.1	0.0	2.3	0.0	2.4	0.0	2.7
1	C	0.0	2.1	0.0	−0.2	0.0	0.1	0.0	2.2	0.0	2.1	0.0	4.2
1	D	0.0	5.2	0.0	2.0	0.0	−0.6	0.0	2.0	0.0	2.3	0.0	4.7
1	E	0.0	2.0	0.0	5.6	0.0	3.1	0.0	2.9	0.0	2.3	0.0	0.6
1	F	0.0	−0.3	0.0	3.1	0.0	5.2	0.0	3.2	0.0	1.5	0.0	0.9

Frame Line	Column Line	F1UNB_SL_R-	
		Horz	Vert
1	A	0.1	0.4
1	G	−0.1	0.9
1	B	0.0	1.0
1	C	0.0	0.4
1	D	0.0	4.7
1	E	0.0	4.7
1	F	0.0	2.9

Frame Line	Column Line	------Dead----		--Collateral-		------Live----		------Snow----		--Wind_Left1-		-Wind_Right1-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	A	14.9	14.1	24.5	20.8	29.0	24.6	18.6	15.8	−44.4	−40.0	−32.1	−31.7
2*	G	−14.9	14.1	−24.5	20.8	−29.0	24.6	−18.6	15.8	32.1	−31.7	44.4	−40.0

Frame Line	Column Line	--Wind_Left2-		-Wind_Right2-		--Wind_Long1-		--Wind_Long2-		−Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	A	−26.4	−20.9	−14.1	−12.6	−51.9	−58.4	−56.7	−45.5	−4.3	−1.0	4.3	1.0
2*	G	14.1	−12.6	26.4	−20.9	56.7	−45.5	51.9	−58.4	−4.3	1.0	4.3	−1.0

Frame Line	Column Line	−MIN_SNOW--		F2UNB_SL_L-		F2UNB_SL_R-	
		Horz	Vert	Horz	Vert	Horz	Vert
2*	A	26.6	22.6	18.4	16.8	18.3	10.5
2*	G	−26.6	22.6	−18.3	10.5	−18.4	16.8

Frame Line	Column Line	------Dead----		--Collateral-		------Live----		------Snow----		--Wind_Left1-		-Wind_Right1-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
4	A	1.8	4.4	3.5	7.7	4.1	9.1	2.6	5.8	−12.3	−19.3	−0.1	−12.3
4	G	−1.8	4.4	−3.5	7.7	−4.1	9.1	−2.6	5.8	0.1	−12.3	12.3	−19.3
4	C	0.0	7.4	0.0	13.1	0.0	15.6	0.0	10.0	0.0	−21.8	0.0	−18.3
4	E	0.0	7.4	0.0	13.1	0.0	15.6	0.0	10.0	0.0	−18.3	0.0	−21.7

Frame Line	Column Line	--Wind_Left2-		-Wind_Right2-		--Wind_Long1-		--Wind_Long2-		−Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
4	A	−11.5	−10.9	0.8	−3.9	−1.8	−17.4	−4.7	−14.5	−4.3	−2.0	4.3	2.0
4	G	−0.8	−3.8	11.5	−10.9	4.7	−14.5	1.8	−17.4	−4.3	2.0	4.3	−2.0
4	C	0.0	−11.1	0.0	−7.6	0.0	−28.6	0.0	−14.8	0.0	2.9	0.0	−2.9
4	E	0.0	−7.6	0.0	−11.1	0.0	−14.8	0.0	−28.6	0.0	−2.9	0.0	2.9

Frame Line	Column Line	−MIN_SNOW--		F3PAT_LL_1-		F3PAT_LL_2-		F3PAT_LL_3-		F3PAT_LL_4-		F3UNB_SL_L-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
4	A	3.8	8.3	1.5	7.1	1.5	0.2	5.3	10.9	−1.2	−1.8	1.1	4.3
4	G	−3.8	8.3	−1.5	0.2	−1.5	7.1	−5.3	10.9	1.2	−1.8	−1.1	1.4
4	C	0.0	14.2	0.0	18.9	0.0	6.7	0.0	5.6	0.0	10.0	0.0	15.8
4	E	0.0	14.2	0.0	6.7	0.0	18.9	0.0	5.6	0.0	10.0	0.0	5.8

Frame Line	Column Line	F3UNB_SL_R-	
		Horz	Vert
4	A	1.1	1.4
4	G	−1.1	4.3
4	C	0.0	5.8
4	E	0.0	15.8

Frame Line	Column Line	------Dead----		--Collateral-		------Live----		------Snow----		--Wind_Left1-		-Wind_Right1-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
6	A	2.3	5.5	4.6	9.7	5.5	11.5	3.5	7.4	−16.1	−24.5	−0.8	−15.7
6	G	−2.3	5.5	−4.6	9.7	−5.5	11.5	−3.5	7.4	0.8	−15.7	16.1	−24.5
6	C	0.0	8.7	0.0	16.3	0.0	19.3	0.0	12.4	0.0	−26.8	0.0	−22.5
6	E	0.0	8.7	0.0	16.3	0.0	19.3	0.0	12.4	0.0	−22.5	0.0	−26.8

Frame Line	Column Line	--Wind_Left2-		-Wind_Right2-		--Wind_Long1-		--Wind_Long2-		−Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
6	A	−14.8	−13.8	0.5	−5.1	−2.8	−22.0	−6.4	−18.5	−5.4	−2.6	5.4	2.6
6	G	−0.6	−5.0	14.8	−13.9	6.4	−18.5	2.8	−22.0	−5.4	2.6	5.4	−2.6
6	C	0.0	−13.6	0.0	−9.3	0.0	−35.7	0.0	−18.0	0.0	3.8	0.0	−3.8
6	E	0.0	−9.3	0.0	−13.6	0.0	−18.0	0.0	−35.7	0.0	−3.8	0.0	3.8

Frame Line	Column Line	−MIN_SNOW--		F4PAT_LL_1-		F4PAT_LL_2-		F4PAT_LL_3-		F4PAT_LL_4-		F4UNB_SL_L-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
6	A	5.0	10.6	1.9	8.8	1.9	0.3	7.2	13.9	−1.7	−2.4	1.4	5.3
6	G	−5.0	10.6	−1.9	0.3	−1.9	8.8	−7.2	13.9	1.7	−2.4	−1.4	1.8
6	C	0.0	17.7	0.0	23.8	0.0	8.1	0.0	6.6	0.0	12.6	0.0	19.8
6	E	0.0	17.7	0.0	8.1	0.0	23.8	0.0	6.6	0.0	12.6	0.0	7.1

Frame Line	Column Line	F4UNB_SL_R-	
		Horz	Vert
6	A	1.4	1.8
6	G	−1.4	5.3
6	C	0.0	7.1
6	E	0.0	19.8

Frame Line	Column Line	------Dead----		--Collateral-		------Live----		------Snow----		--Wind_Left1-		-Wind_Right1-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
7*	A	3.2	7.4	6.6	12.5	7.8	14.8	5.0	9.5	−21.5	−31.1	−2.7	−20.4
7*	G	−3.2	7.4	−6.6	12.5	−7.8	14.8	−5.0	9.5	2.7	−20.4	21.5	−31.1
7*	C	0.0	10.2	0.0	19.5	0.0	23.1	0.0	14.8	0.0	−32.1	0.0	−26.6
7*	E	0.0	10.2	0.0	19.5	0.0	23.1	0.0	14.8	0.0	−26.6	0.0	−32.1

Frame Line	Column Line	--Wind_Left2-		-Wind_Right2-		--Wind_Long1-		--Wind_Long2-		−Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
7*	A	−19.2	−17.6	−0.3	−6.8	−8.0	−38.6	−15.3	−31.2	−12.5	−6.1	12.5	6.1
7*	G	0.3	−6.8	19.2	−17.6	15.3	−31.2	8.0	−38.6	−12.2	6.3	12.2	−6.3
7*	C	0.0	−16.3	0.0	−10.8	0.0	−63.2	0.0	−26.8	0.0	9.0	0.0	−9.0
7*	E	0.0	−10.8	0.0	−16.3	0.0	−26.8	0.0	−63.2	0.0	−9.2	0.0	9.2

Frame Line	Column Line	−MIN_SNOW--		F5PAT_LL_1-		F5PAT_LL_2-		F5PAT_LL_3-		F5PAT_LL_4-		F5UNB_SL_L-	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
7*	A	7.1	13.6	2.6	11.1	2.6	0.7	10.2	17.8	−2.5	−3.0	2.0	6.8
7*	G	−7.1	13.6	−2.6	0.7	−2.7	11.1	−10.2	17.8	2.5	−3.0	−2.0	2.4
7*	C	0.0	21.2	0.0	29.0	0.0	9.7	0.0	7.4	0.0	15.6	0.0	24.2
7*	E	0.0	21.2	0.0	9.7	0.0	29.0	0.0	7.4	0.0	15.6	0.0	8.6

Frame Line	Column Line	F5UNB_SL_R-	
		Horz	Vert
7*	A	2.0	2.4
7*	G	−2.0	6.8
7*	C	0.0	8.6
7*	E	0.0	24.2

2* Frame lines: 2 3 5
7* Frame lines: 7 8

a − Out-Of-Plane Horizontal Load

FRAME LINES: 1

FRAME LINES: 2 3 5

FRAME LINES: 4 6 7 8

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
1	A	3 19	2.1 1.5	0.4 5.9	12 10	−2.5 −1.8	−1.1 −2.5	4	0.750	6.000	12.00	0.375	0.0
1	G	13 18	2.7 −1.8	−1.2 5.0	2 14	−2.3 −0.1	1.1 −2.5	4	0.750	6.000	12.00	0.375	0.0
1	B	8 18	4.5 0.0	−6.7 11.5	4 15	−4.1 0.0	−6.5 −6.9	4	0.750	6.000	12.50	0.375	0.0
1	C	6 21	5.3 0.0	−5.7 11.2	4 15	−4.8 0.0	−5.7 −5.8	4	0.750	6.000	12.50	0.375	0.0
1	D	6 22	6.3 0.0	−3.0 11.4	4 15	−5.7 0.0	−3.0 −2.5	4	0.750	6.000	12.50	0.375	0.0
1	E	7 23	6.0 0.0	−6.9 13.0	5 17	−5.5 0.0	−6.9 −7.0	4	0.750	6.000	12.50	0.375	0.0
1	F	9 19	4.3 0.0	−6.8 12.2	5 17	−3.9 0.0	−6.6 −7.4	4	0.750	6.000	12.50	0.375	0.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
2*	A	1	68.4	59.5	17 15	−25.1 −22.2	−18.9 −26.6	8	1.000	10.00	24.00	0.500	0.0
2*	G	15 1	25.1 −68.4	−18.9 59.5	1 16	−68.4 22.2	59.5 −26.6	8	1.000	10.00	24.00	0.500	0.0

2* Frame lines: 2 3 5

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
4	A	19 26	10.7 10.5	20.3 23.0	10	−6.3	−8.9	4	0.750	6.000	12.00	0.375	0.0
4	G	11 26	6.3 −10.5	−8.9 23.0	18 11	−10.7 6.3	20.3 −8.9	4	0.750	6.000	12.00	0.375	0.0
4	C	14 24	0.0 0.0	−12.7 39.4	15	0.0	−12.7	4	0.750	8.000	8.000	0.500	0.0
4	E	17 25	0.0 0.0	−12.7 39.4	17	0.0	−12.7	4	0.750	8.000	8.000	0.500	0.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
6	A	29	14.1	29.1	10	−8.3	−11.4	4	0.750	8.000	12.00	0.500	0.0
6	G	11 29	8.3 −14.1	−11.4 29.1	29 11	−14.1 8.3	29.1 −11.4	4	0.750	8.000	12.00	0.500	0.0
6	C	15 27	0.0 0.0	−16.2 48.8	15	0.0	−16.2	4	0.750	8.000	8.000	0.500	0.0
6	E	17 28	0.0 0.0	−16.2 48.8	17	0.0	−16.2	4	0.750	8.000	8.000	0.500	0.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
7*	A	19 32	22.5 20.0	34.9 37.8	10 14	−11.0 −2.9	−14.2 −18.7	4	1.000	14.00	12.00	0.625	0.0
7*	G	11 32	11.0 −20.0	−14.2 37.8	18 17	−22.3 2.9	35.0 −18.7	4	1.000	14.00	12.00	0.625	0.0
7*	C	15 30	0.0 0.0	−31.8 58.8	15	0.0	−31.8	4	0.750	10.00	8.250	0.750	0.0
7*	E	17 31	0.0 0.0	−31.8 58.8	17	0.0	−31.8	4	0.750	10.00	8.250	0.750	0.0

7* Frame lines: 7 8

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Wind Press Horz	Wind Suct Horz	Seis Long Vert
8	F	0.3	−2.3	2.5	0.0
8	E	0.2	0.0	0.0	0.0
8	C	0.2	0.0	0.0	0.0

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
8	F	44 20	1.5 0.0	0.2 0.3	40	−1.4	0.2	4	0.750	6.000	12.50	0.375	0.0
8	E	20	0.0	0.2				4	0.625	6.000	6.000	0.188	0.0
8	C	20	0.0	0.2				4	0.625	6.000	6.000	0.188	0.0

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Type	Bend Len (in)	Proj (in)
○	52	Jamb	5/8"			
⊠	24	Endwall	3/4"	A307		2.50
○	8	Endwall	5/8"	A307		2.50
⊠	56	Frame	3/4"	A307	3.00	2.50
⊕	64	Frame	1"	A307	3.00	2.50
⊕	16	WindCol	1"	A307	3.00	2.50
⊠	16	WindCol	3/4"	A307	3.00	2.50

BUILDING BRACING REACTIONS

—— Wall —— Loc	Line	Col Line	± Reactions(k) —— Wind —— Horz	Vert	—— Seismic —— Horz	Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	1								(h)
F_SW	G	1,2							(a)
		6,7							(a)
R_EW	8								(h)
B_SW	A	6,7							(a)
		1,2							(a)

(a)Wind bent in bay
(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

ALABAMA LICENSED PROFESSIONAL ENGINEER
No. 30440
ASHOKBHAI A PATEL

09-04-2025

AllMetal Building Systems

PROJECT	Project Name	ANCHOR BOLT REACTIONS		
ID	25−0027−1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

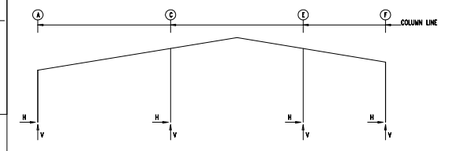
BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Ponel_Shear (lb/ft) Wind	Seis	Note
L_EW	B	Bracing Not Used					0	0	
F_SW	F	9,10							(a)
R_EW	11								(b)
B_SW	A	9,10							(a)

(a)Wind bent in bay
(b)Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

FRAME LINES: 9 10 11



RIGID FRAME:

MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Column Reactions(k) Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
9*	A	10	7.8	24.8	5	-7.3	-5.3	4	0.750	6.000	12.00	0.500	0.0
		13	7.8	28.6	3	-6.4	-12.1						
9*	F	5	8.2	-4.8	9	-10.7	20.5	4	0.750	6.000	12.00	0.500	0.0
		13	-7.8	25.1	4	8.1	-10.4						
9*	C	7	0.0	-22.5	7	0.0	-22.5	4	0.750	10.00	8.250	0.625	0.0
		11	0.0	52.8									
9*	E	8	0.0	-16.6	8	0.0	-16.6	4	0.750	8.000	8.000	0.500	0.0
		12	0.0	52.8									
9*	Frame lines:	9	10										

RIGID FRAME:

MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Column Reactions(k) Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
11	A	1	8.3	25.0	5	-3.9	-2.6	4	0.750	6.000	12.00	0.375	0.0
					7	0.3	-8.9						
11	F	6	4.1	-2.1	2	-6.8	11.4	4	0.750	6.000	12.00	0.375	0.0
		1	-8.3	16.4	7	2.2	-6.7						
11	C	7	0.0	-18.3	7	0.0	-18.3	4	0.750	8.000	8.000	0.625	0.0
		1	0.0	56.1									
11	E	8	0.0	-13.9	8	0.0	-13.9	4	0.750	8.000	8.000	0.500	0.0
		1	0.0	43.4									

RIGID FRAME:

BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Snow_Drift Horz	Snow_Drift Vert	Wind_Left1 Horz	Wind_Left1 Vert
9*	A	1.2	5.2	2.6	9.8	3.1	11.4	2.0	7.5	0.6	2.1	-11.8	-25.5
9*	F	-1.2	4.0	-2.6	6.4	-3.1	7.5	-2.0	4.6	-0.6	1.4	-7.0	-9.6
9*	C	0.0	11.8	0.0	25.0	0.0	27.2	0.0	17.4	0.0	4.9	0.0	-41.5
9*	E	0.0	8.9	0.0	18.8	0.0	19.9	0.0	12.8	0.0	3.6	0.0	-24.8

Frame Line	Column Line	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert
9*	A	3.2	-15.2	-13.3	-14.1	2.4	-4.3	0.4	-24.9	0.3	-17.4	-3.2	-1.1
9*	F	14.7	-21.2	-5.8	0.0	15.0	-12.0	3.8	-19.5	-3.8	-15.5	-8.5	7.6
9*	C	0.0	-37.0	0.0	-21.6	0.0	-16.7	0.0	-48.3	0.0	-31.2	0.0	2.3
9*	E	0.0	-29.5	0.0	-12.5	0.0	-17.3	0.0	-13.6	0.0	-36.5	0.0	-8.8

Frame Line	Column Line	Seismic_Right Horz	Seismic_Right Vert	-MIN_SNOW- Horz	-MIN_SNOW- Vert	F2PAT_LL_1- Horz	F2PAT_LL_1- Vert	F2PAT_LL_2- Horz	F2PAT_LL_2- Vert	F2PAT_LL_3- Horz	F2PAT_LL_3- Vert	F2PAT_LL_4- Horz	F2PAT_LL_4- Vert
9*	A	3.2	1.1	2.8	10.7	2.5	11.2	-0.4	-1.5	4.0	13.6	-0.9	-2.0
9*	F	8.5	-7.6	-2.8	6.9	-2.5	-9.4	0.4	2.7	-4.0	12.7	0.9	-5.2
9*	C	0.0	-2.3	0.0	24.0	0.0	20.0	0.0	12.0	0.0	12.0	0.0	13.4
9*	E	0.0	8.8	0.0	19.2	0.0	11.7	0.0	27.2	0.0	0.9	0.0	19.0

Frame Line	Column Line	F1UNB_SL_L- Horz	F1UNB_SL_L- Vert	F1UNB_SL_R- Horz	F1UNB_SL_R- Vert
9*	A	1.5	6.7	0.3	1.5
9*	F	-1.5	0.7	-0.3	0.7
9*	C	0.0	21.9	0.0	9.6
9*	E	0.0	8.1	0.0	24.3

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Snow_Drift Horz	Snow_Drift Vert	Wind_Left1 Horz	Wind_Left1 Vert
11	A	0.7	3.2	1.3	5.1	1.6	6.1	1.0	3.9	3.3	12.7	-6.5	-13.5
11	F	-0.7	2.5	-1.3	3.5	-1.6	3.9	-1.0	2.5	-3.3	8.1	-3.5	-3.2
11	C	0.0	7.2	0.0	12.0	0.0	14.2	0.0	9.1	0.0	29.7	0.0	-21.6
11	E	0.0	5.7	0.0	8.9	0.0	10.6	0.0	6.8	0.0	22.0	0.0	-12.8

Frame Line	Column Line	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert
11	A	2.0	-7.8	-7.2	-7.5	1.6	-2.0	-0.5	-18.1	-9.7	-11.7	-1.9	-0.8
11	F	7.4	-10.8	-2.8	-0.2	7.6	-6.0	4.3	-13.7	-2.1	-16.5	4.2	3.7
11	C	0.0	-19.6	0.0	-11.2	0.0	-8.5	0.0	-37.6	0.0	-21.4	0.0	1.4
11	E	0.0	-15.9	0.0	-6.4	0.0	-9.4	0.0	-8.9	0.0	-28.8	0.0	-4.4

Frame Line	Column Line	Seismic_Right Horz	Seismic_Right Vert	-MIN_SNOW- Horz	-MIN_SNOW- Vert	F2PAT_LL_1- Horz	F2PAT_LL_1- Vert	F2PAT_LL_2- Horz	F2PAT_LL_2- Vert	F2PAT_LL_3- Horz	F2PAT_LL_3- Vert	F2PAT_LL_4- Horz	F2PAT_LL_4- Vert
11	A	1.9	0.8	1.4	5.6	1.3	5.9	-0.2	-0.8	2.0	7.1	-0.5	-1.0
11	F	4.2	-3.7	-1.4	3.6	-1.3	-0.3	0.2	1.4	-2.0	6.7	0.5	-2.8
11	C	0.0	-1.4	0.0	13.1	0.0	14.7	0.0	6.6	0.0	7.2	0.0	7.0
11	E	0.0	4.4	0.0	9.7	0.0	6.3	0.0	14.4	0.0	0.5	0.0	10.1

Frame Line	Column Line	F2UNB_SL_L- Horz	F2UNB_SL_L- Vert	F2UNB_SL_R- Horz	F2UNB_SL_R- Vert
11	A	0.8	3.5	0.1	0.7
11	F	-0.8	0.5	-0.1	0.5
11	C	0.0	11.4	0.0	5.0
11	E	0.0	4.3	0.0	12.8

9* Frame lines: 9 10

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	=	138.0
Length	(ft)	=	118.0
Eave Height	(ft)	=	21.0/ 24.5
Roof Slope	(rise/12)	=	2.00/ 2.00
Roof Dead Load	(psf)	=	4.0
Wall Dead Load			
Left Endwall	(psf)	=	2.0
Right Endwall	(psf)	=	0.0
Front Sidewall	(psf)	=	2.0
Back Sidewall	(psf)	=	2.0
Roof Live Load	(psf)	=	20.0
Frame Line Load	(psf)	=	12.0
Collateral Load	(psf)	=	10.0
Snow Load	(psf)	=	7.7
Minimum Snow	(psf)	=	11.0
Wind Speed	(mph)	=	115.0
Wind Code		=	IBC 21
Exposure		=	C
Closure		=	Enclosed
Internal Wind Coeff		=	-0.18, +0.18
Risk Category		=	III- High
Importance - Wind		=	1.00
Importance - Seismic		=	1.25
Seismic Design Category		=	C
Seismic Coeff	(Sms)	=	0.46

ID	Description
1	Dead+Collateral+Snow+Snow_Drift
2	Dead+Collateral+0.75Snow+0.45Wind_Left1+0.75Snow_Drift
3	0.6Dead+0.6Wind_Left1
4	0.6Dead+0.6Wind_Right1
5	0.6Dead+0.6Wind_Left2
6	0.6Dead+0.6Wind_Right2
7	0.6Dead+0.6Wind_Long1E
8	0.6Dead+0.6Wind_Long2W
9	1.033Dead+1.03Collateral+0.75Live+0.525Seismic_Left
10	1.033Dead+1.03Collateral+0.75Live+0.525Seismic_Right
11	Dead+Collateral+F1PAT_LL_1
12	Dead+Collateral+F1PAT_LL_2
13	Dead+Collateral+F1PAT_LL_3

RIGHT

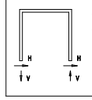
NOTE: THE FRAMING AT ~~LEFT~~ ENDWALL IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THIS FRAME LINE REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

1

ANCHOR BOLT SUMMARY

	Qty	Locale	Dia (in)	Type	Bend Len (in)	Proj (in)
○	28	Jamb	5/8"			
⊠	48	Frame	3/4"	A307	3.00	2.50
⊠	16	WindCol	3/4"	A307	3.00	2.50

WIND BENT REACTIONS



Loc	Wall Line	Col Line	Wind(k) Horz	± Reactions Wind(k) Vert	Seismic(k) Horz	Seismic(k) Vert	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick
F_SW	F	9	9.7	11.7	8.6	10.4	4	0.750	6.000	17.688	0.625
F_SW	F	10	9.7	11.7	8.6	10.4	4	0.750	6.000	17.688	0.625
B_SW	A	10	9.1	9.3	8.6	8.8	4	0.750	6.000	17.688	0.625
B_SW	A	9	9.1	9.3	8.6	8.8	4	0.750	6.000	17.688	0.625

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

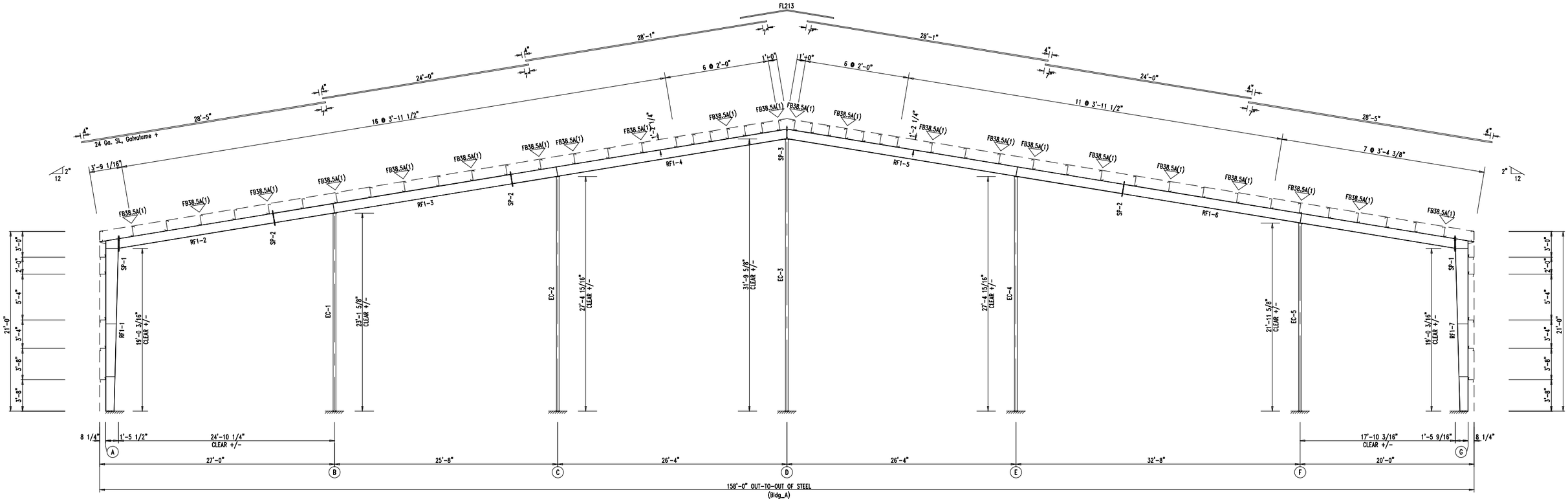
AllMetal Building Systems				
PROJECT	Project Name	ANCHOR BOLT REACTIONS		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	4	0	A325	1.000	2.75
SP-2	4	4	0	A325	0.750	2.00
SP-3	4	2	0	A325	0.750	2.00

CAP PLATE BOLTS				
Mark	Qty	Type	Dia	Length
EC-1	4	A325	0.750	2.00
EC-2	4	A325	0.750	2.00
EC-3	4	A325	0.750	2.00
EC-4	4	A325	0.750	2.00
EC-5	4	A325	0.750	2.00

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A = FBZX1/8

MEMBER TABLE							
Mark	Weight	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	446	19'-10 13/16"	11.2/12.5	0.188	4'-0"	6 x 1/4" x 19'-10 3/16"	6 x 1/4" x 18'-7 15/16"
			12.5/14.4	0.188	6'-2 1/16"	6 x 1/4" x 2'-1 13/16"	
			14.4/17.0	0.188	9'-11"		
RF1-2	449	17'-11 3/4"	W14X22				
RF1-3	680	27'-8 7/16"	W14X22				
RF1-4	766	32'-2 1/4"	W14X22				
RF1-5	927	39'-3 3/8"	W14X22				
RF1-6	924	38'-7 1/16"	W14X22				
RF1-7	468	19'-10 13/16"	17.0/14.3	0.188	9'-11"	6 x 1/4" x 2'-1 13/16"	6 x 5/16" x 18'-7 15/16"
			14.3/12.3	0.188	6'-2 1/16"	6 x 1/4" x 19'-10 3/16"	
			12.3/11.0	0.188	4'-0"		
EC-1	352	23'-1 15/16"	W12X14				
EC-2	470	27'-5 5/16"	W12X16				
EC-3	635	31'-9 5/8"	W12X19				
EC-4	552	27'-5 5/16"	W12X19				
EC-5	333	21'-11 15/16"	W12X14				



RIGID FRAME ELEVATION: FRAME LINE 1

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

6

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

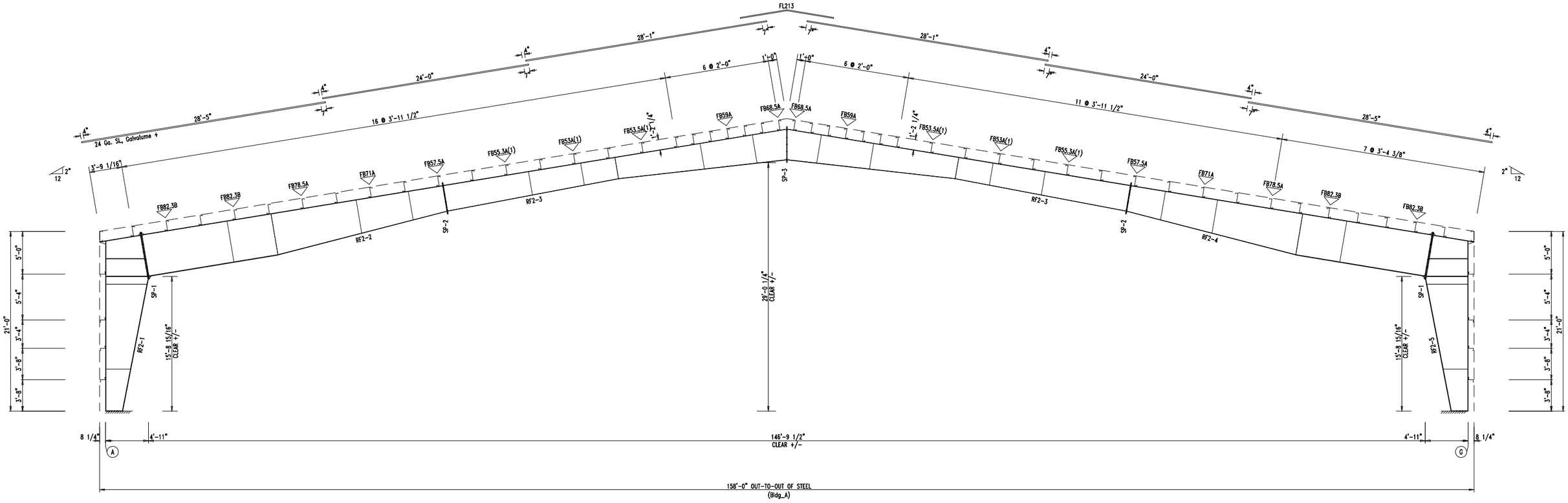
09-04-2025

AllMetal Building Systems					
PROJECT	Project Name	RIGID FRAME ELEVATION			
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:	
PROJECT ADDRESS	Project Address 1	DATE: 9/ 3/25	SHEET	OF	
	Project Address 2				

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	4	8	A325	1.250	4.00
SP-2	4	4	4	A325	0.750	2.00
SP-3	4	4	6	A325	0.750	2.00

▽FLANGE BRACES: Both Sides(U.N.)
FBxxB(1): xx=length(in)
B - FB3X316
A - FB2X1/8

MEMBER TABLE							
Mark	Weight	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF2-1	1825	19'-10 13/16"	22.8/33.9	0.250	4'-10 3/8"	10 x 3/8" x 19'-9 15/16"	10 x 5/8" x 15'-7 11/16"
			33.9/56.6	0.250	9'-11"	10 x 3/8" x 4'-9 1/8"	
			56.6/58.0	0.313	5'-8 9/16"		
RF2-2	2946	35'-2 1/16"	58.0/58.0	0.250	9'-11"	10 x 1/2" x 15'-4 9/16"	10 x 5/8" x 15'-0 9/16"
			58.0/58.0	0.250	5'-1 9/16"	10 x 3/8" x 19'-8"	10 x 3/8" x 18'-0 15/16"
			58.0/47.1	0.250	9'-11"		10 x 3/8" x 2'-0"
			47.1/40.4	0.250	6'-1"		
			40.4/36.0	0.250	4'-0"		
RF2-3	1761	40'-0 15/16"	36.0/33.0	0.188	9'-11"	8 x 3/8" x 20'-0"	8 x 5/16" x 18'-0 1/16"
			33.0/31.2	0.188	6'-1"	8 x 3/8" x 20'-0"	8 x 5/16" x 2'-0"
			31.2/30.0	0.188	4'-0"		8 x 5/16" x 19'-5 1/4"
			30.0/36.1	0.188	9'-11"		
			36.1/39.9	0.188	6'-1"		
			39.9/42.0	0.188	4'-0"		
RF2-4	2948	35'-2 1/16"	36.0/40.4	0.250	4'-0"	10 x 3/8" x 19'-8"	10 x 3/8" x 2'-0"
			40.4/47.1	0.250	6'-1"	10 x 1/2" x 15'-4 9/16"	10 x 3/8" x 18'-0 15/16"
			47.1/58.0	0.250	9'-11"		10 x 5/8" x 15'-0 9/16"
			58.0/58.0	0.250	5'-1 9/16"		
			58.0/58.0	0.250	9'-11"		
RF2-5	1825	19'-10 13/16"	58.0/56.6	0.313	5'-8 9/16"	10 x 3/8" x 4'-9 1/8"	10 x 5/8" x 15'-7 11/16"
			56.6/33.9	0.250	9'-11"	10 x 3/8" x 19'-9 15/16"	
			33.9/22.8	0.250	4'-10 3/8"		



RIGID FRAME ELEVATION: FRAME LINE 2 3 5

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

AllMetal Building Systems				
PROJECT	Project Name	RIGID FRAME ELEVATION		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

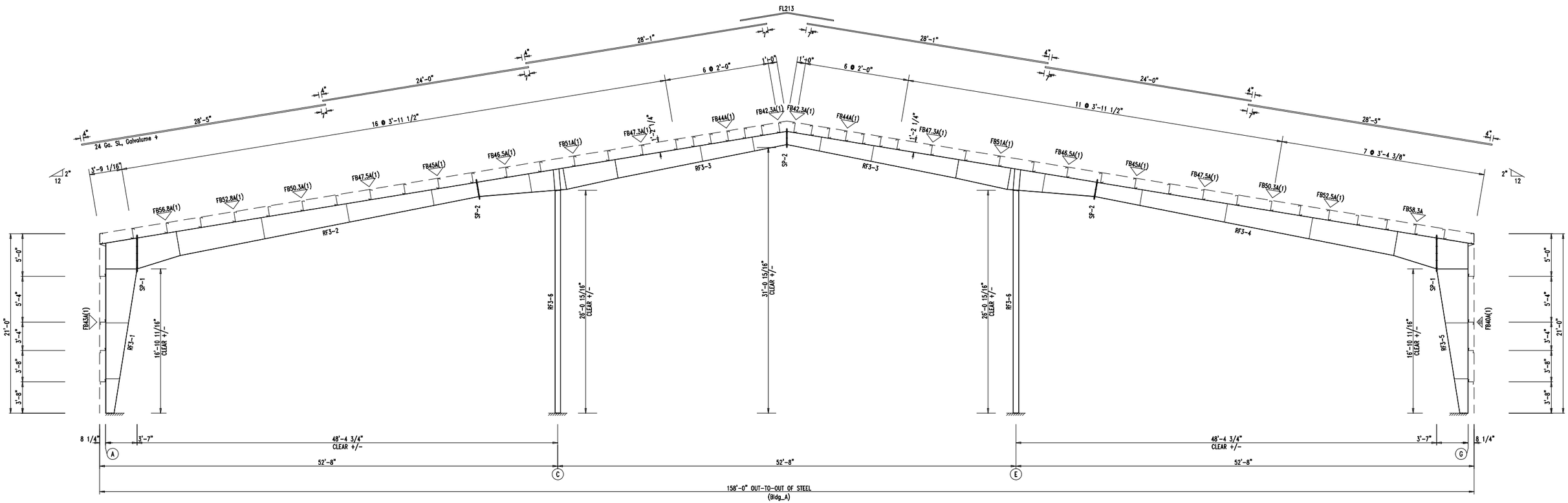
SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	4	6	A325	0.750	2.25
SP-2	4	4	2	A325	0.750	2.00

CAP PLATE BOLTS				
Mark	Qty	Type	Dia	Length
RF3-6	4	A325	0.750	2.00

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A – FBZX1/8

MEMBER TABLE							
Mark	Weight	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF3-1	682	19'-10 15/16"	11.3/18.8	0.188	4'-0"	6 x 1/4" x 19'-10 5/16"	6 x 1/4" x 16'-8 5/8"
			18.8/31.2	0.188	6'-6 7/16"	6 x 1/4" x 4'-3 11/16"	
			31.2/42.5	0.188	9'-11"		
RF3-2	1229	39'-7"	42.5/34.0	0.188	5'-1"	6 x 1/4" x 20'-0"	6 x 1/4" x 5'-1 9/16"
			34.0/30.0	0.188	9'-11"	6 x 1/4" x 19'-5 7/8"	6 x 1/4" x 20'-0"
			30.0/26.1	0.188	9'-11"		6 x 1/4" x 15'-0 3/16"
			26.1/22.1	0.188	9'-11"		
			22.1/20.0	0.188	5'-3"		
RF3-3	1029	36'-1 3/4"	20.0/26.0	0.188	6'-0"	6 x 1/4" x 20'-0"	6 x 5/16" x 10'-0 3/8"
			26.0/30.0	0.188	4'-0"	6 x 1/4" x 16'-0 11/16"	6 x 5/16" x 6'-0 13/16"
			30.0/26.0	0.188	6'-0 11/16"		6 x 1/4" x 19'-9 1/16"
			26.0/22.0	0.188	9'-11"		
			22.0/19.5	0.188	6'-1"		
			19.5/18.0	0.188	4'-0"		
RF3-4	1231	39'-7"	20.0/22.1	0.188	5'-3"	6 x 1/4" x 19'-5 7/8"	6 x 1/4" x 15'-0 3/16"
			22.1/26.1	0.188	9'-11"	6 x 1/4" x 20'-0"	6 x 1/4" x 20'-0"
			26.1/30.0	0.188	9'-11"		6 x 1/4" x 5'-1 9/16"
			30.0/34.0	0.188	9'-11"		
			34.0/42.5	0.188	5'-1"		
RF3-5	682	19'-10 15/16"	42.5/31.2	0.188	9'-11"	6 x 1/4" x 4'-3 11/16"	6 x 1/4" x 16'-8 5/8"
			31.2/18.8	0.188	6'-6 7/16"	6 x 1/4" x 19'-10 5/16"	
			18.8/11.3	0.188	4'-0"		
RF3-6	825	26'-1 1/4"	W8X31				

CONNECTION PLATES	
ID	Mark/Part
1	h1



RIGID FRAME ELEVATION: FRAME LINE 4

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL
09-04-2025

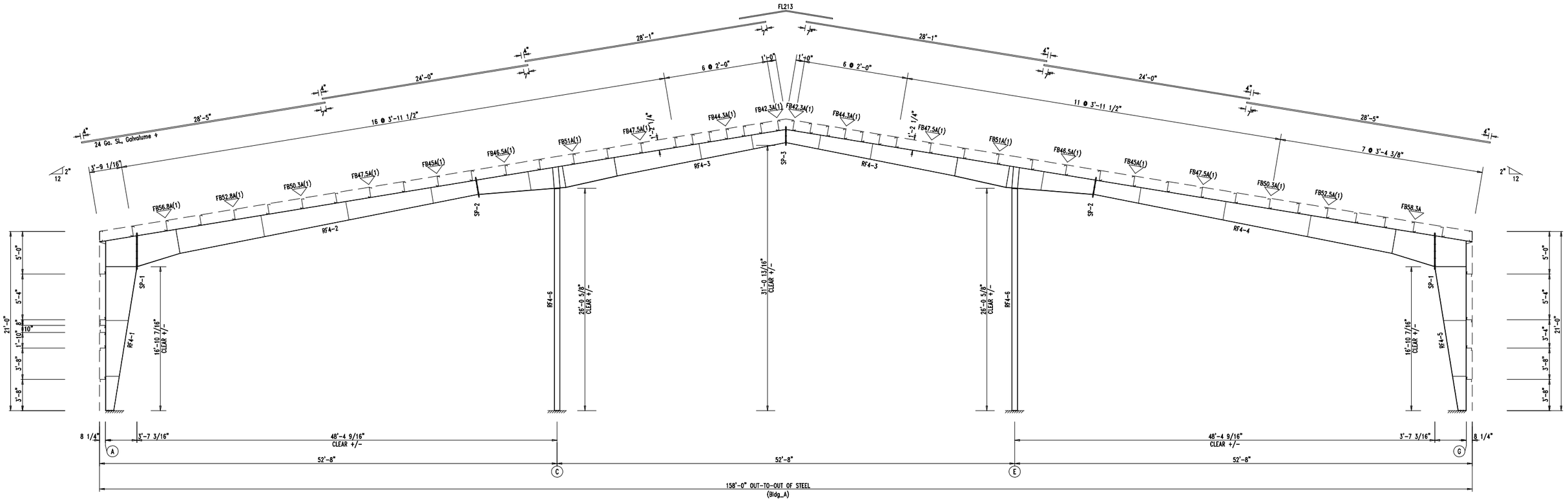
AllMetal Building Systems					
PROJECT	Project Name	RIGID FRAME ELEVATION			
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:	
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF	

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	4	6	A325	1.000	2.50
SP-2	4	4	2	A325	0.750	2.00
SP-3	4	4	2	A325	0.750	2.25

CAP PLATE BOLTS				
Mark	Qty	Type	Dia	Length
RF4-6	4	A325	0.750	2.00

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A – FB2X1/8

MEMBER TABLE						
Mark	Weight	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF4-1	886	19'-10 7/8"	11.0/18.6	0.188	4'-0"	8 x 5/16" x 19'-10 1/16"	8 x 3/8" x 16'-8 3/8"
			18.6/31.1	0.188	6'-6 3/16"	8 x 5/16" x 4'-3 11/16"	
			31.1/42.5	0.188	9'-11"		
RF4-2	1262	39'-6 15/16"	42.5/34.0	0.188	5'-0 15/16"	6 x 1/4" x 20'-0"	6 x 3/8" x 5'-1 1/2"
			34.0/30.0	0.188	9'-11"	6 x 1/4" x 19'-5 13/16"	6 x 1/4" x 20'-0"
			30.0/26.1	0.188	9'-11"		6 x 1/4" x 15'-0 3/16"
			26.1/22.1	0.188	9'-11"		
			22.1/20.0	0.188	5'-3"		
RF4-3	1192	36'-1 3/4"	20.0/26.0	0.188	6'-0"	6 x 3/8" x 20'-0"	6 x 1/2" x 10'-0 3/8"
			26.0/30.0	0.188	4'-0"	6 x 3/8" x 16'-0 9/16"	6 x 1/2" x 6'-0 11/16"
			30.0/26.0	0.188	6'-0 9/16"		6 x 1/4" x 19'-9 1/16"
			26.0/22.0	0.188	9'-11"		
			22.0/19.5	0.188	6'-1"		
			19.5/18.0	0.188	4'-0"		
RF4-4	1265	39'-6 15/16"	20.0/22.1	0.188	5'-3"	6 x 1/4" x 19'-5 13/16"	6 x 1/4" x 15'-0 3/16"
			22.1/26.1	0.188	9'-11"	6 x 1/4" x 20'-0"	6 x 1/4" x 20'-0"
			26.1/30.0	0.188	9'-11"		6 x 3/8" x 5'-1 1/2"
			30.0/34.0	0.188	9'-11"		
			34.0/42.5	0.188	5'-0 15/16"		
RF4-5	882	19'-10 7/8"	42.5/31.1	0.188	9'-11"	8 x 5/16" x 4'-3 11/16"	8 x 3/8" x 16'-8 3/8"
			31.1/18.6	0.188	6'-6 3/16"	8 x 5/16" x 19'-10 1/16"	
			18.6/11.0	0.188	4'-0"		
RF4-6	824	26'-0 15/16"	W8X31				



RIGID FRAME ELEVATION: FRAME LINE 6

09-04-2025

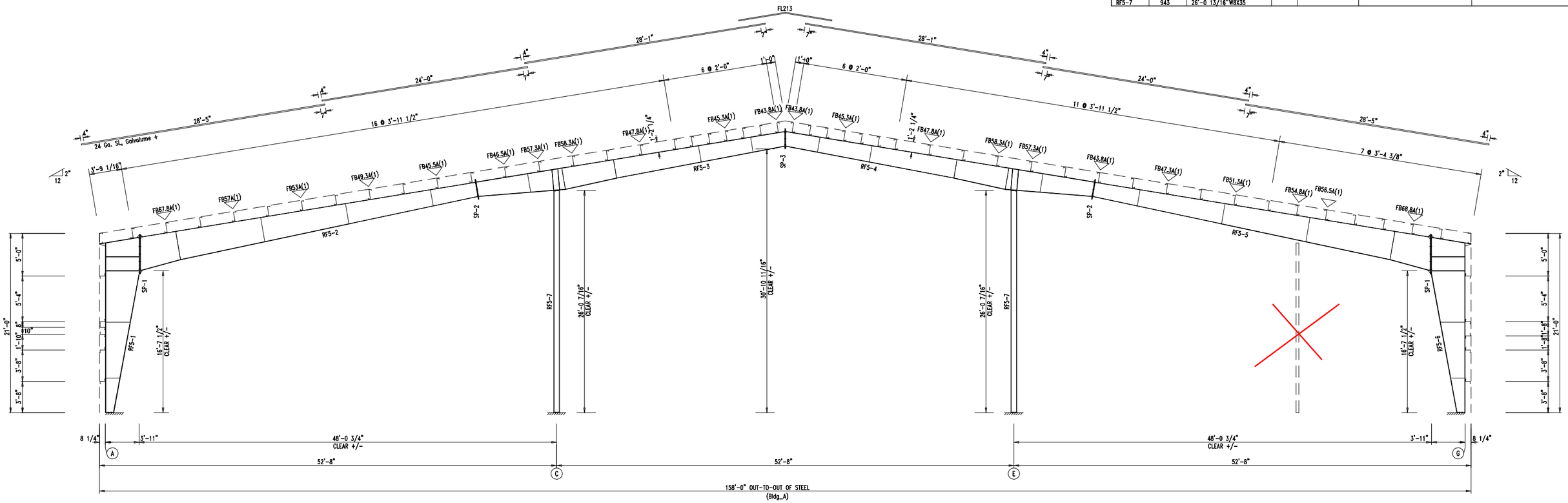
AllMetal Building Systems					
PROJECT	Project Name	RIGID FRAME ELEVATION			
ID	25-0027-1	DESIGN:	DRAFT:		CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25		SHEET	OF

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	4	6	A325	1.250	4.00
SP-2	4	4	2	A325	0.750	2.00
SP-3	4	4	2	A325	0.750	2.25

CAP PLATE BOLTS				
Mark	Qty	Type	Dia	Length
RF5-7	4	A325	0.750	2.25

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A – FB2X1/8

MEMBER TABLE							
Mark	Weight	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF5-1	1732	19'-10 15/16"	10.7/19.4	0.188	4'-0"	14 x 1/2" x 19'-9 13/16"	14 x 1/2" x 16'-5 13/16"
			19.4/33.7	0.188	6'-6 9/16"	14 x 1/2" x 4'-7 7/16"	
			33.7/46.0	0.188	9'-11"		
RF5-2	1538	39'-2 13/16"	46.0/40.0	0.188	4'-9"	6 x 1/4" x 20'-0"	6 x 3/8" x 4'-9 5/16"
			40.0/34.3	0.188	9'-11"	6 x 1/4" x 19'-1 5/16"	6 x 3/8" x 18'-3 1/4"
			34.3/28.7	0.188	9'-11"		6 x 3/8" x 2'-0"
			28.7/23.0	0.188	9'-11"		6 x 1/4" x 14'-9 3/16"
			23.0/20.0	0.188	5'-3"		
RF5-3	1410	36'-1 3/4"	20.0/26.0	0.188	6'-0"	8 x 3/8" x 10'-4"	8 x 1/2" x 10'-0 3/8"
			26.0/30.0	0.188	4'-0"	8 x 5/16" x 20'-0"	8 x 1/2" x 6'-0 11/16"
			30.0/26.0	0.188	6'-0 9/16"	8 x 5/16" x 5'-8 9/16"	8 x 5/16" x 19'-8 11/16"
			26.0/23.0	0.188	9'-11"		
			23.0/21.1	0.188	6'-1"		
			21.1/20.0	0.188	4'-0"		
RF5-4	1410	36'-1 3/4"	20.0/21.1	0.188	4'-0"	8 x 5/16" x 5'-8 9/16"	8 x 5/16" x 19'-8 11/16"
			21.1/23.0	0.188	6'-1"	8 x 5/16" x 20'-0"	8 x 1/2" x 6'-0 11/16"
			23.0/26.0	0.188	9'-11"	8 x 3/8" x 10'-4"	8 x 1/2" x 10'-0 3/8"
			26.0/30.0	0.188	6'-0 9/16"		
			30.0/26.0	0.188	4'-0"		
			26.0/20.0	0.188	6'-0"		
RF5-5	1543	39'-2 13/16"	20.0/23.0	0.188	5'-3"	6 x 1/4" x 19'-1 5/16"	6 x 1/4" x 14'-9 3/16"
			23.0/28.7	0.188	9'-11"	6 x 1/4" x 20'-0"	6 x 3/8" x 2'-9"
			28.7/34.3	0.188	9'-11"		6 x 3/8" x 18'-3 1/4"
			34.3/40.0	0.188	9'-11"		6 x 3/8" x 4'-9 5/16"
			40.0/46.0	0.188	4'-9"		
RF5-6	1732	19'-10 15/16"	46.0/33.7	0.188	9'-11"	14 x 1/2" x 4'-7 7/16"	14 x 1/2" x 16'-5 13/16"
			33.7/19.4	0.188	6'-6 9/16"	14 x 1/2" x 19'-9 13/16"	
			19.4/10.7	0.188	4'-0"		
RF5-7	943	26'-0 13/16"	W8X35				



RIGID FRAME ELEVATION: FRAME LINE 7

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

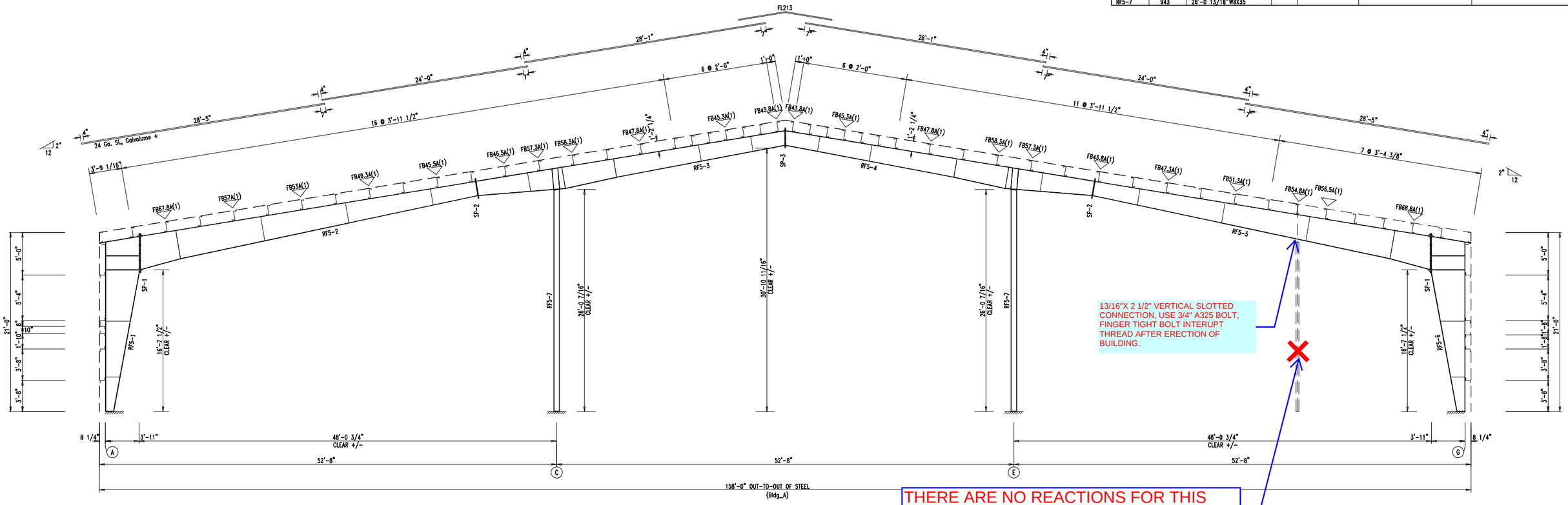
AllMetal Building Systems					
PROJECT	Project Name	RIGID FRAME ELEVATION			
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:	
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF	

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	4	6	A325	1.250	4.00
SP-2	4	4	2	A325	0.750	2.00
SP-3	4	4	2	A325	0.750	2.25

CAP PLATE BOLTS				
Mark	Qty	Type	Dia	Length
RF5-7	4	A325	0.750	2.25

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A – FB2X1/8

MEMBER TABLE							
Mark	Weight	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF5-1	1732	19'-10 15/16	10.7/19.4	0.188	4'-0"	14 x 1/2" x 19'-9 13/16"	14 x 1/2" x 16'-5 13/16"
			19.4/33.7	0.188	6'-6 9/16"	14 x 1/2" x 4'-7 7/16"	
			33.7/46.0	0.188	9'-11"		
RF5-2	1538	39'-2 13/16"	46.0/40.0	0.188	4'-9"	6 x 1/4" x 20'-0"	6 x 3/8" x 4'-9 5/16"
			40.0/34.3	0.188	9'-11"	6 x 1/4" x 19'-1 5/16"	6 x 3/8" x 18'-3 1/4"
			34.3/28.7	0.188	9'-11"		6 x 3/8" x 2'-0"
			28.7/23.0	0.188	9'-11"		6 x 1/4" x 14'-9 3/16"
			23.0/20.0	0.188	5'-3"		
RF5-3	1410	36'-1 3/4"	20.0/26.0	0.188	6'-0"	8 x 3/8" x 10'-4"	8 x 1/2" x 10'-0 3/8"
			26.0/30.0	0.188	4'-0"	8 x 5/16" x 20'-0"	8 x 1/2" x 6'-0 11/16"
			30.0/26.0	0.188	6'-0 9/16"	8 x 5/16" x 5'-8 9/16"	8 x 5/16" x 19'-8 11/16"
			26.0/23.0	0.188	9'-11"		
			23.0/21.1	0.188	6'-1"		
			21.1/20.0	0.188	4'-0"		
RF5-4	1410	36'-1 3/4"	20.0/21.1	0.188	4'-0"	8 x 5/16" x 5'-8 9/16"	8 x 5/16" x 19'-8 11/16"
			21.1/23.0	0.188	6'-1"	8 x 5/16" x 20'-0"	8 x 1/2" x 6'-0 11/16"
			23.0/26.0	0.188	9'-11"	8 x 3/8" x 10'-4"	8 x 1/2" x 10'-0 3/8"
			26.0/30.0	0.188	6'-0 9/16"		
			30.0/26.0	0.188	4'-0"		
			26.0/20.0	0.188	6'-0"		
RF5-5	1543	39'-2 13/16"	20.0/23.0	0.188	5'-3"	6 x 1/4" x 19'-1 5/16"	6 x 1/4" x 14'-9 3/16"
			23.0/28.7	0.188	9'-11"	6 x 1/4" x 20'-0"	6 x 3/8" x 2'-9"
			28.7/34.3	0.188	9'-11"		6 x 3/8" x 18'-3 1/4"
			34.3/40.0	0.188	9'-11"		6 x 3/8" x 4'-9 5/16"
			40.0/46.0	0.188	4'-9"		
RF5-6	1732	19'-10 15/16	46.0/33.7	0.188	9'-11"	14 x 1/2" x 4'-7 7/16"	14 x 1/2" x 16'-5 13/16"
			33.7/19.4	0.188	6'-6 9/16"	14 x 1/2" x 19'-9 13/16"	
			19.4/10.7	0.188	4'-0"		
RF5-7	943	26'-0 13/16"	W8X35				



13/16"X 2 1/2" VERTICAL SLOTTED CONNECTION, USE 3/4" A325 BOLT, FINGER TIGHT BOLT INTERUPT THREAD AFTER ERECTION OF BUILDING.

THERE ARE NO REACTIONS FOR THIS COLUMN. A FOOTING HAS NOT BEEN PROVIDED. REMOVE COLUMN IF COLUMN IS REQUIRED PROVIDE REACTIONS FOR FOOTING DESIGN. --SDG

RIGID FRAME ELEVATION: FRAME LINE 8

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

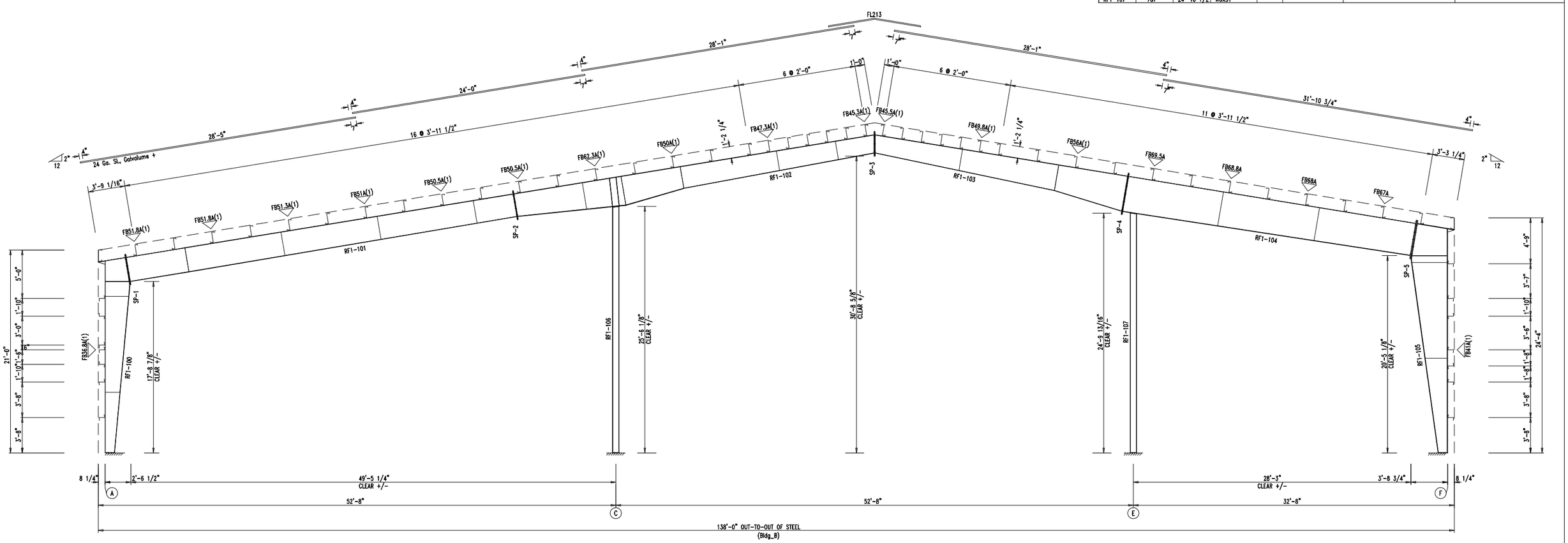
09-04-2025

AllMetal Building Systems					
PROJECT	Project Name	RIGID FRAME ELEVATION			
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:	
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF	

SPLICE BOLT TABLE							CAP PLATE BOLTS				
Mark	Qty Top	Bot	Int	Type	Dia	Length	Mark	Qty	Type	Dia	Length
SP-1	4	4	4	A325	0.750	2.25	RF1-106	4	A325	0.750	2.25
SP-2	4	4	4	A325	0.750	2.00	RF1-107	4	A325	0.750	2.00
SP-3	4	4	2	A325	0.750	2.25					
SP-4	4	4	6	A325	0.750	2.25					
SP-5	4	4	6	A325	1.000	3.25					

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A - FB2X1/8

MEMBER TABLE						
Mark	Weight	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-100	559	19'-10 11/16"	11.3/18.0	0.188	6'-3"	6 x 1/4" x 19'-9 15/16"	6 x 1/4" x 17'-5 3/16"
			18.0/28.7	0.188	9'-11"	6 x 1/4" x 2'-9 5/16"	
			28.7/30.0	0.188	4'-0"		
RF1-101	1290	40'-0 1/16"	30.0/30.0	0.188	6'-0 9/16"	6 x 1/4" x 5'-8 9/16"	6 x 1/4" x 6'-0 9/16"
			30.0/29.4	0.188	9'-11"	6 x 5/16" x 20'-0"	6 x 1/4" x 20'-0"
			29.4/28.8	0.188	9'-11"	6 x 5/16" x 14'-2 3/8"	6 x 1/4" x 13'-10 3/8"
			28.8/28.2	0.188	9'-11"		
			28.2/28.0	0.188	4'-1 3/8"		
RF1-102	1316	37'-3 1/16"	28.0/33.1	0.188	7'-0 5/8"	6 x 5/16" x 20'-0"	6 x 1/2" x 11'-0 7/8"
			33.1/36.0	0.188	4'-0"	6 x 5/16" x 17'-1 7/8"	6 x 1/2" x 6'-1 13/16"
			36.0/27.0	0.188	6'-1 1/4"		6 x 5/16" x 19'-8 3/8"
			27.0/24.5	0.188	9'-11"		
			24.5/22.9	0.188	6'-1"		
			22.9/22.0	0.188	4'-0"		
RF1-103	904	26'-1 15/16"	22.0/26.7	0.188	9'-0 5/16"	6 x 5/16" x 19'-3 5/16"	6 x 1/4" x 18'-7 3/4"
			26.7/32.0	0.188	9'-11"	6 x 1/4" x 6'-9 3/8"	6 x 5/16" x 7'-2 1/8"
			32.0/43.7	0.188	7'-1 3/8"		
RF1-104	1289	29'-9"	43.7/42.5	0.188	9'-9 3/8"	6 x 1/4" x 9'-7 3/8"	6 x 5/16" x 9'-7 3/8"
			42.5/41.3	0.188	9'-11"	6 x 1/4" x 20'-0"	6 x 5/16" x 20'-0"
			41.3/40.0	0.188	9'-11"		
RF1-105	1049	23'-2 15/16"	43.8/43.1	0.188	4'-0"	6 x 1/2" x 3'-9 13/16"	6 x 1/2" x 2'-0"
			43.1/26.8	0.188	9'-11"	6 x 1/2" x 3'-1 15/16"	6 x 1/2" x 18'-2 3/4"
			26.8/10.8	0.188	9'-9 1/16"	6 x 1/2" x 20'-0"	
RF1-106	920	25'-6 9/16"	W8X35				
RF1-107	787	24'-10 1/2"	W8X31				



RIGID FRAME ELEVATION: FRAME LINE 9 10

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

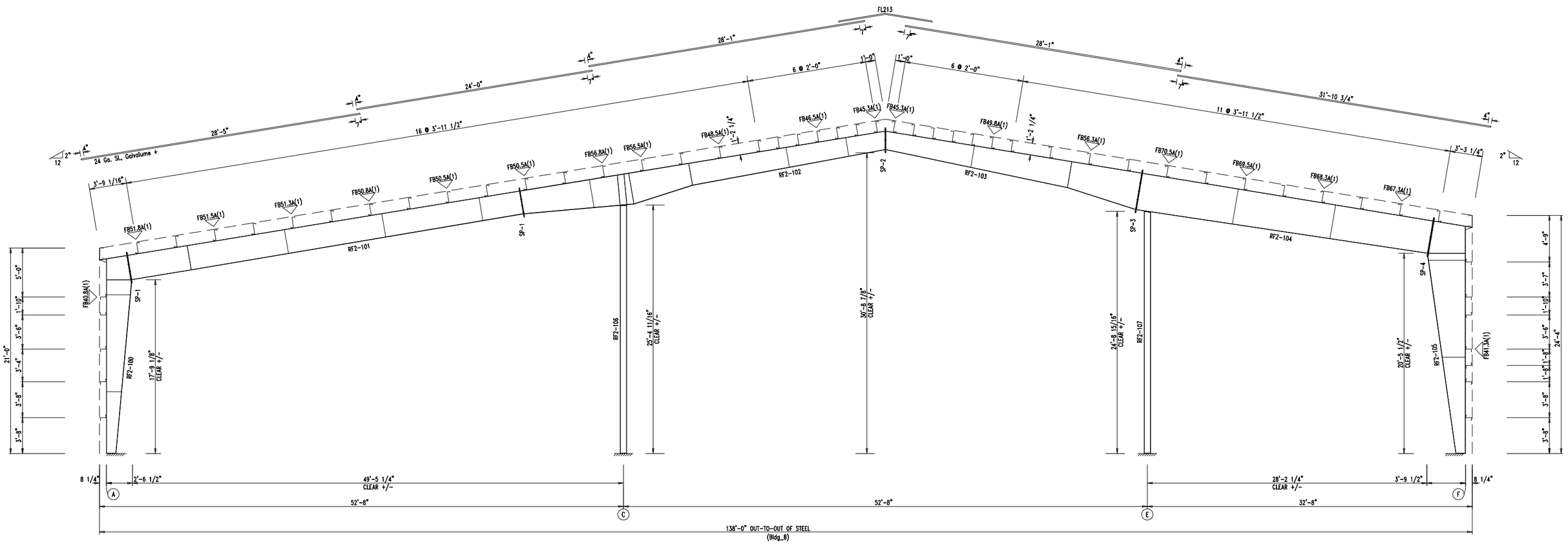
AllMetal Building Systems					
PROJECT	Project Name	RIGID FRAME ELEVATION			
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:	
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF	

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	4	4	A325	0.750	2.00
SP-2	4	4	2	A325	0.750	2.00
SP-3	4	4	6	A325	0.750	2.00
SP-4	4	4	6	A325	0.750	2.25

CAP PLATE BOLTS				
Mark	Qty	Type	Dia	Length
RF2-106	4	A325	0.750	2.00
RF2-107	4	A325	0.750	2.00

FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A = FB2X1/8

MEMBER TABLE							
Mark	Weight	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF2-100	544	19'-10 15/16"	11.3/18.0	0.188	6'-3 7/16"	6 x 1/4" x 19'-10 5/16"	6 x 1/4" x 17'-5 9/16"
			18.0/28.7	0.188	9'-11"	6 x 1/4" x 2'-9 5/16"	
			28.7/30.0	0.188	4'-0"		
RF2-101	1230	40'-0 1/8"	30.0/30.0	0.188	6'-0 13/16"	6 x 1/4" x 20'-0"	6 x 1/4" x 6'-0 13/16"
			30.0/29.4	0.188	9'-11"	6 x 1/4" x 19'-11 1/4"	6 x 1/4" x 20'-0"
			29.4/28.8	0.188	9'-11"		6 x 1/4" x 13'-10 1/2"
			28.8/28.2	0.188	9'-11"		
			28.2/28.0	0.188	4'-1 1/2"		
RF2-102	1199	37'-3 1/8"	28.0/34.4	0.188	7'-0 3/4"	6 x 1/4" x 20'-0"	6 x 3/8" x 11'-1 1/8"
			34.4/38.0	0.188	4'-0"	6 x 1/4" x 17'-2 3/16"	6 x 3/8" x 6'-2 1/4"
			38.0/27.0	0.188	6'-1 7/16"		6 x 1/4" x 19'-8 3/8"
			27.0/24.5	0.188	9'-11"		
			24.5/22.9	0.188	6'-1"		
			22.9/22.0	0.188	4'-0"		
RF2-103	858	26'-2"	22.0/26.7	0.188	9'-0 7/16"	6 x 1/4" x 6'-0 15/16"	6 x 1/4" x 18'-7 7/8"
			26.7/32.0	0.188	9'-11"	6 x 1/4" x 20'-0"	6 x 1/4" x 7'-2 7/16"
			32.0/44.9	0.188	7'-1 1/2"		
RF2-104	1218	29'-8 5/16"	45.0/43.3	0.188	9'-9 3/16"	6 x 1/4" x 9'-7 3/16"	6 x 1/4" x 9'-7 1/4"
			43.3/41.7	0.188	9'-11"	6 x 1/4" x 20'-0"	6 x 1/4" x 20'-0"
			41.7/40.0	0.188	9'-11"		
RF2-105	787	23'-2 15/16"	45.0/44.4	0.188	4'-0"	6 x 1/4" x 3'-10 13/16"	6 x 1/4" x 2'-0"
			44.4/27.7	0.188	9'-11"	6 x 1/4" x 3'-2 5/16"	6 x 1/4" x 18'-3 7/16"
			27.7/11.3	0.188	9'-9 5/8"	6 x 1/4" x 20'-0"	
RF2-106	808	25'-5 1/16"	W8X31				
RF2-107	784	24'-9 9/16"	W8X31				



RIGID FRAME ELEVATION: FRAME LINE 11

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

6

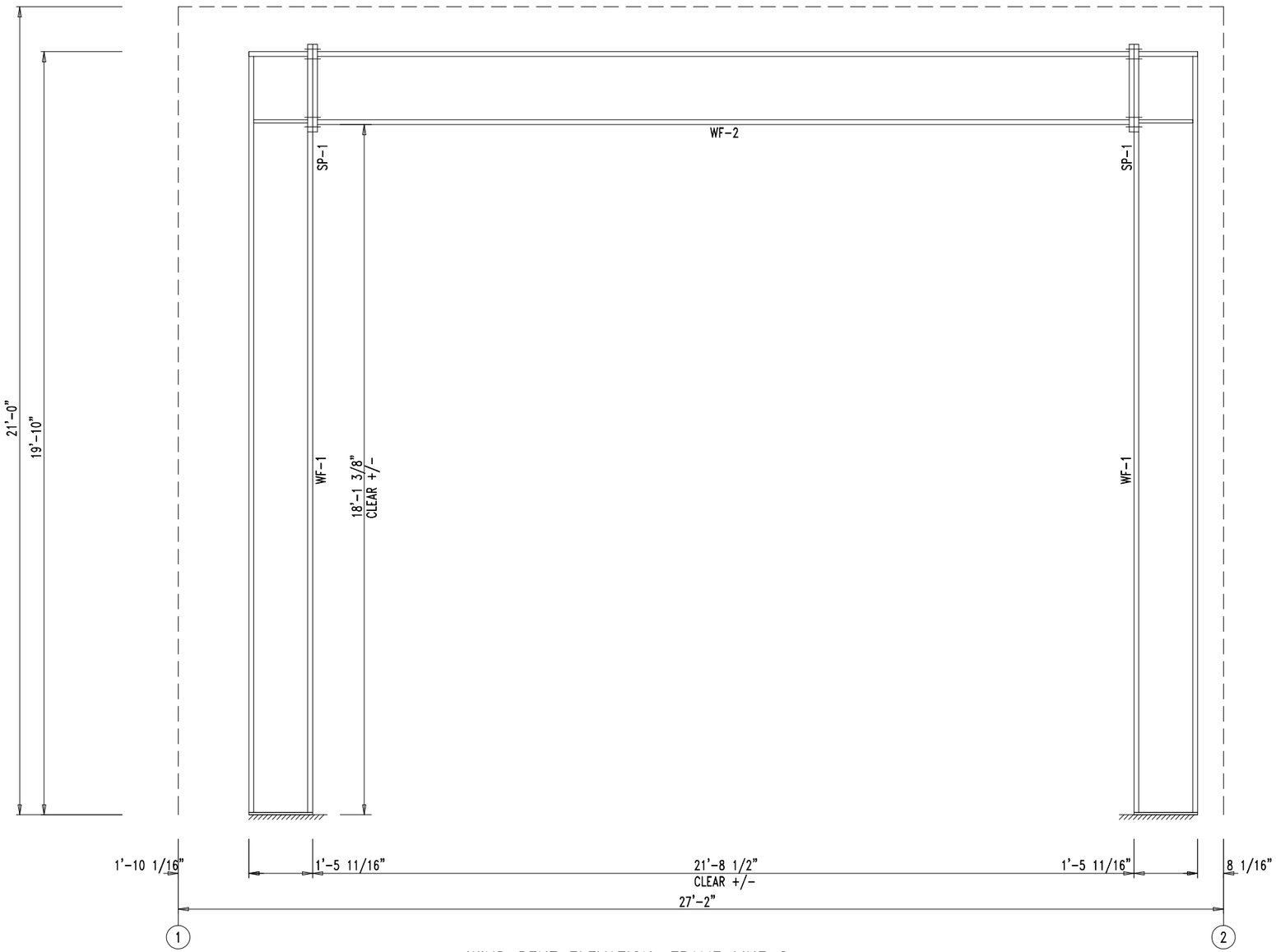
ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

AllMetal Building Systems				
PROJECT	Project Name	RIGID FRAME ELEVATION		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

SPLICE BOLTS					
Splice Mark	Quan Top/	Bot	-----Bolt----- Type	Dia	Length
SP- 1	4	4	A325	1.250	4.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-2	W21X48	21'-8 1/2"
WF-1	W18X35	19'-10"



WIND BENT ELEVATION: FRAME LINE G

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

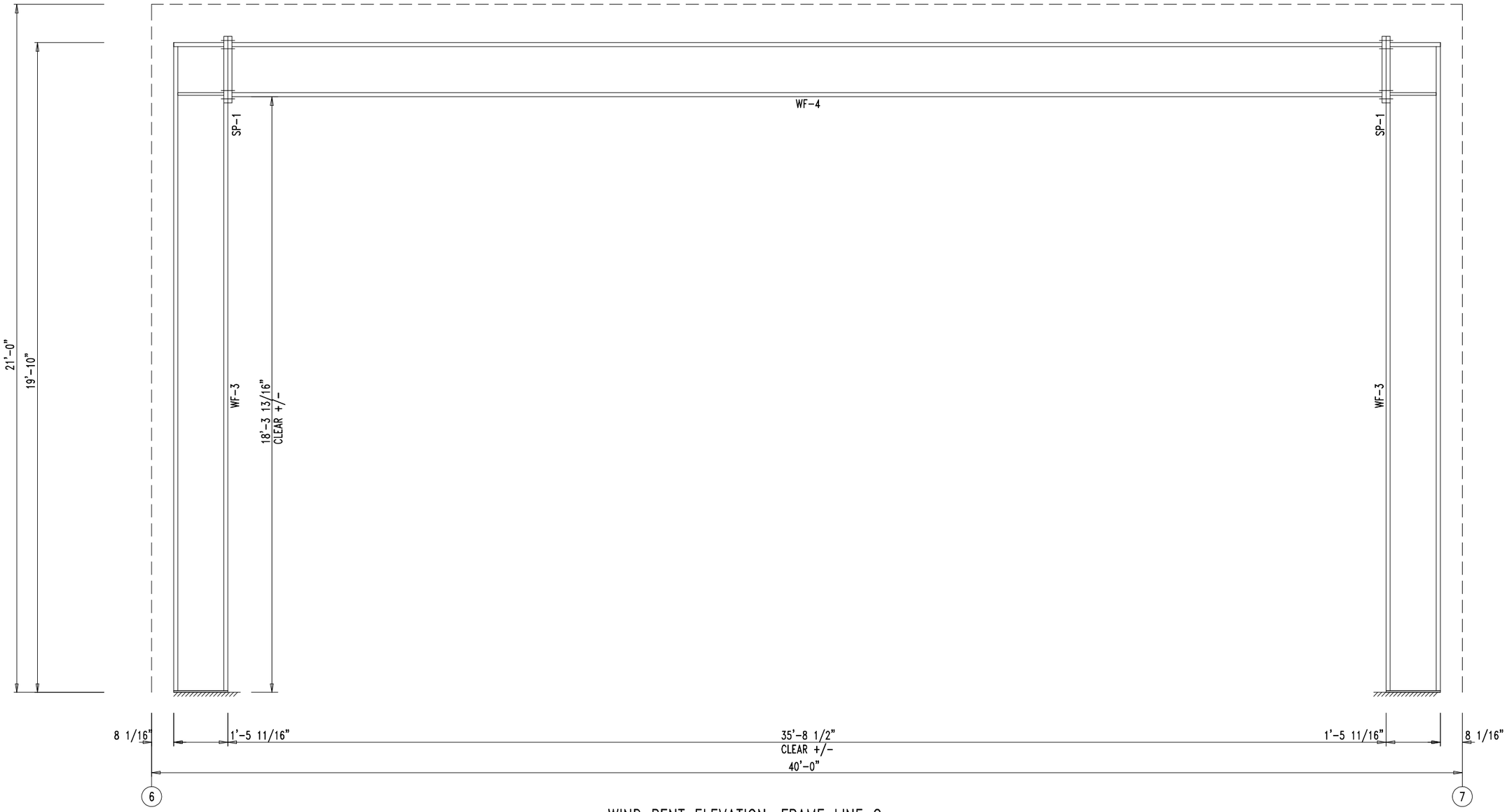
09-04-2025

ALLMETAL BUILDING SYSTEMS
1943 East Malvern Hwy, Dothan Al 36305
(334) 792-6121 (334) 886-3545 (fax)

DESCRIPTION: @DRAWING						
CUSTOMER: Customer					PROJECT: Project Name	
LOCATION: Project Address 1						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
ME	BL	9/ 3/25	N.T.S.		Bldg_A	OF

SPLICE BOLTS					
Splice Mark	Quan Top/	Bot	-----Bolt----- Type	Dia	Length
SP- 1	4	4	A325	1.500	5.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-4	W18X76	35'-8 1/2"
WF-3	W18X35	19'-10"



WIND BENT ELEVATION: FRAME LINE G

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

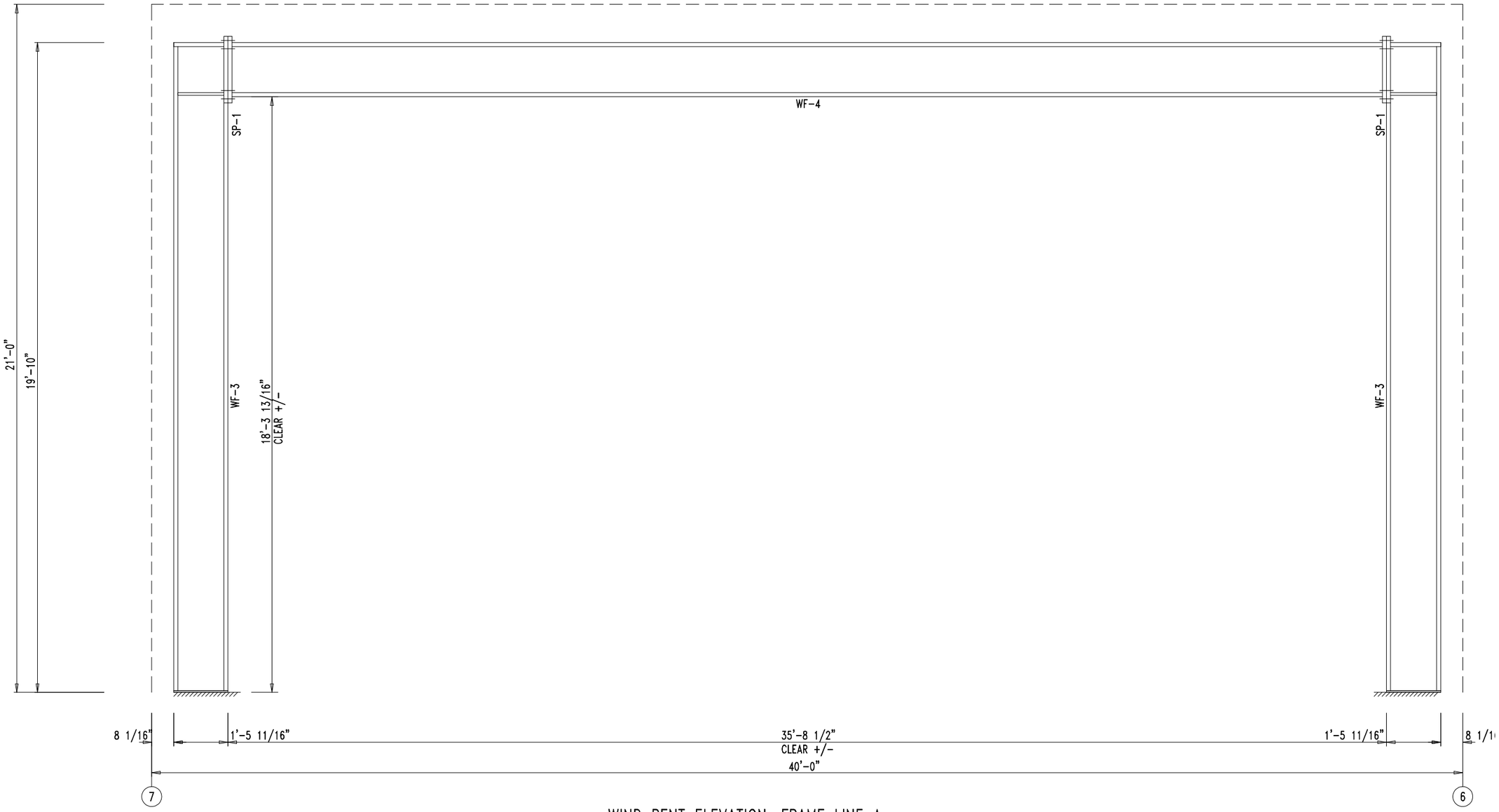
09-04-2025

ALLMETAL BUILDING SYSTEMS
1943 East Malvern Hwy, Dothan Al 36305
(334) 792-6121 (334) 886-3545 (fax)

DESCRIPTION: @DRAWING						
CUSTOMER: Customer					PROJECT: Project Name	
LOCATION: Project Address 1						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
ME	BL	9/ 3/25	N.T.S.		Bldg_A	OF

SPLICE BOLTS					
Splice Mark	Quan Top/	Bot	-----Bolt----- Type	Dia	Length
SP- 1	4	4	A325	1.500	5.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-4	W18X76	35'-8 1/2"
WF-3	W18X35	19'-10"



WIND BENT ELEVATION: FRAME LINE A

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

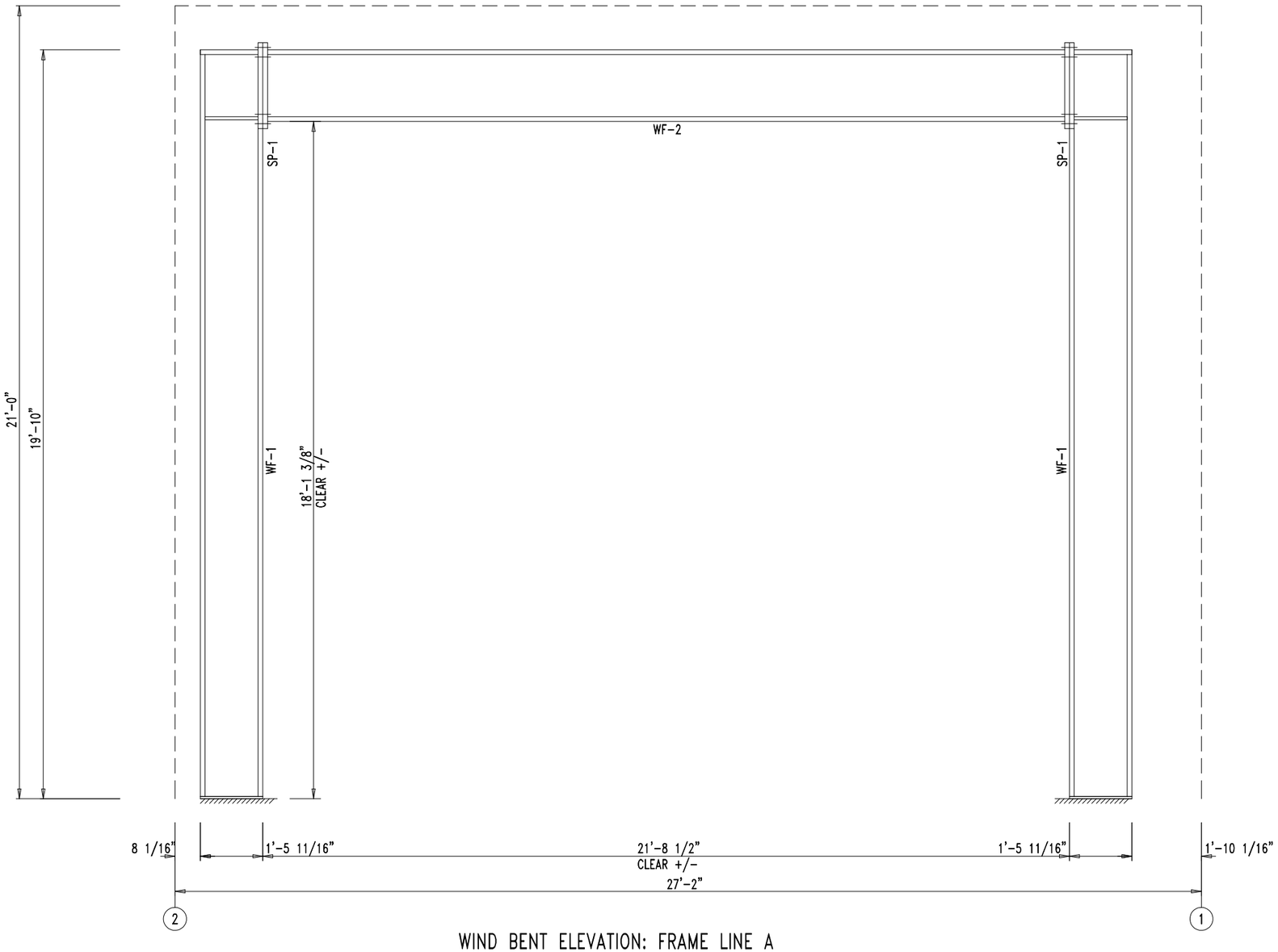
09-04-2025

ALLMETAL BUILDING SYSTEMS
1943 East Malvern Hwy, Dothan Al 36305
(334) 792-6121 (334) 886-3545 (fax)

DESCRIPTION: @DRAWING						
CUSTOMER: Customer					PROJECT: Project Name	
LOCATION: Project Address 1						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
ME	BL	9/ 3/25	N.T.S.		Bldg_A	OF

SPLICE BOLTS					
Splice Mark	Quan Top/	Bot	-----Bolt----- Type	Dia	Length
SP- 1	4	4	A325	1.250	4.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-2	W21X48	21'-8 1/2"
WF-1	W18X35	19'-10"



WIND BENT ELEVATION: FRAME LINE A

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

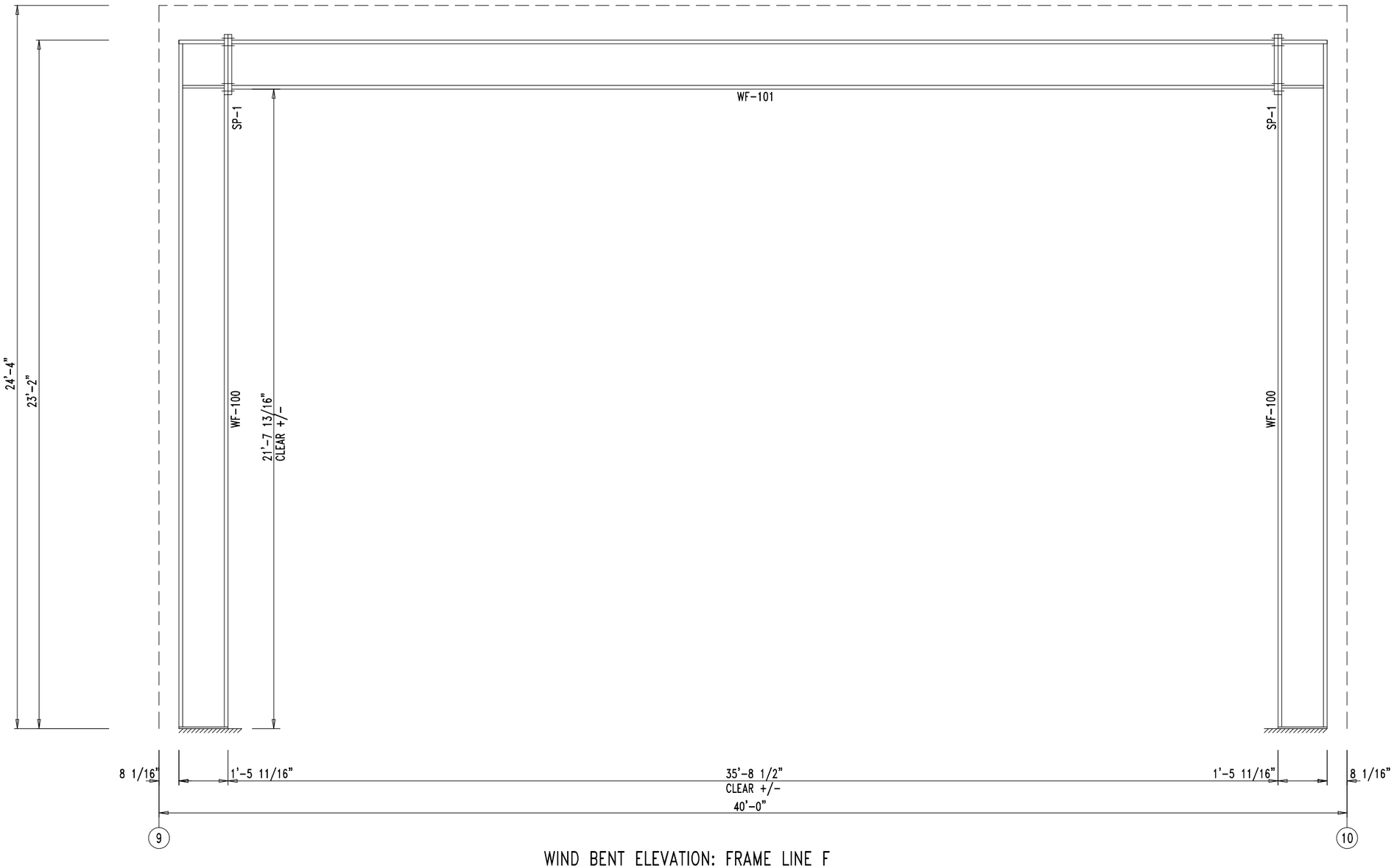
09-04-2025

ALLMETAL BUILDING SYSTEMS
1943 East Malvern Hwy, Dothan Al 36305
(334) 792-6121 (334) 886-3545 (fax)

DESCRIPTION: @DRAWING						
CUSTOMER: Customer					PROJECT: Project Name	
LOCATION: Project Address 1						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
ME	BL	9/ 3/25	N.T.S.		Bldg_A	OF

SPLICE BOLTS					
Splice Mark	Quan Top/	Bot	-----Bolt----- Type	Dia	Length
SP- 1	4	4	A325	1.500	5.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-101	W18X76	35'-8 1/2"
WF-100	W18X35	23'-2"



WIND BENT ELEVATION: FRAME LINE F

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

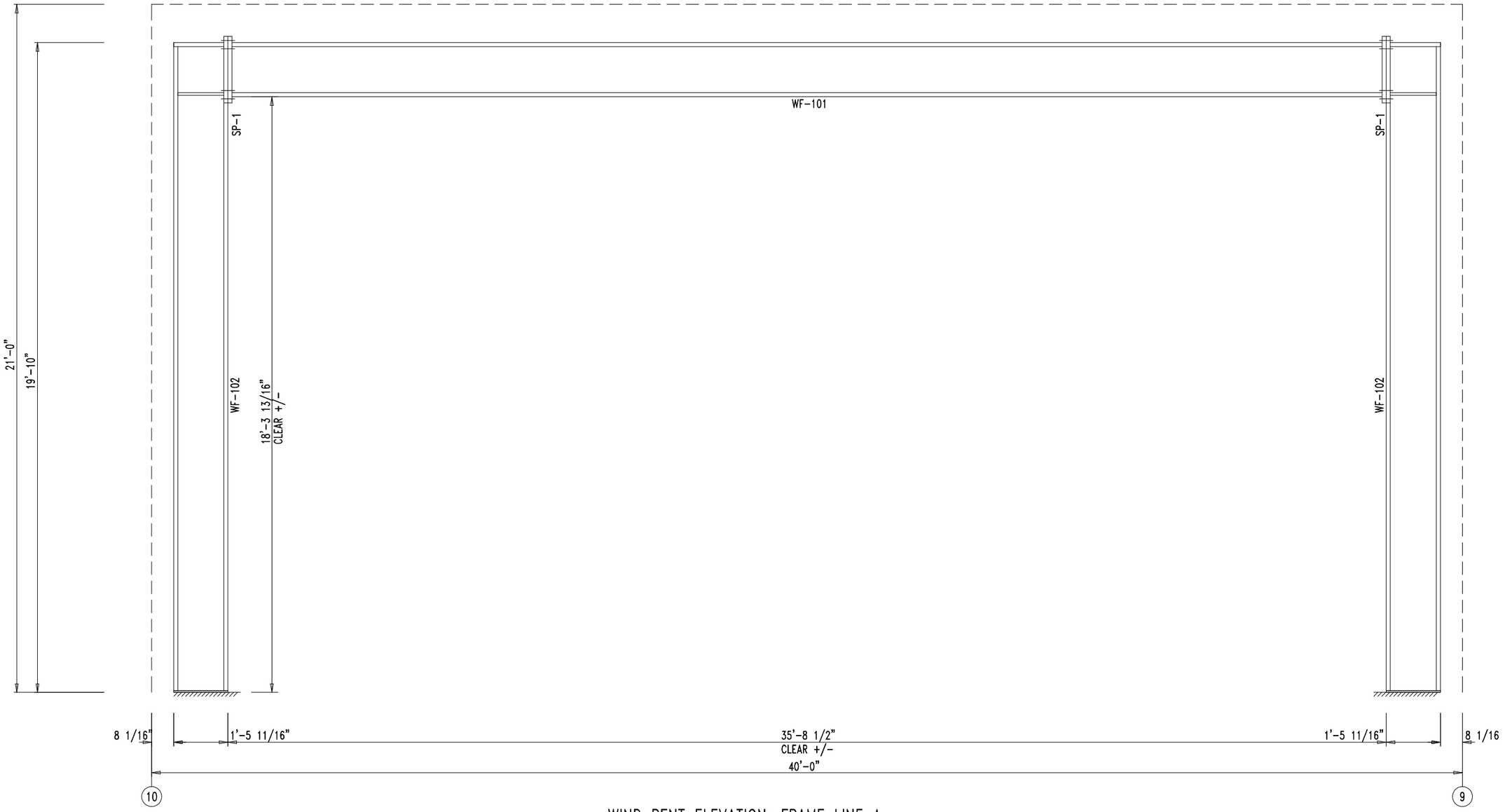
09-04-2025

ALLMETAL BUILDING SYSTEMS
1943 East Malvern Hwy, Dothan Al 36305
(334) 792-6121 (334) 886-3545 (fax)

DESCRIPTION: @DRAWING						
CUSTOMER: Customer					PROJECT: Project Name	
LOCATION: Project Address 1						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
ME	BL	9/ 3/25	N.T.S.		Bldg_B	OF

SPLICE BOLTS					
Splice Mark	Quan Top/	Bot	-----Bolt----- Type	Dia	Length
SP- 1	4	4	A325	1.500	5.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-101	W18X76	35'-8 1/2"
WF-102	W18X35	19'-10"



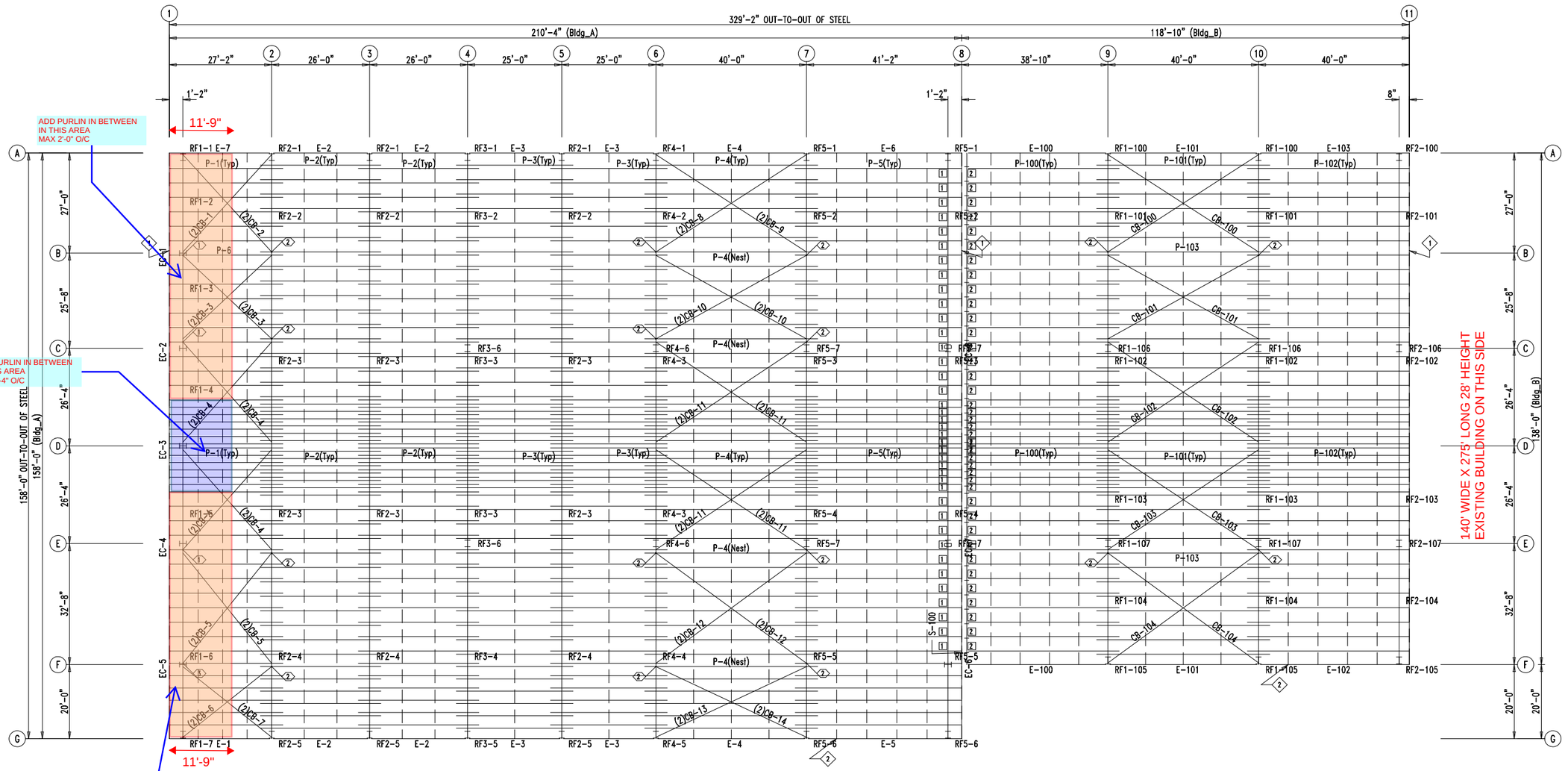
WIND BENT ELEVATION: FRAME LINE A

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

ALLMETAL BUILDING SYSTEMS
1943 East Malvern Hwy, Dothan Al 36305
(334) 792-6121 (334) 886-3545 (fax)

DESCRIPTION: @DRAWING						
CUSTOMER: Customer					PROJECT: Project Name	
LOCATION: Project Address 1						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
ME	BL	9/ 3/25	N.T.S.		Bldg_B	OF



ROOF FRAMING PLAN

SPECIAL BOLTS ROOF PLAN					
◇ ID	QUAN	TYPE	DIA	LENGTH	WASH
1	2	A325	7/8"	2 1/4"	2
2	2	A325	1/2"	1 1/4"	0
3	4	A325	7/8"	2 1/4"	2

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
Bldg_A		
P-1	14X35Z14	30'-1 3/4"
P-2	14X35Z14	32'-0"
P-3	14X35Z14	31'-0"
P-4	14X35Z12	46'-0"
P-5	14X35Z12	44'-1 3/4"
P-6	14X35Z12	30'-1 3/4"
E-1	E145341L	27'-1 1/2"
E-2	E145341L	25'-11 1/2"
E-3	E145341L	24'-11 1/2"
E-4	E145341L	39'-11 1/2"
E-5	E145341L	41'-1 1/2"
E-6	E145341L	41'-1 1/2"
E-7	E145341L	27'-1 1/2"
CB-1	3/8_CBL	36'-10 3/4"
CB-2	3/8_CBL	36'-11"
CB-3	3/8_CBL	34'-4 3/4"
CB-4	1/4_CBL	37'-8 1/2"
CB-5	3/8_CBL	39'-6"
CB-6	3/8_CBL	32'-2 3/4"
CB-7	3/8_CBL	32'-2 1/2"
CB-8	3/8_CBL	47'-11"
CB-9	3/8_CBL	47'-11 1/4"
CB-10	3/8_CBL	46'-0 1/4"
CB-11	1/4_CBL	48'-6 3/4"
CB-12	3/8_CBL	49'-11 1/2"
CB-13	3/8_CBL	44'-5 1/4"
CB-14	3/8_CBL	44'-5"
Bldg_B		
P-100	14X35Z12	41'-9 3/4"
P-101	14X35Z12	46'-0"
P-102	14X35Z10	42'-11 3/4"
P-103	14X35Z10	46'-0"
S-100	14X25Z14	7"
E-100	E145341L	38'-9 1/2"
E-101	E145341L	39'-11 1/2"
E-102	E145341L	39'-11 1/2"
E-103	E145341L	39'-11 1/2"
CB-100	1/2_CBL	47'-11 1/4"
CB-101	HW375	46'-1 1/4"
CB-102	HW373	48'-6 3/4"
CB-103	HW374	48'-6 3/4"
CB-104	1/2_CBL	49'-10 1/2"

ANGLE TABLE ROOF PLAN		
◇ID	MARK	LENGTH
1	L4x2	20'-0"
2	FL569	500'-0"

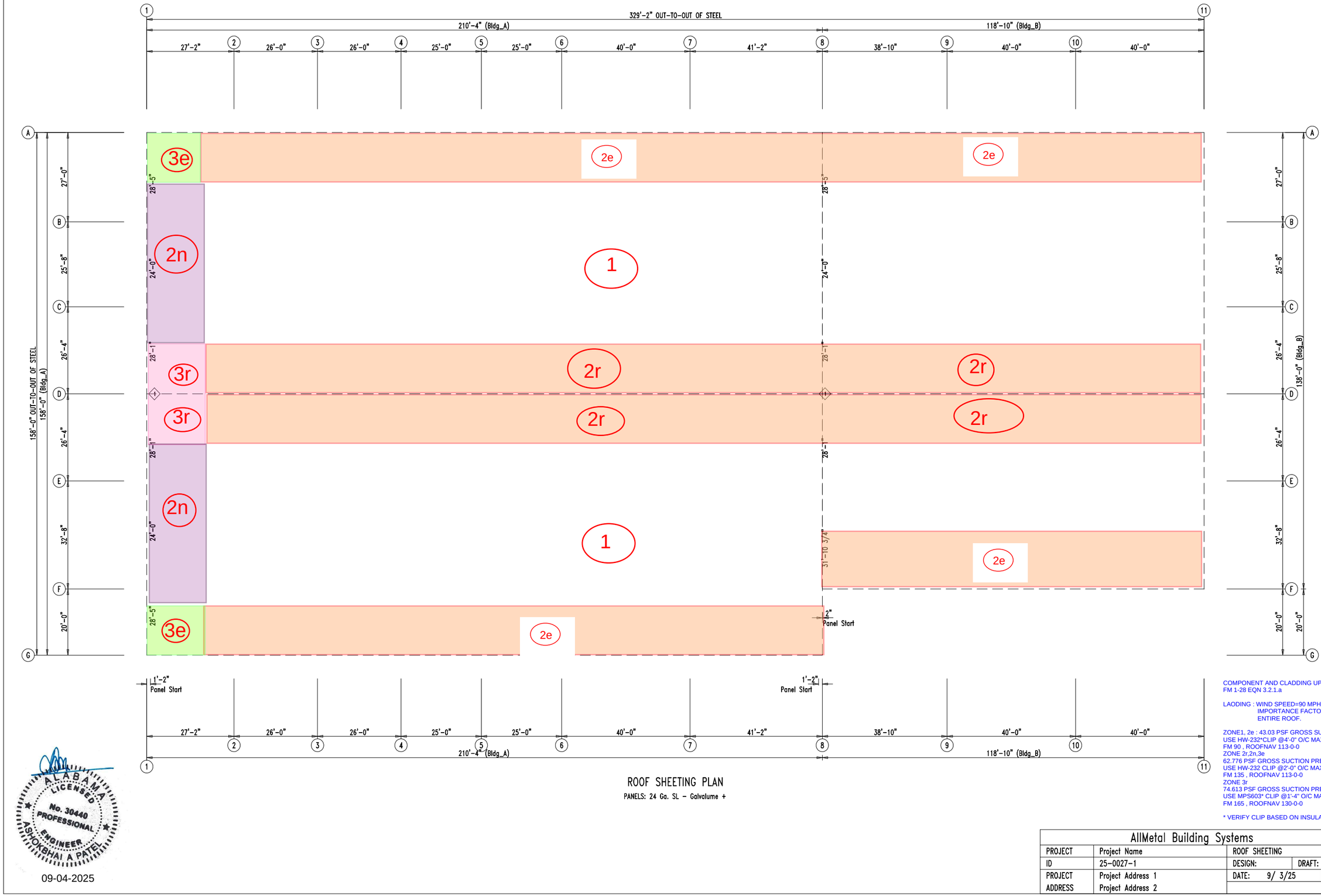
CONNECTION PLATES ROOF PLAN	
□ID	MARK/PART
1	j1
2	j100

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

AllMetal Building Systems				
PROJECT	Project Name	ROOF FRAMING		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

TRIM TABLE ROOF PLAN		
◇ID	MARK	LENGTH
1	FL213	10'-2"



ROOF SHEETING PLAN

PANELS: 24 Ga. SL – Galvalume +

COMPONENT AND CLADDING UPLIFT DESIGN PRESSURE(p) PER FM 1-28 EQN 3.2.1.a

LAODING : WIND SPEED=90 MPH, GROUND ROUGH NESS= "C"
IMPORTANCE FACTOR=1.15, 16 PSF WIND PRESSURE OVER ENTIRE ROOF.

ZONE1, 2e : 43.03 PSF GROSS SUCTION PRESSURE-PANEL
USE HW-232*CLIP @4'-0" O/C MAX WITH (2) 1/4"-14X 1 1/4" SCREWS .
FM 90 , ROOFNAV 113-0-0
ZONE 2r,2n,3e
62.776 PSF GROSS SUCTION PRESSURE-PANEL
USE HW-232 CLIP @2'-0" O/C MAX WITH (2) 1/4"-14 X 1 1/4"SCREWS .
FM 135 , ROOFNAV 113-0-0
ZONE 3r
74.613 PSF GROSS SUCTION PRESSURE-PANEL
USE MPS603* CLIP @1'-4" O/C MAX WITH (3) 1/4"-14 x1 1/4"SCREWS
FM 165 , ROOFNAV 130-0-0

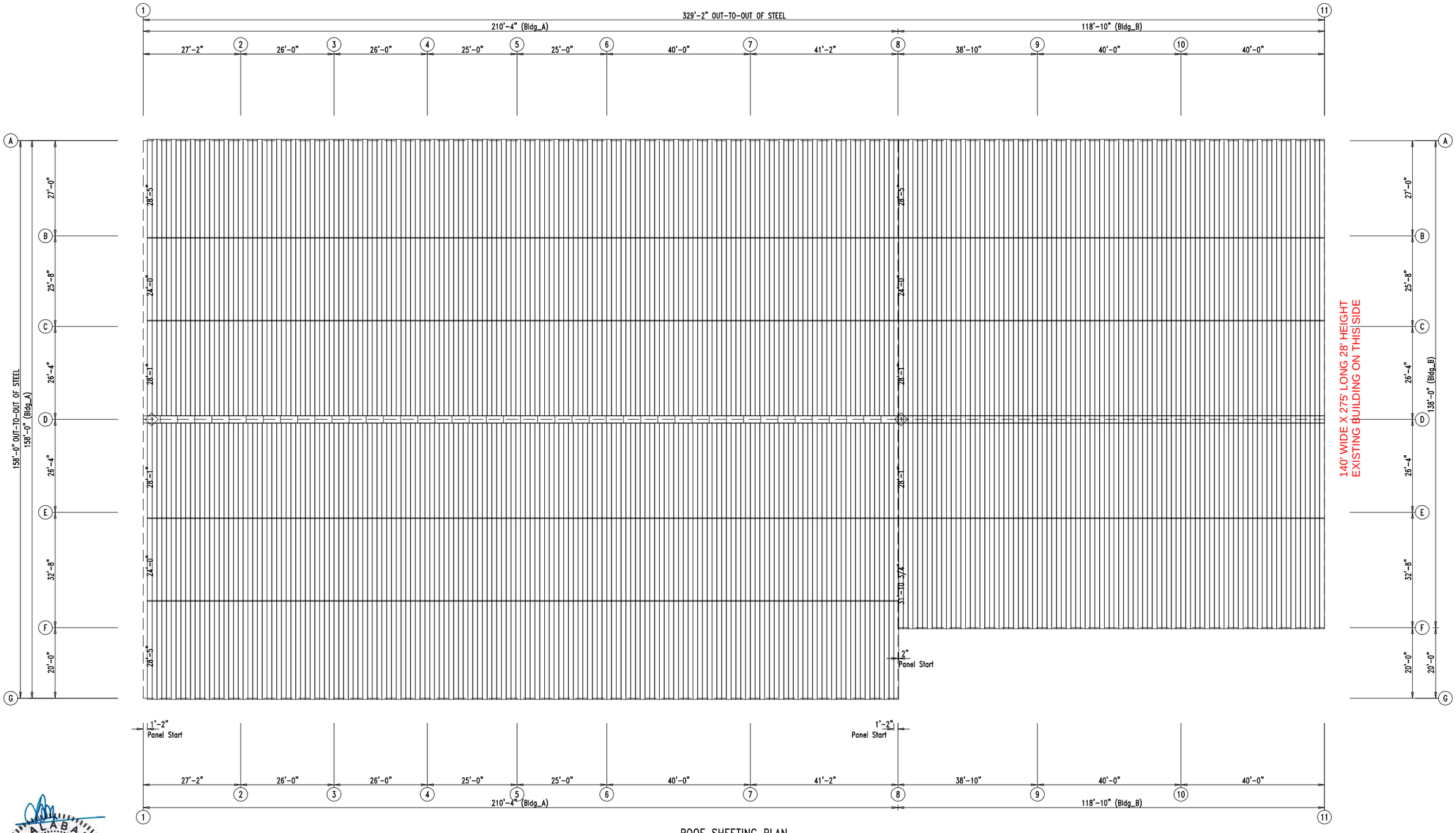
* VERIFY CLIP BASED ON INSULATION THICKNESS

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

AllMetal Building Systems				
PROJECT	Project Name	ROOF SHEETING		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

TRIM TABLE ROOF PLAN		
◇ID	MARK	LENGTH
1	FL213	10'-2"



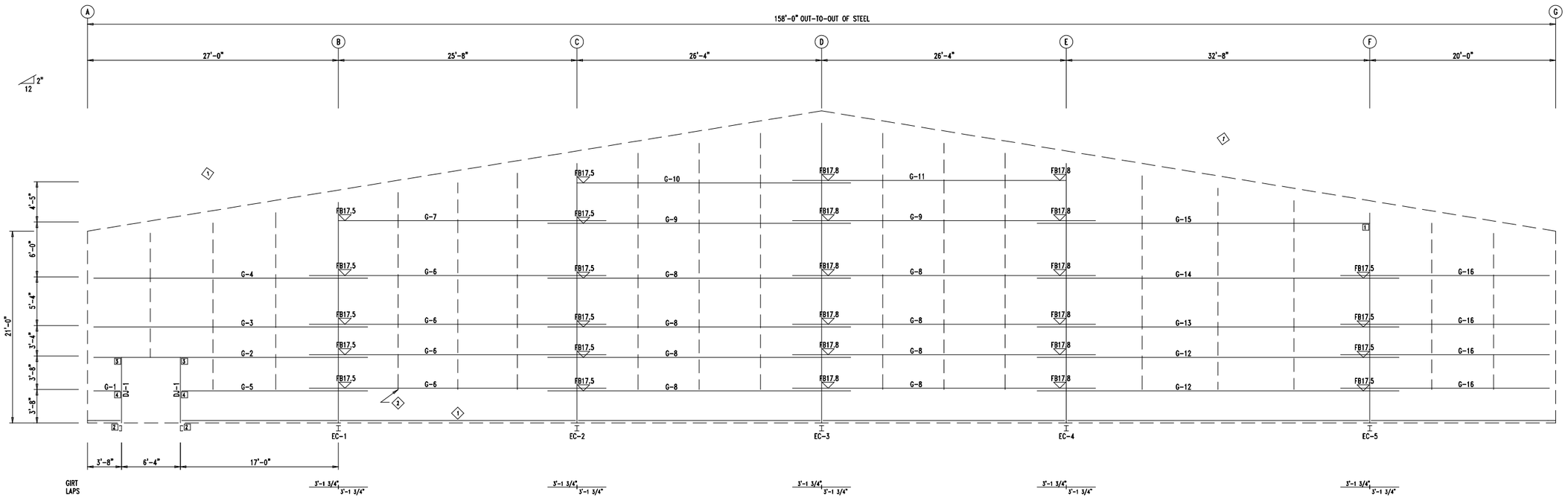
ROOF SHEETING PLAN

PANELS: 24 Ga. SL – Galvalume +

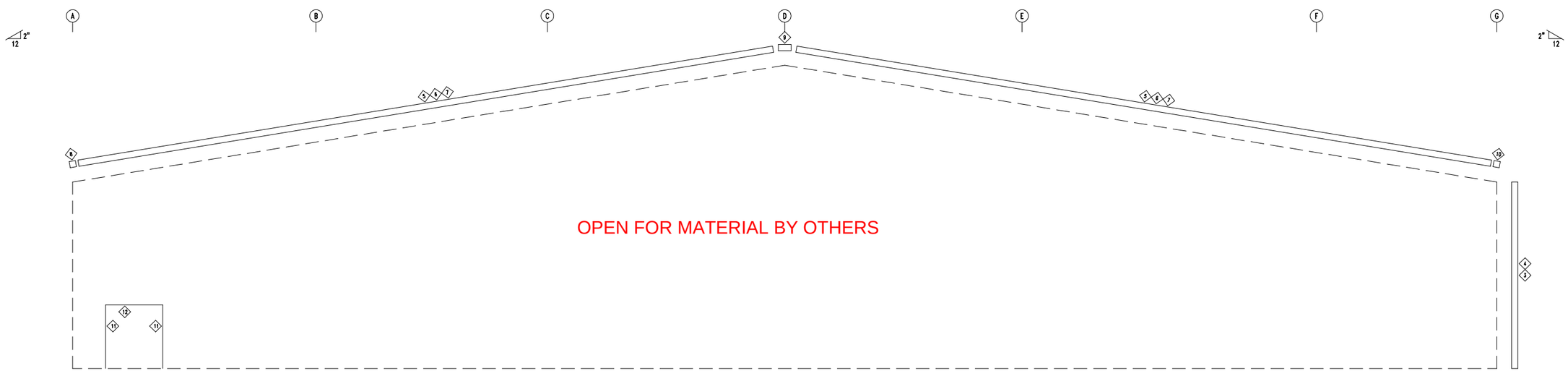
ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

AllMetal Building Systems					
PROJECT	Project Name	ROOF SHEETING			
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:	
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF	



ENDWALL FRAMING: FRAME LINE 1



OPEN FOR MATERIAL BY OTHERS

ENDWALL SHEETING & TRIM: FRAME LINE 1

THE WALL PANELS NOT DESIGNED OR SUPPLIED BY MBM, MUST PROVIDE LATERAL SUPPORT FOR THE OUTER FLANGE OF THE GIRTS. IT IS THE RESPONSIBILTY OF THE CONTRACTOR TO CONFIRM THAT WALL PANELS ARE DESIGNED TO SPAN THE GIRT SPACING SHOWN ON THE ERECTION DRAWINGS PROVIDED BY MBM.

12-W

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Rof	4	A325	3/4"	2"

TRIM TABLE
FRAME LINE 1

◇ID	MARK	LENGTH	DETAIL
3	FL23	10'-2"	
4	FL238	12'-0"	
5	FL111	20'-2"	
6	FL215	10'-2"	
7	FL117	10'-2"	
8	MTR LT	10'-2"	
9	FL125	2'-1"	
10	MTR RT	10'-2"	
11	FL23	10'-2"	TRIM_239
12	FL25	7'-1"	TRIM_232

MEMBER TABLE
FRAME LINE 1

MARK	PART	LENGTH
EC-1	W12X14	23'-1 15/16"
EC-2	W12X16	27'-5 5/16"
EC-3	W12X19	31'-9 5/8"
EC-4	W12X19	27'-5 5/16"
EC-5	W12X14	21'-11 15/16"
DJ-1	8X25x16	7'-0 1/4"
G-1	8X25Z16	3'-4"
G-2	8X25Z12	30'-1 1/2"
G-3	8X25Z14	30'-1 1/2"
G-4	8X25Z12	30'-1 1/2"
G-5	8X25Z16	19'-10"
G-6	8X25Z16	31'-11 1/2"
G-7	8X35Z14	29'-1 1/2"
G-8	8X25Z16	32'-7 1/2"
G-9	8X35Z14	32'-7 1/2"
G-10	8X25Z16	29'-9 1/2"
G-11	8X25Z16	29'-9 1/2"
G-12	8X25Z16	38'-11 1/2"
G-13	8X25Z14	38'-11 1/2"
G-14	8X25Z12	38'-11 1/2"
G-15	8X35Z12	35'-9 3/4"
G-16	8X25Z16	23'-1 1/2"

ANGLE TABLE
FRAME LINE 1

◇ID	MARK	LENGTH
1	L4x2	20'-0"
2	FL569	500'-0"

FLANGE BRACE TABLE
FRAME LINE 1

▽ID	MARK	LENGTH
1	FB17.5	11'-5 1/2"
2	FB17.8	11'-5 3/4"

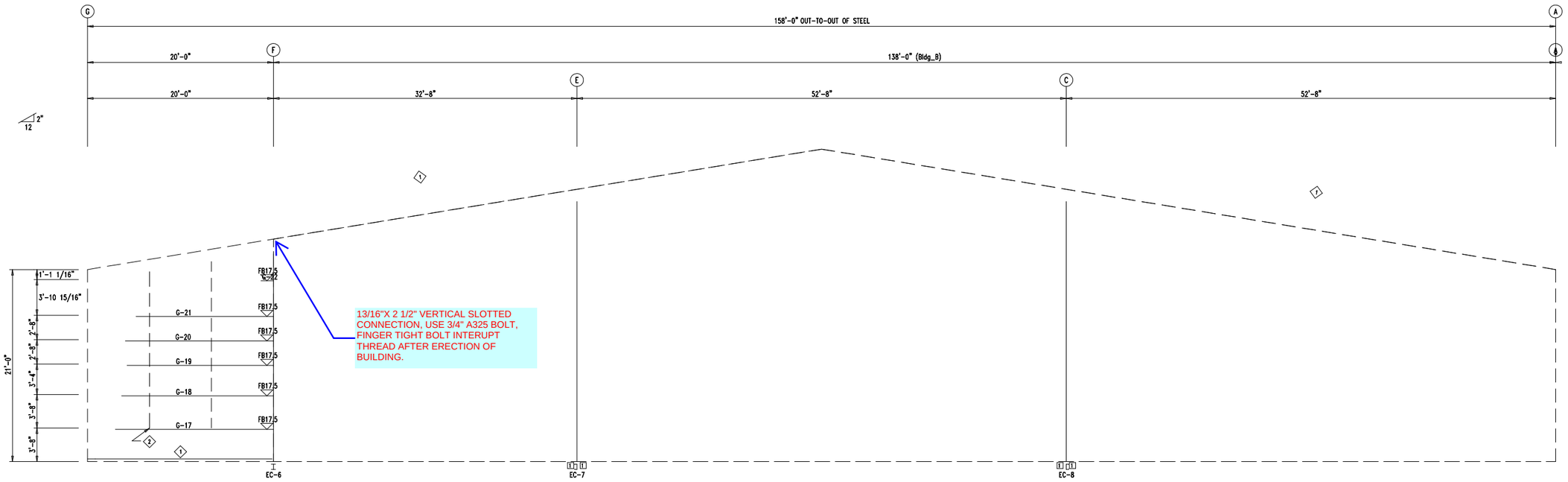
CONNECTION PLATES
FRAME LINE 1

□ID	MARK/PART
1	d1
2	SC104
3	j2
4	SC100

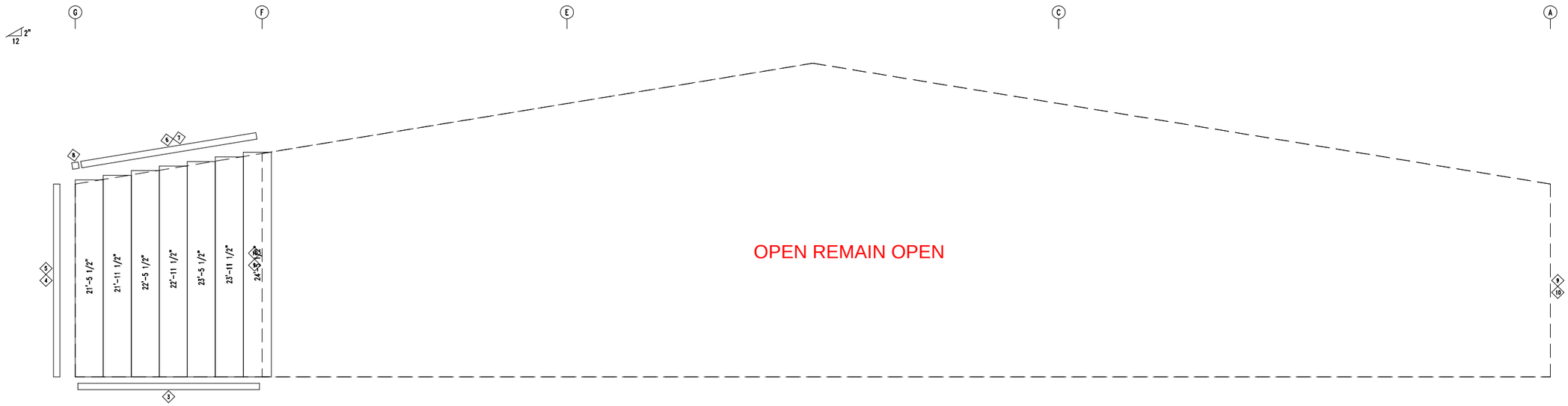
ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

AllMetal Building Systems				
PROJECT	Project Name	ENDWALL FRAMING		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF



ENDWALL FRAMING: FRAME LINE 8



ENDWALL SHEETING & TRIM: FRAME LINE 8

PANELS: 24 Ga. PR - NEED SIG 200

BOLT TABLE
FRAME LINE 8

LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	5/8"	2"

TRIM TABLE
FRAME LINE 8

◇ID	MARK	LENGTH	DETAIL
3	FL72	10'-2"	TRIM_74
4	FL830	10'-2"	TRIM_198
5	FL831	12'-0"	TRIM_198
6	FL111	20'-2"	
7	FL215	10'-2"	
8	MTR LT	10'-2"	
9	FL801	12'-0"	
10	FL802	14'-0"	

MEMBER TABLE
FRAME LINE 8

MARK	PART	LENGTH
EC-6	W12X14	20'-0 7/16"
EC-7	12X35c12	25'-10 1/2"
EC-8	12X35c12	25'-10 1/2"
G-17	8X25Z16	17'-11 5/8"
G-18	8X25Z16	17'-3 11/16"
G-19	8X25Z16	16'-8 7/16"
G-20	8X25Z16	16'-2 5/8"
G-21	8X25Z16	15'-8 7/8"
G-22	8X25Z16	1'-3 15/16"

ANGLE TABLE
FRAME LINE 8

◇ID	MARK	LENGTH
1	L4x2	20'-0"
2	FL569	SCRAP

FLANGE BRACE TABLE
FRAME LINE 8

▽ID	MARK	LENGTH
1	FB17.5	1'-5 1/2"

CONNECTION PLATES
FRAME LINE 8

□ID	MARK/PART
1	SC104

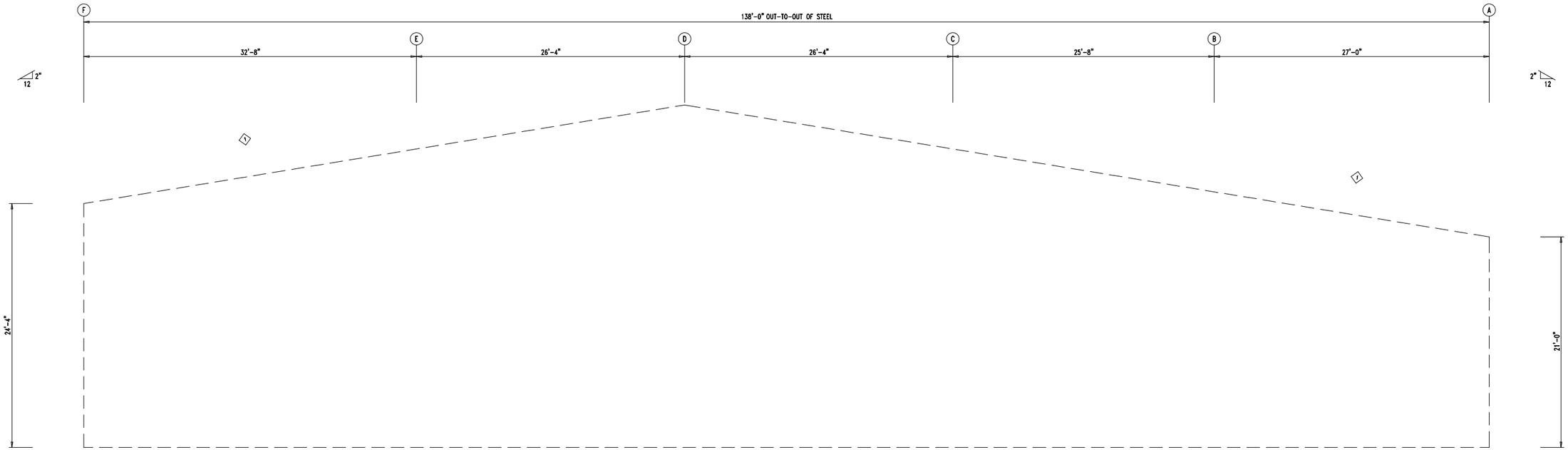
ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

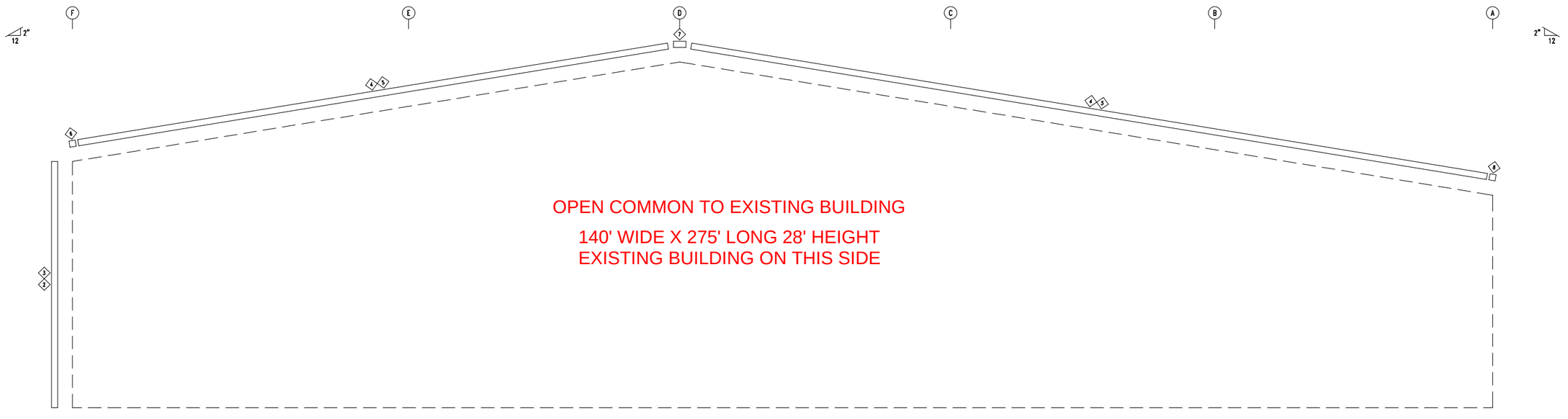
AllMetal Building Systems				
PROJECT	Project Name	ENDWALL FRAMING		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

TRIM TABLE FRAME LINE 11		
◇ID	MARK	LENGTH
2	FL23B	12'-0"
3	FL23C	14'-2"
4	FL111	20'-2"
5	FL215	10'-2"
6	MTR LT	10'-2"
7	FL125	2'-1"
8	MTR RT	10'-2"

ANGLE TABLE FRAME LINE 11		
◇ID	MARK	LENGTH
1	L4x2	20'-0"



ENDWALL FRAMING: FRAME LINE 11



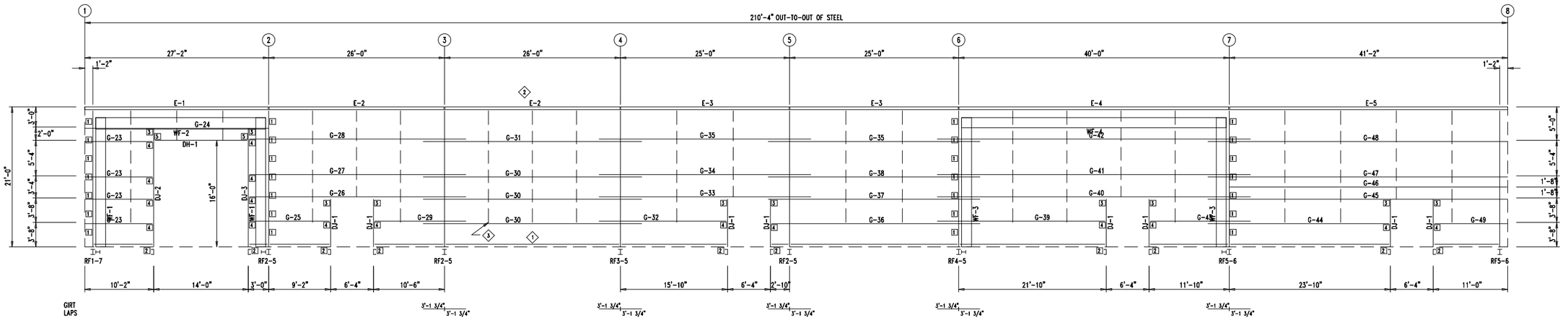
ENDWALL SHEETING & TRIM: FRAME LINE 11

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

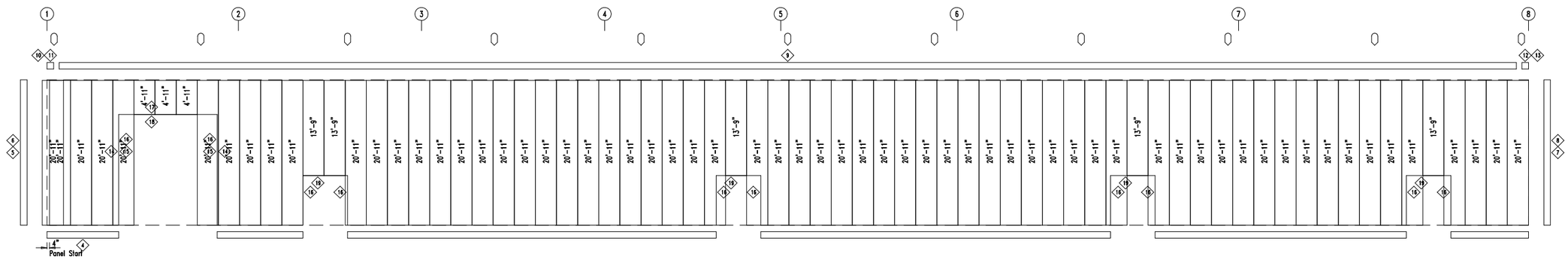
09-04-2025

AllMetal Building Systems					
PROJECT	Project Name	ENDWALL FRAMING			
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:	
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF	

DOWNSPOUT LOCATIONS



SIDEWALL FRAMING: FRAME LINE G



SIDEWALL SHEETING & TRIM: FRAME LINE G

PANELS: 24 Ga. PR - NEED SIG 200

BOLT TABLE
FRAME LINE G

LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	1 1/4"	4"
WF-1 - RF1-7	56	A325	5/8"	1 3/4"
WF-1 - RF2-5	56	A325	5/8"	1 3/4"
WF-3 - WF-4	8	A325	1 1/2"	5"
WF-5 - RF4-5	56	A325	5/8"	1 3/4"
WF-3 - RF5-6	56	A325	5/8"	1 3/4"

TRIM TABLE
FRAME LINE G

◇ID	MARK	LENGTH	DETAIL
4	FL72	10'-2"	TRIM_74
5	FL23	10'-2"	
6	FL238	12'-0"	
7	FL830	10'-2"	TRIM_198
8	FL831	12'-0"	TRIM_198
9	FL248A	20'-2"	
10	WTR LT	10'-2"	
11	LEFT	5 1/2"	
12	WTR RT	10'-2"	
13	RIGHT	5 1/2"	
14	FL-37	10'-2"	TRIM_242
15	FL22	7'-3"	TRIM_239
16	FL23	10'-2"	TRIM_239
17	FL-37	10'-2"	TRIM_235
18	FL28C	14'-4"	TRIM_232
19	FL25	7'-1"	TRIM_232

MEMBER TABLE
FRAME LINE G

MARK	PART	LENGTH
WF-1	W18X35	19'-10"
WF-2	W21X48	21'-8 1/2"
WF-3	W18X35	19'-10"
WF-4	W18X76	35'-8 1/2"
DJ-1	8X25c16	7'-0 1/4"
DJ-2	8X25c12	17'-8 1/4"
DJ-3	8X25c16	17'-8 1/4"
DH-1	8X25c16	13'-11 1/2"
E-1	E145341L	27'-1 1/2"
E-2	E145341L	25'-11 1/2"
E-3	E145341L	24'-11 1/2"
E-4	E145341L	39'-11 1/2"
E-5	E145341L	41'-1 1/2"
G-23	8X25Z16	9'-10"
G-24	8X25Z12	27'-5 1/2"
G-25	8X25Z16	11'-6 1/2"
G-26	8X25Z16	31'-10"
G-27	8X25Z16	31'-10"
G-28	8X35Z12	31'-10"
G-29	8X25Z16	13'-4"
G-30	8X25Z16	32'-3 1/2"
G-31	8X35Z14	32'-3 1/2"
G-32	8X25Z16	18'-8"
G-33	8X25Z16	31'-3 1/2"
G-34	8X25Z16	31'-3 1/2"
G-35	8X35Z14	31'-3 1/2"
G-36	8X25Z16	30'-8"
G-37	8X25Z16	31'-3 1/2"
G-38	8X25Z14	31'-3 1/2"
G-39	8X25Z16	24'-8"
G-40	8X25Z12	46'-3 1/2"
G-41	8X25Z12	46'-3 1/2"
G-42	8X35Z12	46'-3 1/2"
G-43	8X25Z16	14'-8"
G-44	8X25Z16	26'-8"
G-45	8X25Z12	44'-3 1/2"
G-46	8X35Z14	41'-1 1/2"
G-47	8X25Z12	44'-3 1/2"
G-48	8X35Z12	44'-3 1/2"
G-49	8X25Z16	10'-8"

ANGLE TABLE
FRAME LINE G

◇ID	MARK	LENGTH
1	L4x2	20'-0"
2	HW7618	8'-0"
3	FL569	500'-0"

CONNECTION PLATES
FRAME LINE G

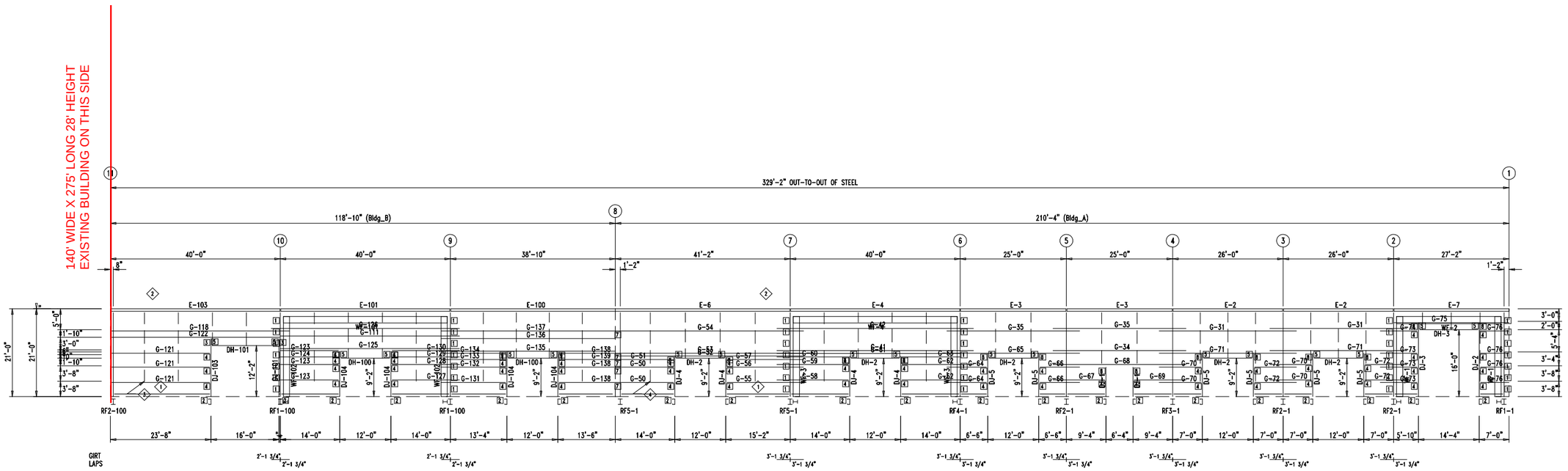
□ID	MARK/PART
1	WBCLP-3
2	SC104
3	J2
4	SC100
5	SC119

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

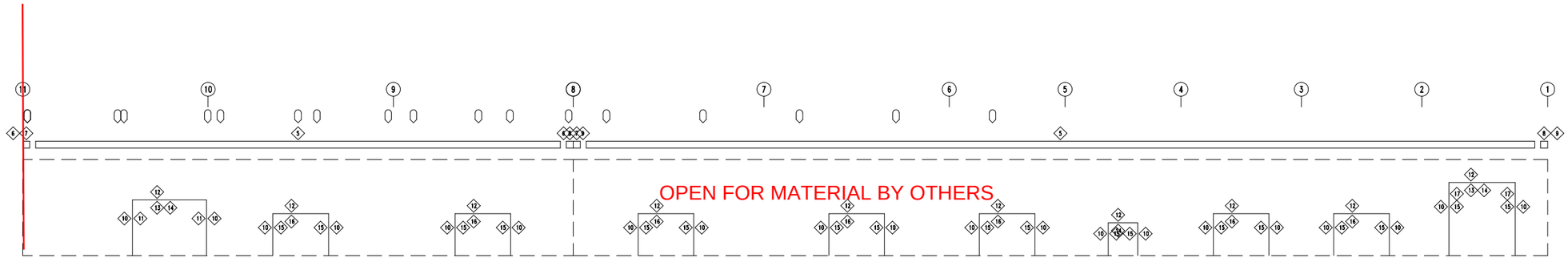
09-04-2025

AllMetal Building Systems				
PROJECT	Project Name	SIDEWALL FRAMING		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

DOWNSPOUT LOCATIONS



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

THE WALL PANELS NOT DESIGNED OR SUPPLIED BY MBM, MUST PROVIDE LATERAL SUPPORT FOR THE OUTER FLANGE OF THE GIRTS. IT IS THE RESPONSIBILTY OF THE CONTRACTOR TO CONFIRM THAT WALL PANELS ARE DESIGNED TO SPAN THE GIRT SPACING SHOWN ON THE ERECTION DRAWINGS PROVIDED BY MBM. 12-W

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

BOLT TABLE
FRAME LINE A

LOCATION	QUAN	TYPE	DIA	LENGTH
Bldg_A				
WF-3 – WF-4	8	A325	1 1/2"	5"
WF-3 – RF5-1	56	A325	5/8"	1 3/4"
WF-3 – RF4-1	56	A325	5/8"	1 3/4"
WF-1 – WF-2	8	A325	1 1/4"	4"
WF-1 – RF2-1	56	A325	5/8"	1 3/4"
WF-1 – RF1-1	56	A325	5/8"	1 3/4"
Bldg_B				
WF-102 – WF-101	8	A325	1 1/2"	5"
WF-102 – RF1-100	56	A325	5/8"	1 3/4"

TRIM TABLE
FRAME LINE A

◇ID	MARK	LENGTH	DETAIL
5	FL248A	20'-2"	
6	WTR LT	10'-2"	
7	LEFT	5 1/2"	
8	WTR RT	10'-2"	
9	RIGHT	5 1/2"	
10	FL-37	10'-2"	TRIM_242
11	FL23C	14'-2"	TRIM_239
12	FL-37	10'-2"	TRIM_235
13	FL26	10'-4"	TRIM_232
14	FL25	7'-1"	TRIM_232
15	FL23	10'-2"	TRIM_239
16	FL26B	12'-4"	TRIM_232
17	FL22	7'-3"	TRIM_239

MEMBER TABLE
FRAME LINE A

MARK	PART	LENGTH
Bldg_A		
WF-1	W18X35	19'-10"
WF-2	W21X48	21'-8 1/2"
WF-3	W18X35	19'-10"
WF-4	W18X76	35'-8 1/2"
DJ-1	8X25c16	7'-0 1/4"
DJ-2	8X25c12	17'-8 1/4"
DJ-3	8X25c16	17'-8 1/4"
DJ-4	8X25c16	9'-8 1/4"
DJ-5	8X25c16	10'-4 1/4"
DH-2	8X25c16	11'-11 1/2"
DH-3	8X25c16	14'-3 1/2"
E-2	E145341L	25'-11 1/2"
E-3	E145341L	24'-11 1/2"
E-4	E145341L	39'-11 1/2"
E-6	E145341L	41'-1 1/2"
E-7	E145341L	27'-1 1/2"
G-31	8X35Z14	32'-3 1/2"
G-34	8X25Z16	31'-3 1/2"
G-35	8X35Z14	31'-3 1/2"
G-41	8X25Z12	46'-3 1/2"
G-42	8X35Z12	46'-3 1/2"
G-50	8X25Z16	13'-8"
G-51	8X25Z16	13'-8"
G-52	8X35Z12	44'-3 1/2"
G-53	8X25Z12	44'-3 1/2"
G-54	8X35Z12	44'-3 1/2"
G-55	8X25Z16	18'-0"
G-56	8X25Z14	18'-0"
G-57	8X35Z14	18'-0"
G-58	8X25Z16	16'-10"
G-59	8X25Z14	16'-10"
G-60	8X35Z14	16'-10"
G-61	8X35Z12	43'-5 1/2"
G-62	8X25Z16	16'-10"
G-63	8X25Z16	14'-0"
G-64	8X25Z16	9'-4"
G-65	8X25Z16	31'-3 1/2"
G-66	8X25Z16	9'-4"
G-67	8X25Z16	12'-2"
G-68	8X25Z16	31'-3 1/2"
G-69	8X25Z16	12'-2"
G-70	8X25Z16	9'-10"
G-71	8X25Z16	32'-3 1/2"
G-72	8X25Z16	9'-10"
G-73	8X25Z16	8'-8"
G-74	8X35Z14	8'-8"
G-75	8X35Z12	27'-5 1/2"
G-76	8X25Z16	6'-8"
Bldg_B		
WF-101	W18X76	35'-8 1/2"
WF-102	W18X35	19'-10"
DJ-101	8X25c16	14'-1 3/4"
DJ-103	8X25c12	14'-1 3/4"
DJ-104	8X25c16	10'-10 1/4"
DH-100	8X25c16	11'-11 1/2"
DH-101	8X25c16	15'-11 1/2"
E-100	E145341L	38'-9 1/2"
E-101	E145341L	39'-11 1/2"
E-103	E145341L	39'-11 1/2"
G-111	8X25Z12	42'-1 1/2"
G-118	8X25Z12	42'-1 1/2"
G-121	8X25Z14	23'-4"
G-122	C8x18.75	39'-11 1/2"
G-123	8X25Z16	13'-8 1/2"
G-124	8X25Z16	14'-10"
G-125	8X35Z12	42'-5 1/2"
G-126	8X25Z12	44'-3 1/2"
G-127	8X25Z16	15'-10"
G-128	8X35Z14	15'-10"
G-129	8X25Z12	15'-10"
G-130	8X25Z12	15'-10"
G-131	8X25Z16	15'-2"
G-132	8X35Z14	15'-2"
G-133	8X25Z12	15'-2"
G-134	8X25Z12	15'-2"
G-135	8X35Z12	40'-11 1/2"
G-136	8X25Z12	40'-11 1/2"
G-137	8X25Z12	40'-11 1/2"
G-138	8X25Z16	13'-2"
G-139	8X25Z16	13'-2"

ANGLE TABLE
FRAME LINE A

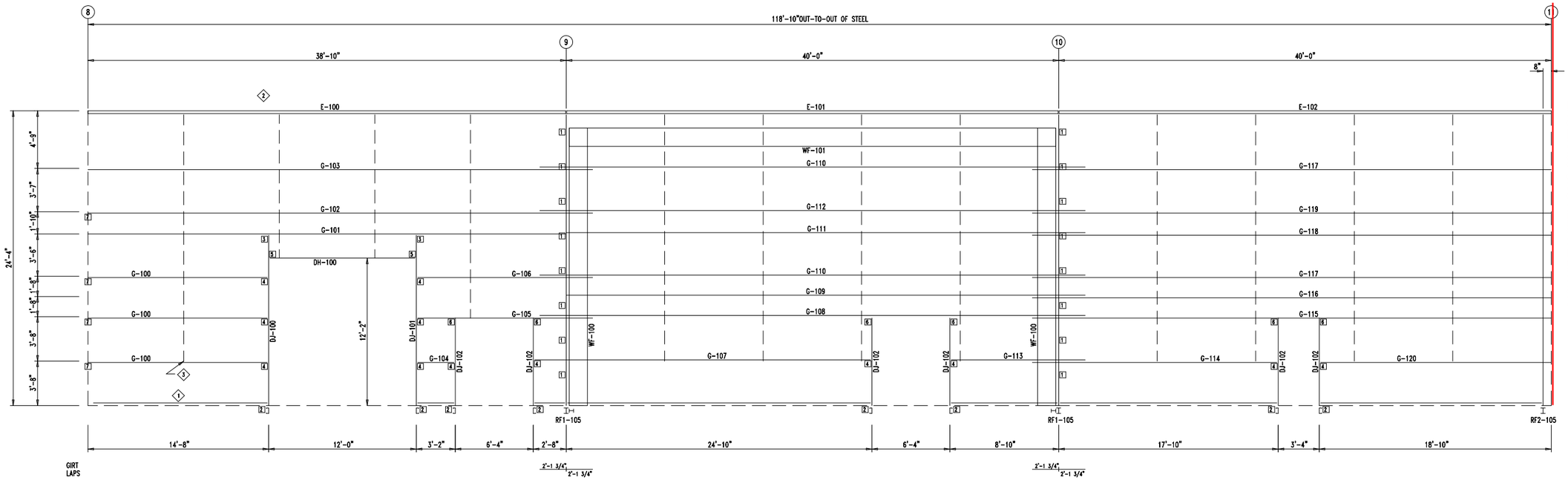
◇ID	MARK	LENGTH
1	L4x2	20'-0"
2	HW7618	8'-8"
3	FL569	SCRAP
4	FL569	500'-0"

CONNECTION PLATES
FRAME LINE A

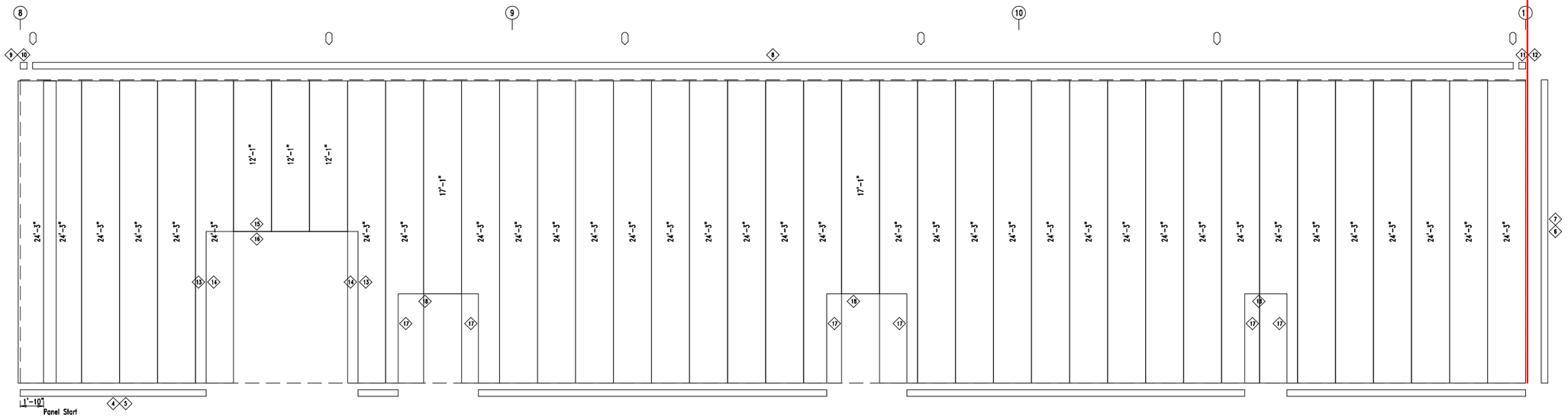
□ID	MARK/PART
1	WBCLP-3
2	SC104
3	j101
4	SC100
5	SC119
6	j102
7	r100
8	j2

AllMetal Building Systems				
PROJECT	Project Name	SIDEWALL FRAMING		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

DOWNSPOUT LOCATIONS



SIDEWALL FRAMING: FRAME LINE F



SIDEWALL SHEETING & TRIM: FRAME LINE F

PANELS: 24 Ga. PR - NEED SIG 200

BOLT TABLE
FRAME LINE F

LOCATION	QUAN	TYPE	DIA	LENGTH
WF-100 - WF-101	8	A325	1 1/2"	5"
WF-100 - RF1-105	64	A325	5/8"	1 3/4"

TRIM TABLE
FRAME LINE F

◇ID	MARK	LENGTH	DETAIL
4	FL72	10'-2"	TRIM_74
5	FL72	SCRAP	TRIM_74
6	FL23B	12'-0"	
7	FL23C	14'-2"	
8	FL248A	20'-2"	
9	WTR LT	10'-2"	
10	LEFT	5 1/2"	
11	WTR RT	10'-2"	
12	RIGHT	5 1/2"	
13	FL-37	10'-2"	TRIM_242
14	FL23C	14'-2"	TRIM_239
15	FL-37	10'-2"	TRIM_235
16	FL26B	12'-4"	TRIM_232
17	FL23	10'-2"	TRIM_239
18	FL25	7'-1"	TRIM_232

MEMBER TABLE
FRAME LINE F

MARK	PART	LENGTH
WF-100	W18X35	23'-2"
WF-101	W18X76	35'-8 1/2"
DJ-100	8X25c14	14'-1 3/4"
DJ-101	8X25c16	14'-1 3/4"
DJ-102	8X25c16	7'-0 1/4"
DH-100	8X25c16	11'-11 1/2"
E-100	E145341L	38'-9 1/2"
E-101	E145341L	39'-11 1/2"
E-102	E145341L	39'-11 1/2"
G-100	8X25Z16	14'-4"
G-101	C8x13.75	38'-9 1/2"
G-102	8X35Z14	40'-11 1/2"
G-103	8X35Z12	40'-11 1/2"
G-104	8X25Z16	2'-6 1/2"
G-105	8X25Z12	14'-0"
G-106	8X35Z12	14'-0"
G-107	8X25Z12	26'-10 1/2"
G-108	8X25Z12	44'-3 1/2"
G-109	8X25Z14	42'-5 1/2"
G-110	8X35Z12	44'-3 1/2"
G-111	8X25Z12	42'-1 1/2"
G-112	8X35Z14	44'-3 1/2"
G-113	8X25Z16	10'-8"
G-114	8X25Z16	19'-8"
G-115	8X25Z12	42'-1 1/2"
G-116	8X25Z14	42'-1 1/2"
G-117	8X35Z12	42'-1 1/2"
G-118	8X25Z12	42'-1 1/2"
G-119	8X35Z14	42'-1 1/2"
G-120	8X25Z16	18'-6"

ANGLE TABLE
FRAME LINE F

◇ID	MARK	LENGTH
1	L4x2	20'-0"
2	HW7618	8'-0"
3	FL569	500'-0"

CONNECTION PLATES
FRAME LINE F

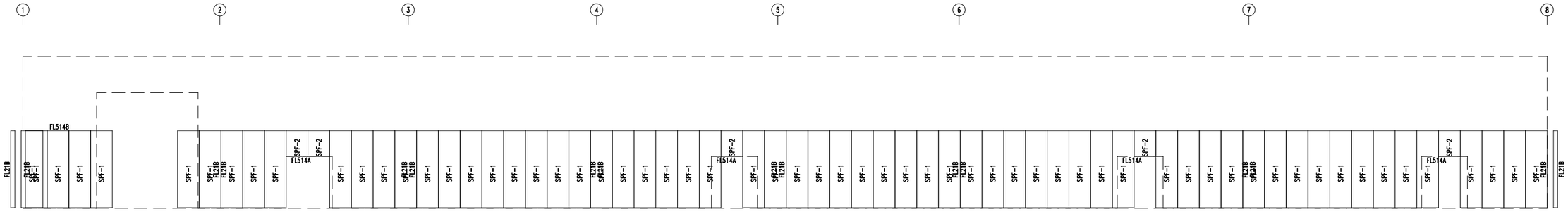
ID	MARK/PART
1	WBCLP-3
2	SC104
3	j101
4	SC100
5	SC119
6	j102
7	r100

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

AllMetal Building Systems				
PROJECT	Project Name	SIDEWALL FRAMING		
ID	25-0027-1	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Project Address 1 Project Address 2	DATE: 9/ 3/25	SHEET	OF

LINER PANEL TABLE FRAME LINE G	
MARK	LENGTH
SPF-1	10'-8"
SPF-2	3'-7"



SIDEWALL LINER SHEETING & TRIM: FRAME LINE G

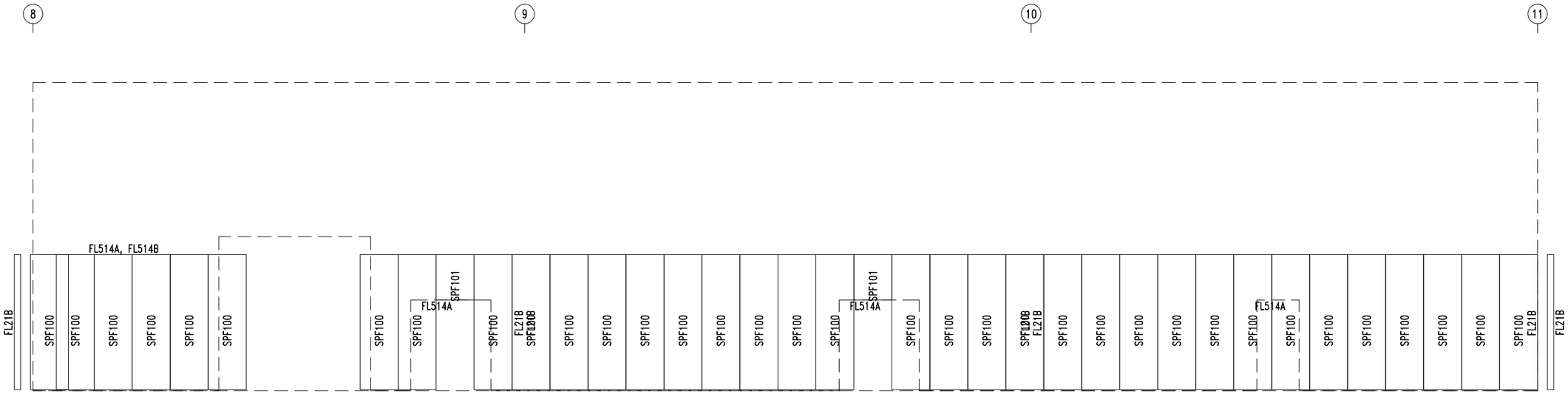
PANELS: 24 Ga. PU – NEED SIG 200

09-04-2025

ALLMETAL BUILDING SYSTEMS
1943 East Malvern Hwy, Dothan Al 36305
(334) 792-6121 (334) 886-3545 (fax)

DESCRIPTION: @DRAWING						
CUSTOMER: Customer					PROJECT: Project Name	
LOCATION: Project Address 1						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
ME	BL	9/ 3/25	N.T.S.		Bldg_A	OF

LINER PANEL TABLE FRAME LINE F	
MARK	LENGTH
SPF100	10'-8"
SPF101	3'-7"



SIDEWALL LINER SHEETING & TRIM: FRAME LINE F

PANELS: 24 Ga. PU – NEED SIG 200

ALABAMA LICENSED No. 30440 PROFESSIONAL ENGINEER ASHOKBHAI A PATEL

09-04-2025

ALLMETAL BUILDING SYSTEMS
1943 East Malvern Hwy, Dothan Al 36305
(334) 792-6121 (334) 886-3545 (fax)

DESCRIPTION: @DRAWING						
CUSTOMER: Customer					PROJECT: Project Name	
LOCATION: Project Address 1						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
ME	BL	9/ 3/25	N.T.S.		Bldg_B	OF

ALABAMA ENERGY INFRASTRUCTURE TRAINING CENTER

PACAKAGE A: SITEWORK

3711 INDUSTRIAL COURT, JASPER, ALABAMA 35501

BEVILL STATE COMMUNITY COLLEGE

OWNER ALABAMA COMMUNITY COLLEGE SYSTEM
135 SOUTH UNION STREET
MONTGOMERY, AL 36104-4340
PHONE: 334.293.4500
FAX: 334.293.4526

BEVILL STATE COMMUNITY COLLEGE
1411 INDIANA AVENUE
JASPER, ALABAMA 35501

ARCHITECT LATHAN ASSOCIATES ARCHITECTS, P.C.
300 CHASE PARK SOUTH
SUITE 200
HOOVER, ALABAMA 35244
EMAIL: RFI@LATHANASSOCIATES.COM

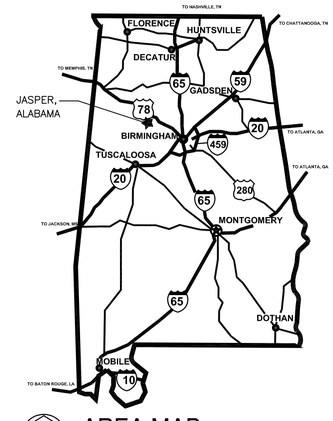
CIVIL TTL, INC.
10 INVERNESS CENTER PARKWAY
SUITE 350
HOOVER, ALABAMA 35242

ACCS No. 2023 039 BSCC

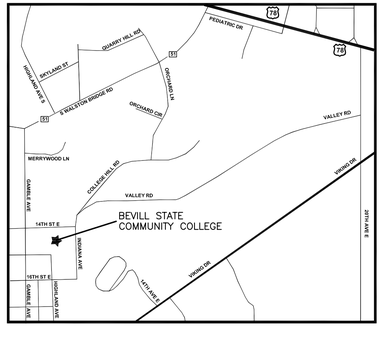
DRAWING INDEX (SET - 9 TOTAL SHEETS)

GENERAL (1 SHEET)

CIVIL DRAWINGS (8 SHEETS)



AREA MAP
STATE OF ALABAMA



VICINITY MAP
JASPER, ALABAMA

LATHAN ARCHITECTS

ALABAMA ENERGY INFRASTRUCTURE TRAINING CENTER
PACKAGE A: SITEWORK
3711 INDUSTRIAL COURT, JASPER, ALABAMA 35501
BEVILL STATE COMMUNITY COLLEGE

STATE OF ALABAMA
No. 33365
REGISTERED ARCHITECT

SHEET TITLE:
TITLE AND INDEX

PROJ. MGR.: S. CALMA
DRAWN: MSC
DATE: AUGUST 8, 2024
REVISIONS

JOB NO. 24-40A
SHEET NO:
T1
1 OF 1

0 1" 2"

EXISTING UTILITY NOTES

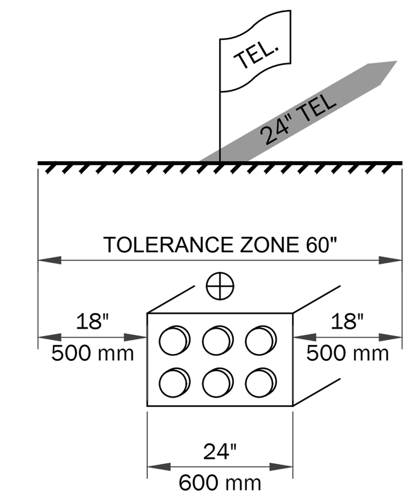
APWA UNIFORM COLOR CODE
FOR MARKING UNDERGROUND UTILITY LINES

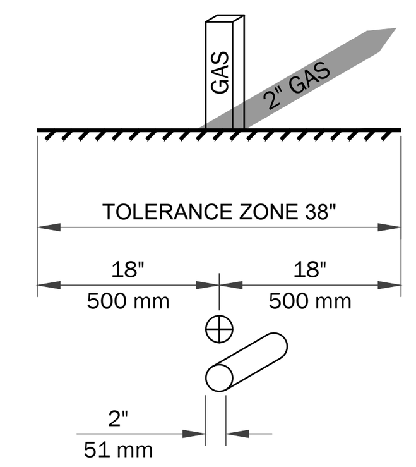
WHITE - Proposed excavation
PINK - Temporary survey markings
RED - Electric Power Lines, Cables, Conduit and Lighting Cables
YELLOW - Gas, Oil, Steam, Petroleum or Gaseous Materials
BLUE - Potable Water
PURPLE - Reclaimed Water, Irrigation and Slurry Lines
GREEN - Sewers and Drain Lines

TO HAVE UNDERGROUND UTILITY LINES LOCATED DIAL:
- 811 (INSIDE ALABAMA)
- (800) 292-8525 (INSIDE AND OUTSIDE OF ALABAMA)

WWW.AL811.COM

ALABAMA Know what's below. 811 Call before you dig





ANY EXCAVATION WITHIN THE TOLERANCE ZONE SHOULD BE PERFORMED WITH NON-POWERED HAND TOOLS OR NON-INVASIVE METHODS UNTIL THE MARKED FACILITY IS EXPOSED. THE WIDTH OF THE TOLERANCE ZONE MAY BE SPECIFIED IN LAW OR CODE. IF NOT, 500 mm (18") IS REQUIRED FROM EACH SIDE OF THE FACILITY. THE TOLERANCE ZONE INCLUDES THE WIDTH OF THE FACILITY PLUS 18" (500 mm) MEASURED HORIZONTALLY FROM EACH SIDE OF THE FACILITY.

GENERAL PROJECT NOTES:

1. THE LOCATIONS OF THE EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AS PROVIDED BY UTILITY OWNERS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE EXACT LOCATION OF ALL UNDERGROUND UTILITIES "POTHOLING" THEM BEFORE COMMENCING WORK. THE CONTRACTOR SHALL CONTACT THE ALABAMA ONE-CALL, LOCAL WATER AUTHORITIES, AND UTILITY PROVIDERS, ETC. FOR UTILITY LOCATES. IN THE EVENT OF ANY DAMAGE TO IN-PLACE UTILITIES, THEY SHALL BE REPAIRED AND REPLACED TO THE SATISFACTION OF THE ENGINEER AND THE UTILITY OWNER AT THE CONTRACTOR'S EXPENSE.
2. ANY EXISTING PROPERTY CORNERS (I.E.- IRON PIPES,CAPPED PIPES, CAPPED MONUMENTS, ETC). DISPLACED OR DAMAGED DURING CONSTRUCTION SHALL BE RESET. THIS WORK SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT AND SHALL BE A FINAL PUNCH LIST/CLOSEOUT ITEM. PROJECT PROPERTY CORNERS SHALL BE STAKED AND FLAGGED BY THE OWNER'S REPRESENTATIVE.
3. THE CONTRACTOR MUST MAINTAIN ACCESSIBLE DRIVES AND PUBLIC ROADWAYS. ANY ADDITIONAL STONE, GRADING, INSTALLATION, ETC. TO MAKE SIDEWALKS, DRIVES, AND ROADWAYS ACCESSIBLE DURING CONSTRUCTION SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT AND NO ADDITIONAL COMPENSATION SHALL BE GIVEN.
4. THE CONTRACTOR SHALL KEEP THE PROJECT RIGHTS-OF-WAY CLEAN FROM TRASH AND DEBRIS. PLACEMENT/DISCARDING OF TRASH AND REFUSE IN UTILITY TRENCHES AND/OR OTHER EXCAVATIONS ASSOCIATED WITH THE PROJECT SHALL BE PROHIBITED. THE CONTRACTOR SHALL PROVIDE TRASH RECEPTACLES FOR WORKER USE. THE ROADWAYS AND SIDEWALKS SHALL BE SWEPT AND WASHED DOWN TO LIMIT THE TRACKING OF DIRT FROM THE PROJECT ONTO PUBLIC RIGHTS-OF-WAY DAILY. THIS WORK SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT AND NO ADDITIONAL COMPENSATION SHALL BE GIVEN.
5. CONFLICTS MAY ARISE BETWEEN EXISTING AND PROPOSED UNDERGROUND FACILITIES. CROSSINGS OF REQUIRED AND EXISTING GRAVITY UTILITIES SHALL BE EXCAVATED AND ELEVATIONS VERIFIED AT THE BEGINNING OF THE PROJECT BEFORE ANY UTILITIES ARE INSTALLED TO MAKE SURE THERE ARE NO CONFLICTS. WHEN THESE CONFLICTS ARE IDENTIFIED, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE OWNER'S REPRESENTATIVE. ADJUSTMENTS AS SPECIFIED BY THE OWNER'S REPRESENTATIVE SHALL BE MADE IN THE PROPOSED AND/OR EXISTING FACILITIES. IF CONFLICTS OCCUR WHILE INSTALLING GRAVITY UTILITIES AND THE CONTRACTOR DID NOT IDENTIFY ELEVATIONS AT CROSSINGS IN ADVANCE, THEN THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COSTS ASSOCIATED WITH THE CORRECTIVE ACTION, INCLUDING BUT NOT LIMITED TO, REMOVING AND INSTALLING THE MAIN AND/OR STRUCTURES. WITH THE NUMEROUS EXISTING UTILITIES ON-SITE, IT IS IMPERATIVE THAT THESE BE VERIFIED BEFORE INSTALLATION OF PROPOSED WORK.
6. AT THE END OF THE PROJECT THE CONTRACTOR SHALL POWER WASH ALL CONCRETE SURFACES (I.E., CURB AND GUTTERS, SIDEWALK, DRIVES. STORM SEWER BOXES, BRICK PAVERS, EXISTING BUILDING BRICK, ETC.), SPECIFICALLY EXISTING CONCRETE ABUTTING REQUIRED CONCRETE SURFACES WITHIN THE PROJECT RIGHT-OF-WAY TO ELIMINATE STAINING FROM EARTHEN MATERIAL, CONSTRUCTION EQUIPMENT, OILS, PAINTS, ETC. THIS WORK SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT AND NO ADDITIONAL COMPENSATION SHALL BE GIVEN.
7. EXISTING LANDSCAPED AREAS PARALLEL TO THE PROJECT IMPACTED/DAMAGED DURING CONSTRUCTION SHALL BE RETURNED TO THEIR ORIGINAL CONDITION. THERE SHALL BE NO ADDITIONAL COMPENSATION FOR THIS WORK.
8. ALL ACCESSIBLE RAMPS AND SIDEWALKS SHALL BE ADA COMPLIANT.
9. ALL TEMPORARY STONE FOR ROADWAY, SIDEWALK, DRIVES, ETC. SHALL BE CONSIDERED INCIDENTAL TO THE PROJECT. NO TEMPORARY STONE SHALL BE WASTED ON THE SITE SPECIFICALLY IN THE FINAL SUBGRADE LAYER AND TOPSOIL. EXCESSIVE STONE WILL INHIBIT THE GROWTH OF THE LANDSCAPE. ALL STONE SHALL BE REMOVED FROM AREAS TO RECEIVE TOPSOIL, NO EXCEPTIONS.
10. THE CONTRACTOR SHALL INSTALL TEMPORARY ASPHALT PATCHING WITHIN 24 HOURS AFTER THE COMPLETED INSTALLATION OF UTILITY CROSSINGS ON ROADWAYS OPEN TO TRAFFIC. IF THE ROADWAY IS CLOSED TO LOCAL TRAFFIC THEN ALL ASPHALT CUT LOCATIONS SHALL BE PATCHED BEFORE THE ROADWAY IS REOPENED. THE CONTRACTOR SHALL NOT BE ALLOWED TO INSTALL ALL UTILITIES AND THEN TEMPORARY ASPHALT PATCH ALL AT ONE TIME. TEMPORARY ASPHALT PATCHING MUST OCCUR PERIODICALLY PHASED AS REFERENCED ABOVE.
11. WHEN TEMPORARY ASPHALT PATCHING OCCURS THE MIX SHALL BE HOT MIXED AS SPECIFIED IN THE PLANS. ASPHALT COLD MIXES SHALL NOT BE ACCEPTED. POORLY PATCHED CROSSINGS DISPLAYING NONUNIFORM, UNSMOOTH FINISHES SHALL NOT BE ACCEPTED AND SHALL BE REMOVED AT ONCE. THE REPATCH OF THE AREA SHALL BE PAID FOR AT THE CONTRACTOR'S EXPENSE.
12. THE CONTRACTOR SHALL NOTE EXISTING STORM DRAIN AND STORM DRAIN STRUCTURES TO BE RETAINED AS PART OF THIS PROJECT. THIS EXISTING INFRASTRUCTURE SHALL BE USED TO DRAIN THE PROJECT DURING PHASES OF CONSTRUCTION. PROPER EROSION CONTROL METHODS SHALL BE USED TO PROTECT THIS INFRASTRUCTURE AT ALL TIMES.
13. THE CONTRACTOR SHALL COORDINATE HIS WORK WITH THE OWNER, PRIVATE UTILITY COMPANIES, AND ANY OTHER OWNER OR GOVERNING AGENCY WITH EXISTING INFRASTRUCTURE OR JURISDICTION IN THIS AREA.

DEMOLITION NOTES:

1. THE PROJECT DEMOLITION, CLEARING AND GRUBBING GENERAL AREAS HAVE BEEN REFLECTED ON THE DEMOLITION PLAN. THE AREA IS GENERAL IN NATURE AND IS INTENDED TO GIVE THE CONTRACTOR AN APPROXIMATE AREA OF DEMOLITION. REGARDLESS OF THE AREA SHOWN, THE CONTRACTOR SHALL DEMOLISH, CLEAN AND GRUB ALL AREAS AND EXISTING INFRASTRUCTURE (ABOVE AND BELOW GROUND) NECESSARY TO COMPLETE ALL FINAL IMPROVEMENTS AS SHOWN ON THE CONSTRUCTION PLANS.
2. ALL AREAS DISTURBED BY THE CONTRACTOR; INCLUDING BUT NOT LIMITED TO ACTUAL IMPROVED AREAS, LAYDOWN AREAS, AREAS DISTURBED BY MOVING EQUIPMENT SHALL BE IMPROVED PER THE REQUIREMENTS OF THE PLANS, NO EXCEPTIONS.
3. ANY PERMANENT AND/OR CONSTRUCTION FENCING (EXISTING OR REQUIRED PER THE PLANS) REQUIRED TO BE REMOVED/RESET FOR INSTALLATION OF SITE, UTILITY, BUILDING, ETC. IMPROVEMENTS SHALL BE DONE SO AT NO ADDITIONAL COST TO THE PROJECT AND IS CONSIDERED INCIDENTAL. THE PLANS HAVE BEEN NOTED WITH GENERAL AREAS THIS IS TO OCCUR IN. THE REMOVAL AND/OR REPLACEMENT LIMITS WILL BE DETERMINED IN THE FIELD.

GRADING NOTES:

1. ALL DISTURBED AREAS SHALL BE SEEDED AND MULCHED TO PROVIDE TEMPORARY GRASSING UNTIL THE NEXT PACKAGE UNLESS OTHERWISE NOTED ON THE EROSION CONTROL PLAN. REFER TO EROSION CONTROL NOTES FOR TEMPORARY GRASSING AND MULCHING DURING GRADING OPERATIONS.
2. ALL ENGINEERED FILL MATERIALS SHALL BE REVIEWED AND APPROVED BY THE OWNER'S REPRESENTATIVE WELL IN ADVANCE OF FILL OPERATIONS. THE CONTRACTOR SHALL IDENTIFY ALL BORROW SOURCES FOR PD SAMPLES TO BE TAKEN AND EVALUATED. ALL EMBANKMENT FILL AND BORROW EXCAVATION MATERIALS SHALL BE COMPACTED IN LOOSE 8" LIFTS AS PER THE OWNER'S REPRESENTATIVES REQUIREMENTS. SEE THE GEOTECHNICAL REPORT FOR THIS INFORMATION.
3. THE CONTRACTOR SHALL CLEAR AND GRUB AS NECESSARY WHERE GRADING OPERATIONS ARE TO BE PERFORMED AS SHOWN. THE MAJORITY OF THE PROJECT WILL REQUIRE CLEARING AND REMOVAL OF EXISTING SIDEWALK, DRIVES, CURB AND GUTTER, CURBING, TREE STUMP REMOVAL, TOPSOIL, GRADING, ETC. AS SHOWN THROUGHOUT THE PROJECT CONSTRUCTION PLANS AND CONTRACT DOCUMENTS.
4. GRADING OPERATIONS SHALL INCLUDE TOPSOIL STRIPPING AND REMOVAL THROUGHOUT THE PROJECT SITE, UNCLASSIFIED EXCAVATION, AND BORROW EXCAVATION, ROCK REMOVAL, ETC. TO BRING THE SITE TO FINISHED SUBGRADE (ONLY LEAVING PAVEMENTS AND TOPSOIL TO REACH FINAL FINISHED GRADE) AS SHOWN ON THE CONSTRUCTION PLANS. NO EXTRA PAYMENT WILL BE MADE FOR EXCESS MATERIAL BROUGHT ON-SITE, MATERIAL REQUIRED TO BE MOVED MULTIPLE TIMES BECAUSE OF CONSTRUCTION PHASING, OR EXCESS MATERIAL TO BE REMOVED FROM THE SITE UPON GRADING COMPLETION.
5. THERE SHALL BE NO DEBRIS (ROOTS, ROCKS, ETC.) IN THE TOPSOIL LARGER THAN ½" IN DIAMETER. THERE ALSO SHALL BE NO WASTED TEMPORARY GRAVEL, CONCRETE, OR ANY OTHER BUILDING MATERIALS FOUND IN THE TOPSOIL. ANY FOUND DEBRIS SHALL BE REMOVED IMMEDIATELY.
6. ALL REMAINING TOPSOIL SHALL BE STOCKPILED AND TEMPORARILY GRASSED FOR USE IN THE NEXT PACKAGE.

STORM DRAIN NOTES:

1. STORM DRAIN STRUCTURE RINGS AND COVERS AND STEPS SHALL BE INSTALLED ON THE STRUCTURE WALL FREE OF PIPING AND/OR INLET THROAT OR AS DIRECTED BY THE OWNER'S REPRESENTATIVE.
2. STORM DRAIN STRUCTURES MEASURING FOUR (4) FEET OR GREATER IN DEPTH FROM THE FINISHED TOP OF THE STORM STRUCTURE TO THE INVERT OUT ELEVATION SHALL HAVE STEPS INSTALLED.
3. ALL REQUIRED STORM SEWER STRUCTURE RING AND COVER TOPS SHALL MATCH TOP OF CURB, ROADWAY AND/OR VEGETATED FINISHED GRADE ELEVATIONS UNLESS NOTED OTHERWISE ON THE CONSTRUCTION PLANS. ANY ADJUSTMENTS TO LEVEL RING AND COVER TOP ELEVATIONS WITH FINAL ASPHALT, SODDING, ETC. SHALL BE CONSIDERED A SUBSIDIARY OBLIGATION OF THE STORM DRAIN STRUCTURE INSTALLATION.
4. THE CONTRACTOR SHALL BE REQUIRED TO MAINTAIN STORMWATER FLOW IN EXISTING AND PROPOSED STORM SEWERS WITHIN THE PROJECT LIMITS AND IF AFFECTED BY CONSTRUCTION ACTIVITIES, OUTSIDE THE PROJECT LIMITS. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ANY AND ALL MATERIAL AND LABOR REQUIRED FOR TEMPORARY STORM SEWERS AND/OR PUMPS THAT MAY BE REQUIRED FOR BYPASSING. THE OWNER OR ITS REPRESENTATIVES SHALL NOT ACCEPT ANY RESPONSIBILITY FOR THE ADEQUACY OR ACCURACY, OR ACCEPT ANY RESPONSIBILITY FROM CLAIMS OR DAMAGES RESULTING FROM THE FAILURE OF THE CONTRACTOR'S TEMPORARY STORM SEWER BYPASS FACILITIES.
5. ALL STORM DRAIN STRUCTURES ARE REQUIRED TO HAVE REBAR REINFORCEMENT IN THE WALLS, BOTTOM, AND TOP. ALTHOUGH THE TOPS VARY FOR AN S-INLET, GRATE INLET, AND JUNCTION BOX, THE BOX ITSELF IS THE SAME AND REBAR REINFORCEMENT SHALL BE PLACED PER THE STANDARD DETAIL AND NOTES.
6. CONICAL MANHOLE SECTIONS AND MANHOLE RIMS AND COVERS SHALL BE ORIENTED AS PER THE PLANS AND AS DIRECTED BY THE OWNER'S REPRESENTATIVE TO ENSURE THE BEST ACCESS INTO THE MANHOLE. FAILURE TO ORIENT CORRECTLY SHALL RESULT IN REORIENTATION AT THE CONTRACTOR'S EXPENSE.
7. WHEN TYING TO EXISTING UTILITY PIPING WITH STORM DRAIN, THE CONTRACTOR SHALL USE EXTREME CARE ONLY EXCAVATING AND REMOVING THE NECESSARY AMOUNT OF PIPING TO INSTALL THE REQUIRED STRUCTURE. DAMAGE TO THE EXISTING UTILITY PIPING DUE TO OVEREXCAVATION OR POOR EXCAVATION WORK SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO REMOVE/REPLACE AT HIS COST.
8. CONTRACTOR SHALL MAKE SURE THAT THERE IS FLEXIBILITY IN EACH STORM STRUCTURE CONICAL SECTION AND RING AND COVER TO ENSURE FINAL RING ELEVATION MATCHES FINAL PAVEMENT ELEVATION. FAILURE TO DO SO WILL REQUIRE REMOVAL OF AS MUCH STRUCTURE AS NECESSARY TO ALLOW TOP RING AND COVER TO MATCH PAVEMENT.
9. THE CONTRACTOR MAY USE PRECAST CONCRETE STORM STRUCTURES FOR THE STANDARD/SPECIAL STRUCTURES REQUIRED ON THE CONSTRUCTION PLANS.
10. THE CONTRACTOR SHALL NOTE EXISTING STORM DRAIN AND STORM DRAIN STRUCTURES TO BE RETAINED AS PART OF THIS PROJECT. THIS EXISTING INFRASTRUCTURE SHALL BE USED TO DRAIN THE PROJECT DURING PHASES OF CONSTRUCTION. PROPER EROSION CONTROL METHODS SHALL BE USED TO PROTECT THIS INFRASTRUCTURE.
11. THE CONTRACTOR SHALL BACKFILL THE SPACE (WHEN BETWEEN 6 INCHES AND 2 FEET) BETWEEN STORM DRAIN AND SANITARY SEWER MAINS WHEN CROSSING WITH NO. 57 STONE MECHANICALLY CONSOLIDATED IN-PLACE TO PREVENT ANY SETTLEMENT AT THE CROSSING. THIS STONE SHALL EXTEND THE WIDTH OF THE UTILITY TRENCH TO APPROXIMATELY FOUR (4) FEET TO EITHER SIDE OF THE CROSSING.
12. THE CONTRACTOR SHALL GROUT AS NECESSARY ALL LIFTING HOLES IN STORM DRAIN PIPING SECTIONS BEFORE BACKFILL. THIS SHALL BE REQUIRED REGARDLESS IF PREFABRICATED LIFTING PLUGS ARE USED OR NOT. THE COMBINATION OF THE TWO (2) IS RECOMMENDED TO ENSURE THAT THE LIFTING HOLES DO NOT REMAIN OPEN ALLOWING EARTHEN MATERIAL TO ENTER THE DRAIN POSSIBLY CAUSING A SINK HOLE AT THE SURFACE.

GENERAL UTILITY NOTES:

1. THE CONTRACTOR SHALL BE PREPARED TO CAMERA ANY DISCOVERED UTILITY MAIN FOUND DURING CONSTRUCTION NOT SHOWN ON THE PLANS TO VERIFY IF THE MAIN SHOULD BE TIED TO THE PROPOSED SYSTEMS OR BE ABANDONED AND/OR REMOVED.
2. ALL STORM DRAIN AND SANITARY SEWER SYSTEM STRUCTURES AND PIPING SHALL REMAIN ACTIVE UNTIL PROPOSED PROJECT UTILITIES ARE INSTALLED AND CAN COME INTO SERVICE. THIS APPLIES TO AREA INLETS IN YARDS AND/OR ROOF DRAINS. ANY WATER OR SEWER DAMAGE TO PRIVATE PROPERTY DUE TO FAILURE OF THE CONTRACTOR TO COORDINATE REMOVAL OF EXISTING UTILITIES AND TIE-INS TO REQUIRED UTILITIES SHALL BE PAID FOR BY THE CONTRACTOR INCLUDING ALL CLEANUP AND ADDITIONAL WORK REQUIRED TO CORRECT THE DAMAGE.
3. THE CONTRACTOR SHALL REMOVE/RESET/RAISE ALL PRIVATE UTILITY COMPANY BOXES, MANHOLE RING AND COVER, ETC. IF THESE ITEMS ARE BEING RETAINED. ANY ITEMS DAMAGED DURING THIS WORK SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE.

EROSION CONTROL NOTES:

1. REGARDLESS IF AN NPDES PERMIT IS REQUIRED OR NOT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR USING BEST MANAGEMENT PRACTICES (BMP'S) FOR EROSION AND SEDIMENT CONTROL THROUGHOUT CONSTRUCTION. AN EROSION CONTROL PLAN IS PROVIDED AS A MINIMUM GUIDE FOR PROVIDING STRUCTURAL BMP'S. PHASING, TEMPORARY GRASSING, AND OTHER METHODS AS PROVIDED IN THE ALABAMA HANDBOOK FOR EROSION CONTROL, SEDIMENT CONTROL, AND STORM WATER MANAGEMENT, SHALL BE UTILIZED TO MINIMIZE EROSION. NO EXTRA COMPENSATION SHALL BE GIVEN TO THE CONTRACTOR FOR MAINTAINING EROSION CONTROL ITEMS OR ADDITIONAL EROSION CONTROL ITEMS REQUIRED TO COMPLY WITH THE NPDES PERMIT.
2. THE DESIGN OF THE CBMPP, IF REQUIRED, SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR'S QCP. IN THE EVENT THAT SEDIMENT BASINS ARE REQUIRED BY THE DESIGN, NO ADDITIONAL COMPENSATION SHALL BE GIVEN TO THE CONTRACTOR FOR STOCKPILING MATERIAL TO LATER FILL THE BASINS, ADDITIONAL GRADING TO FILL THE BASINS, TEMPORARY PIPING, RESEEDING AND REMULCHING, RESTORING PERMANENT DRAINAGE STRUCTURES AND GRADES TO THEIR PERMANENT REQUIREMENTS, OR ANY OTHER ITEMS OF WORK THAT ARE REQUIRED BY THE PHASING OF CONSTRUCTION OR THE CBMPP.
3. ANY FINES INCURRED DUE TO FAILURE TO MAINTAIN EROSION CONTROL MEASURES SHALL BE PAID FOR BY THE CONTRACTOR. ANY ADDITIONAL WORK AND MATERIALS REQUIRED TO COMPLY WITH ANY VIOLATIONS SHALL BE AT THE CONTRACTOR'S EXPENSE.
4. ALL TEMPORARY RIPRAP USED FOR EROSION CONTROL PURPOSES SHALL BE INCLUDED IN THE PRICE OF EROSION CONTROL. TEMPORARY RIPRAP BERMS SHALL BE SPREAD OUT IN AREAS WHERE PERMANENT RIPRAP IS REQUIRED AND SHALL BE SPREAD IN A MANNER TO NOT IMPEDE FLOW OF STORM DRAINS AFTER THE SITE IMPROVEMENTS ARE COMPLETE AND THE PROJECT IS STABILIZED. THERE SHALL BE NO ADDITIONAL COMPENSATION FOR TEMPORARY RIPRAP OR SPREADING IT UPON COMPLETION OF THE SITE IMPROVEMENTS. ALL TEMPORARY RIPRAP THAT IS SPREAD FOR USE AS PERMANENT RIPRAP SHALL BE PLACED ON THE STONE BEDDING AND FILTER FABRIC AS SHOWN IN THE DETAILS. COSTS FOR STONE AND FILTER FABRIC PLACED UNDERNEATH ALL TEMPORARY RIPRAP THAT IS SPREAD IN PERMANENT LOCATIONS SHALL BE INCLUDED IN THE UNIT PRICE BID FOR EROSION CONTROL MANAGEMENT AND MAINTENANCE, OR IF THERE ARE NO UNIT PRICES, THE COST SHALL BE INCIDENTAL TO THE PROJECT.
5. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN AND KEEP CLEAN ALL EROSION & SEDIMENT CONTROL STRUCTURES UNTIL THE NPDES PERMIT IS ACCEPTED AS COMPLETE BY THE QCP & ADEM, AND IS TERMINATED BY THE CONTRACTOR.
6. SILT FENCES SHALL HAVE SEDIMENT DEPOSITS REMOVED IF THEY REACH A DEPTH OF FIFTEEN INCHES (15") OR 1/2 THE HEIGHT OF THE FENCE. SEDIMENT REMOVED FROM THE SILT FENCE SHALL BE PLACED ONSITE AND STABILIZED.
7. THE PROJECT AREA SHALL REMAIN CLEAN AT ALL TIMES. THE CONTRACTOR SHALL USE WHATEVER MEANS NECESSARY TO KEEP THE PROJECT AREA CLEAN, INCLUDING MOTORIZED STREET SWEEPERS, WATER AND VACUUM TRUCKS, HAND SWEEPING AND SHOVELING, ETC. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ADDRESS THIS ISSUE EACH DAY INCLUDING WEEKENDS AND SPECIFICALLY PRE AND POST RAIN EVENTS.
8. THE CONTRACTOR SHALL IDENTIFY WORK AREA ENTRANCE/EXIT LOCATIONS FOR EQUIPMENT AND INSTALL TEMPORARY GRAVEL DRIVES TO REDUCE TRACKING ONTO PUBLIC RIGHT OF WAY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING ALL STREETS CLEAN OF ANY SEDIMENT FROM THE CONSTRUCTION SITE ON A DAILY BASIS, NO EXCEPTIONS.
9. ALL DISTURBED AREAS, INCLUDING THE EARTHEN STOCKPILES, SHALL BE MULCHED UPON COMPLETION OF GRADING OPERATIONS. ADEM REGULATIONS REQUIRE ALL DISTURBED AREAS NOT UNDERGOING ACTIVE DISTURBANCE OR ACTIVE CONSTRUCTION FOR LONGER THAN THIRTEEN (13) DAYS TO BE PROVIDED WITH TEMPORARY GROUND COVER.
10. THE CONTRACTOR SHALL INSTALL WATTLES, SANDBAGS, AND/OR SILT FENCE TRENCHED THROUGH PAVEMENT AFTER SAW-CUTTING THE ASPHALT TO AVOID RUNOFF INTO OTHER ROADWAYS, DRIVES, AND AREAS PARALLEL AND ADJACENT TO THE PROJECT. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ADDRESS THIS ISSUE EACH DAY INCLUDING WEEKENDS AND SPECIFICALLY PRE AND POST RAIN EVENTS.
11. WATTLES FOR SEDIMENT CONTROL SHALL HAVE A MINIMUM DIAMETER OF 12”.
12. THE CONTRACTOR SHALL INSTALL STONE AND/OR STABILIZE ENTRANCE/EXIT, SIDEWALKS, ROADWAY/DRIVES, ETC. AS NECESSARY. ALL STONES FOR CONSTRUCTION ENTRANCE/EXIT , SIDEWALKS, ROADWAY/DRIVES, ETC. ARE CONSIDERED INCIDENTAL REGARDLESS THE NUMBER OF TIMES FRESH STONE IS REQUIRED FOR EROSION CONTROL MEASURES. AT THE END OF THE PROJECT, ALL STONE SHALL BE REMOVED AND NOT WASTED ON THE PROJECT SITE.
13. WHEN INSTALLING SILT FENCE OR OTHER BMP'S, THE CONTRACTOR SHALL USE THE LOCATIONS PROVIDED ON THE DRAWINGS OR THE CBMPP. WASTEFUL AND/OR POORLY PLANNED INSTALLATIONS SHALL NOT RECEIVE ADDITIONAL PAY FOR REINSTALLATION AFTER MOVING TO ANOTHER PHASE OF THE WORK.
14. ADEM CLOSELY MONITORS DEVELOPMENTS FOR EROSION & SEDIMENT CONTROL VIOLATIONS. VIOLATIONS CAN LEAD TO THEM ISSUING A STOP WORK ORDER. THE PROJECT SHALL FALL UNDER THE SAME GUIDELINES. ANY FINES AND LEGAL FEES ASSOCIATED WITH THE CONTRACTOR'S FAILURE TO PROPERLY INSTALL AND MAINTAIN EROSION CONTROL MEASURES SHALL BE PAID FOR BY THE CONTRACTOR INCLUDING ANY ADDITIONAL REQUIREMENTS PLACED ON THE PROJECT BY THE FINING AGENCY. THERE SHALL BE NO CLAIMS CONSIDERED OF LOST CONTRACT TIME, MONEY, ETC. DURING THE STOP WORK PERIOD. THIS IS A SITUATION TOTALLY IN THE CONTROL OF THE CONTRACTOR AND HE WILL MEET HIS RESPONSIBILITIES TO MAINTAIN A STABILIZED CONSTRUCTION SITE.
15. ALL INLETS/STRUCTURES SHALL BE COVERED BY DOME INLET PROTECTORS DURING CONSTRUCTION UNLESS OTHERWISE NOTED TO AVOID SEDIMENT RUNOFF. THESE UNITS SHALL BE KEPT CLEAN DURING CONSTRUCTION. IF THE INLET/STRUCTURE IS TOO LARGE, THEN SEDIMENT LOGS OR SILT FENCE SHALL BE USED TO PROTECT THE INLET.
16. ALL MEANS NECESSARY SHALL BE USED TO ESTABLISH TEMPORARY EROSION CONTROL INCLUDING EROSION CONTROL NETTING, SODDING, REPEATED SEEDING AND MULCHING, ETC.
17. A BEST MANAGEMENT PLAN SHALL AT A MINIMUM RETURN ALL EXPOSED OR DISTURBED AREAS TO ORIGINAL OR BETTER CONDITION WITH AT LEAST A GOOD STAND OF GRASS AND/OR SOD. EROSION CONTROL MEASURES INCLUDING CONSTRUCTION EXIT PADS, SHOWN HEREIN TO PREVENT EROSION AND SEDIMENT RUNOFF ARE A MINIMUM AND SHALL NOT BE INTERPRETED AS BEING ALL THAT IS REQUIRED FOR THE PROJECT. CONTRACTOR SHALL BE MINDFUL DURING ALL PHASES OF CONSTRUCTION AND INSTALL AND UTILIZE ANY AND ALL ADDITIONAL ITEMS NECESSARY TO CONTROL ALL EROSION AND SEDIMENTATION ON THE PROJECT AT ALL TIMES AS REQUIRED BY ADEM AND THE ALABAMA HANDBOOK FOR EROSION CONTROL AND STORMWATER MANAGEMENT ON CONSTRUCTION SITES AND URBAN AREAS, MOST RECENT EDITION.
18. OWNER'S REPRESENTATIVE RESERVES THE RIGHT TO DIRECT ADDITIONAL ITEMS OR REVISE IN-FIELD PLACEMENT OF EROSION CONTROL ITEMS AS DEEMED NECESSARY DURING ALL PHASES OF THE PROJECT.
19. CONTRACTOR SHALL BE RESPONSIBLE FOR CLEANING OUT ALL SANITARY OR STORM SEWER MAINS AND MANHOLES ON A CONTINUAL BASIS IF CONSTRUCTION DEBRIS ENTERS SUCH MAINS. IN NO EVENT SHALL CONTRACTOR DISPOSE OF ANY DEBRIS OR MATERIALS IN SEWERS. CONTRACTOR SHALL IMMEDIATELY REMOVE ANY SUCH DEBRIS OR MATERIAL TO SATISFACTION OF OWNER'S REPRESENTATIVE.
20. CONTRACTOR SHALL BE OBSERVANT OF FORECASTED RAIN EVENTS AND PROMPTLY REPAIR, MAINTAIN, INSTALL NECESSARY EROSION CONTROL ITEMS PRIOR TO SUCH RAIN EVENTS. CONTRACTOR SHALL PROMPTLY MEDIATE, CLEAN UP, REMOVE ANY EROSION OR SEDIMENTATION FROM ALL EROSION CONTROL ITEMS, STRUCTURES, TRAPS, BASINS, ETC. AND REPAIR, MAINTAIN, RE-INSTALL, SUPPLEMENT SUCH IMMEDIATELY FOLLOWING EACH RAIN EVENT OR AS DIRECTED BY OWNER'S REPRESENTATIVE.
21. ALL CONCRETE WASHOUT WATER SHALL BE COLLECTED IN A LEAK PROOF CONTAINER SO THAT IT DOES NOT REACH THE SOIL SURFACE AND THEN MIGRATE TO SURFACE WATERS OR INTO GROUNDWATER. ALL OF THE COLLECTED CONCRETE WASHOUT WATER AND SOLIDS SHALL BE RECYCLED.

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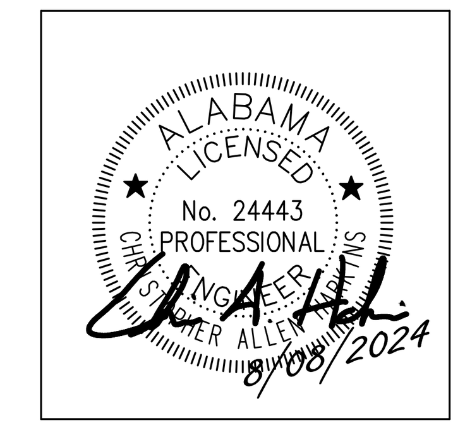

SHEET TITLE:
CIVIL NOTES

PROJ. MGR.: CAH
DRAWN: CAH
DATE: AUGUST 8, 2024
REVISIONS

JOB NO. 24-40A
SHEET NO:

C0.1
1 OF 8

0 1" 2"

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EXISTING LIGHT POLE TO REMAIN
SAW CUT & REMOVE ASPHALT
EXISTING ASPHALT TO REMAIN
EXISTING GAS LINE TO REMAIN
RAISE CLEANOUT (SEE SHEET C3.0)
RAISE CLEANOUT (SEE SHEET C3.0)
RAISE CLEANOUT (SEE SHEET C3.0)
EXISTING SANITARY SEWER LINE TO REMAIN
EXISTING BUILDING TO REMAIN
SAW CUT & REMOVE ASPHALT
REMOVE CONCRETE
12" Corrugated Plastic Pipe
Invert In: 338.06'
Finished Floor Elevation = 348.7'
EXISTING CONCRETE TO REMAIN
EXISTING WATER VALVES AND ASSOCIATED PIPING TO REMAIN (TYP.)
EXISTING STORM LINES TO REMAIN
EXISTING UNDERGROUND COMMUNICATIONS LINE TO REMAIN
EXISTING ASPHALT TO REMAIN
EXISTING UNDERGROUND ELECTRICAL LINE TO REMAIN
EXISTING OVERHEAD POWER LINES & ASSOCIATED POLES TO REMAIN (TYP.)

DEMOLITION LEGEND

- APPROXIMATE LIMITS OF CONCRETE REMOVAL
- APPROXIMATE LIMITS OF ASPHALT PAVEMENT REMOVAL

NOTES:

1. SEE SHEET C0.1 FOR ALL APPLICABLE NOTES.

0 30 60 90
SCALE: 1"=30'

ALABAMA LICENSED No. 24443 PROFESSIONAL ENGINEER ALMON
8/08/2024

SHEET TITLE:
SITE DEMOLITION PLAN

PROJ. MGR.:	CAH
DRAWN:	CAH
DATE: AUGUST 8, 2024	
REVISIONS	

JOB NO. 24-40A

SHEET NO:
C1.0
2 OF 8

0 1" 2"

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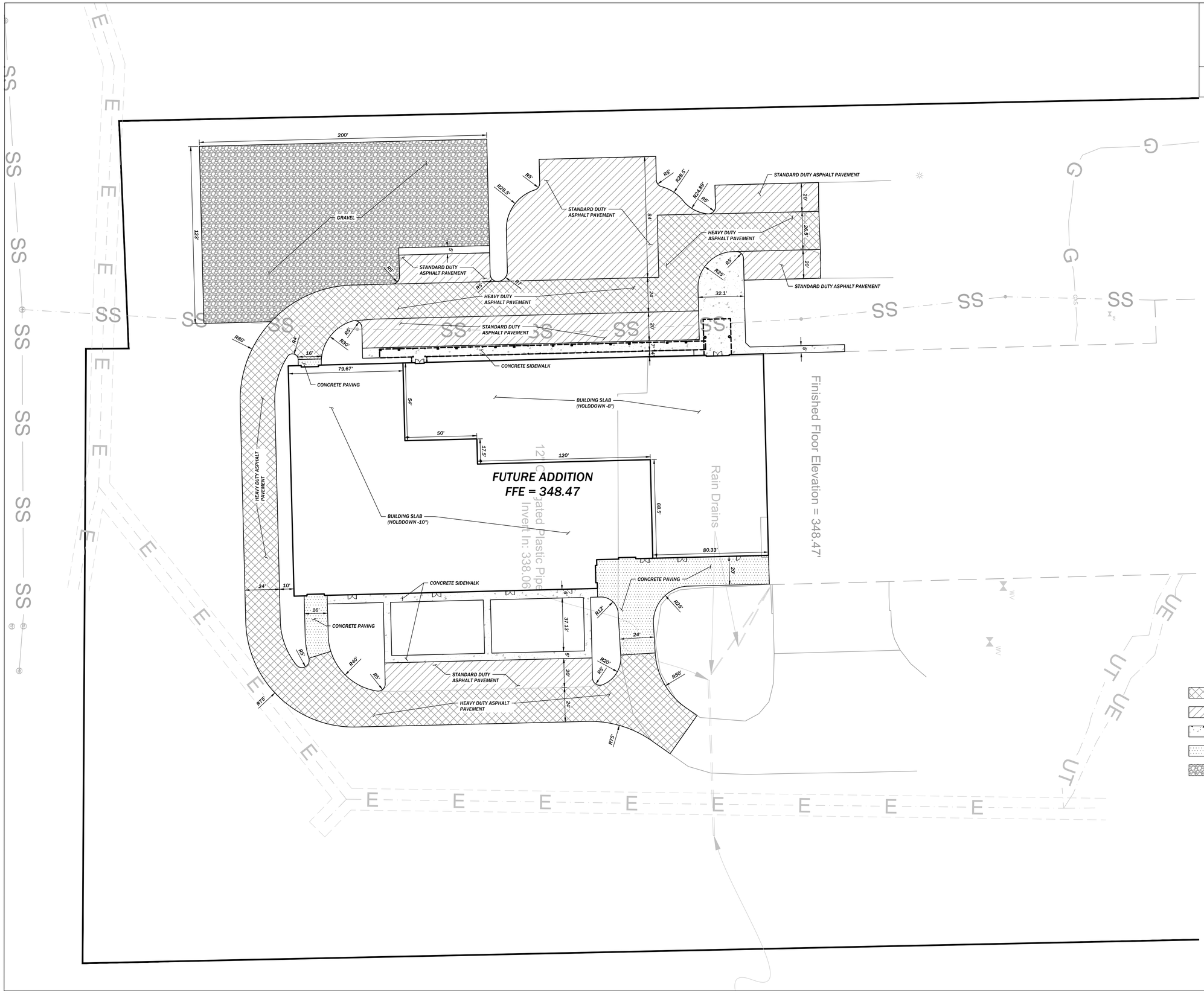

PAVING LEGEND

- *FUTURE ASPHALT PAVEMENT (HEAVY DUTY) (HOLD DOWN 11.5")*
- *FUTURE STANDARD DUTY ASPHALT PAVEMENT (HOLD DOWN 9.5")*
- *FUTURE CONCRETE SIDEWALK (HOLD DOWN 4")*
- *FUTURE CONCRETE PAVING (HOLD DOWN 10")*
- *FUTURE GRAVEL (NO. 57 STONE) (HOLD DOWN 4")*

NOTES:

1. SEE SHEET C0.1 FOR ALL APPLICABLE NOTES.
2. ALL LANDSCAPED/GRASSED AREAS SHALL BE HELD DOWN 4" FOR FUTURE TOPSOIL EXCEPT FOR THE PERIMETER SLOPES AS INDICATED ON SHEET C4.0.

0 30 60 90
SCALE: 1"=30'

ALABAMA LICENSED No. 24443 PROFESSIONAL ENGINEER CHRISTOPHER A. HALL
8/08/2024

SHEET TITLE:
SUBGRADE HOLDDOWN PLAN

PROJ. MGR.:	CAH
DRAWN:	CAH
DATE: AUGUST 8, 2024	
REVISIONS	

JOB NO. 24-40A

SHEET NO:

C2.0

3 OF 8

0 1" 2"

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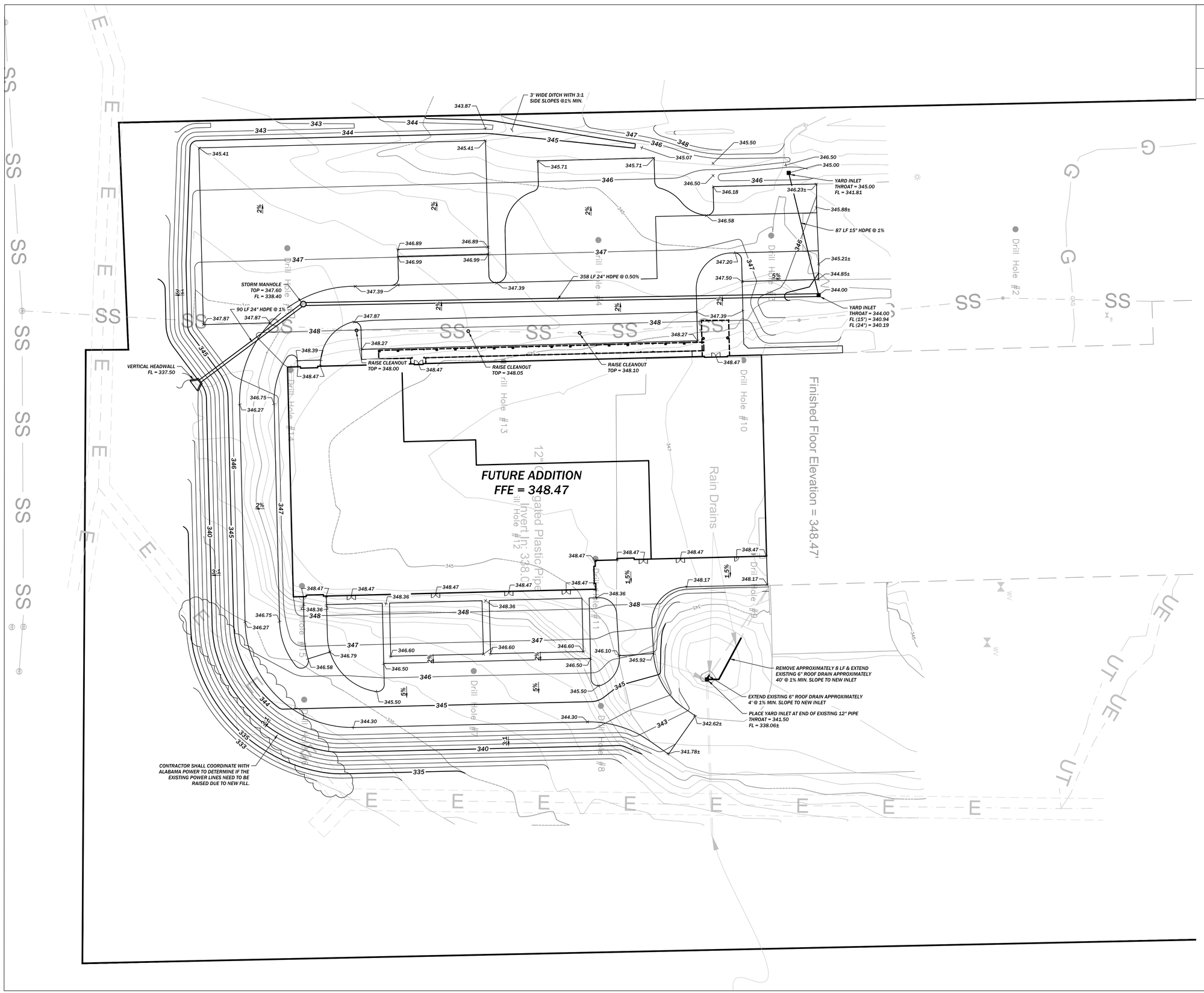

NOTES:

1. SEE SHEET C0.1 FOR ALL APPLICABLE NOTES.
2. ALL GRADES SHOWN ARE TO FINISHED GRADE. CONTRACTOR TO USE HOLD DOWNS INDICATED ON SHEET C2.0 FOR SUBGRADE (FINAL GRADE IN THIS PACKAGE) ELEVATIONS.

0 30 60 90
SCALE: 1"=30'

ALABAMA LICENSED
No. 24443
PROFESSIONAL
ENGINEER
CHRISTOPHER ALLEN HOLT
8/08/2024

SHEET TITLE:
GRADING & DRAINAGE PLAN

PROJ. MGR.:	CAH
DRAWN:	CAH
DATE:	AUGUST 8, 2024
REVISIONS	

JOB NO. 24-40A

SHEET NO:
C3.0
4 OF 8

0 1" 2"

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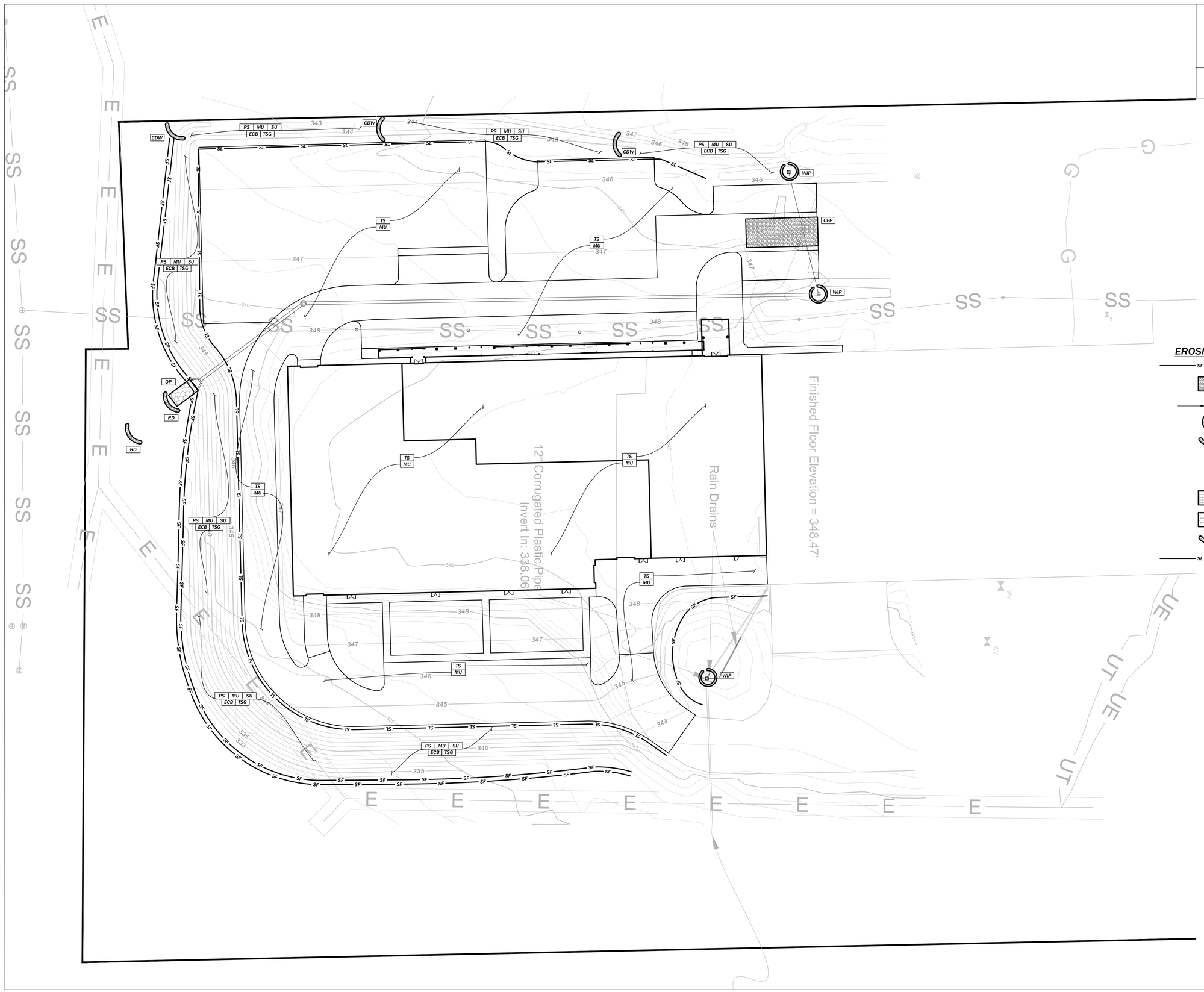

EROSION CONTROL LEGEND

- SF — SILT FENCE
- CEP — CONSTRUCTION EXIT PAD
- DV — DIVERSION CHANNEL
- WIP — WATTLE INLET PROTECTION
- CDW — WATTLE CHECK DAM
- TSG — TOPSOILING
- MU — MULCHING
- PS — PERMANENT SEEDING
- SU — SURFACE ROUGHENING
- ECB — EROSION CONTROL BLANKET
- OP — OUTLET PROTECTION
- RD — ROCK FILTER DAM
- SL — SEDIMENT LOG
- TS — TEMPORARY SEEDING

NOTES:

1. SEE SHEET C0.1 FOR ALL APPLICABLE NOTES.
2. ALL AREAS SHALL RECEIVE TEMPORARY SEEDING AND MULCH WITH THE EXCEPTION OF THE PERIMETER SLOPES AND DITCHES AS NOTED.

0 30 60 90
SCALE: 1"=30'

ALABAMA LICENSED
No. 24443
PROFESSIONAL
ENGINEER
8/08/2024

SHEET TITLE:
EROSION CONTROL PLAN

PROJ. MGR.:	CAH
DRAWN:	CAH
DATE: AUGUST 8, 2024	
REVISIONS	

JOB NO. 24-40A

SHEET NO:
C4.0
5 OF 8

0 1" 2"

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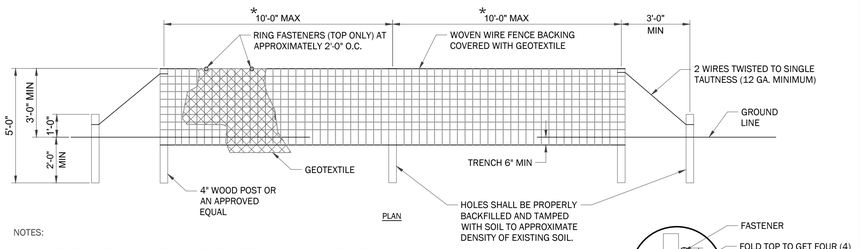
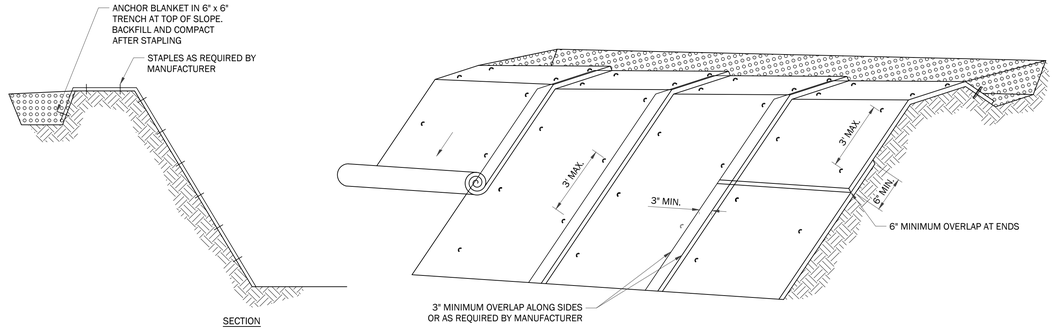


* AASHTO M288 RECOMMENDS A 4 FT POST SPACING.

NOTES:

1. SILT FENCES ARE TEMPORARY EROSION CONTROL ITEMS, THAT SHALL BE ERECTED OPPOSITE ERODIBLE AREAS SUCH AS NEWLY GRADED FILL SLOPES AND ADJACENT TO STREAMS AND CHANNELS.
2. SILT FENCE SHOULD BE PLACED WELL INSIDE RIGHT OF WAY AND ALONG EDGE OF CLEARING LIMITS, THIS WILL ALLOW ROOM FOR A BACK UP FENCE IF FIRST BECOMES FULL. SILT FENCES SHALL BE IN PLACE PRIOR TO ANY CONSTRUCTION OPERATION.
3. WHEREVER POSSIBLE SILT FENCES SHALL BE CONSTRUCTED ACROSS A FLAT AREA IN THE SHAPE OF A HORSESHOE. THIS AIDS IN PONDING OF RUNOFF AND FACILITATES SEDIMENTATION.
4. AFTER THE CONSTRUCTION AREA IS STABILIZED AND EROSION ACTIVITY CURTAILED, SILT FENCES SHALL BE REMOVED.
5. RING FASTENERS USED TO SECURE GEOTEXTILES TO WOVEN WIRE SHALL BE 13 GA. (AMERICAN).
6. IF WOOD POSTS ARE USED, STAPLES FOR SECURING WOVEN WIRE TO POSTS SHALL BE (9) GAUGE, GALVANIZED, 1-1/2" LONG, 5 PER POST AT APPROX. 1'-0" O.C.
7. WOVEN WIRE TO BE 14 GAUGE (MIN.), 6"X6".
8. MACHINE TRENCHED GEOTEXTILE SHAL BE TRENCHED VERTICAL AT LEAST 8" DEEP.
9. FOR J-HOOK APPLICATION AND INTERCEPTED FLOW PERPENDICULAR TO THE SLOPE CAUSING CONCENTRATED IMPOUNDMENT, A MAXIMUM POST SPACING SHOULD BE REDUCED TO 5 FT.

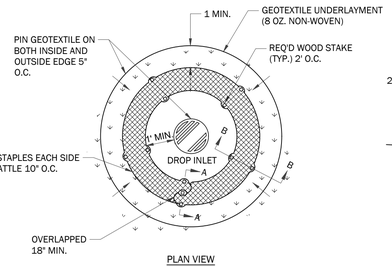
SILT FENCE - TYPE "A"
NOT TO SCALE



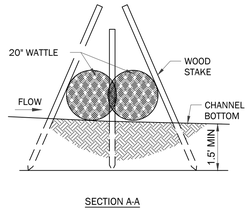
NOTES:

1. SLOPE SURFACE SHALL BE FREE OF ROCKS AND SOIL CLODS TO MAINTAIN GOOD SOIL CONTACT.
2. APPLY SEED, FERTILIZER, AND/OR LIME PRIOR TO THE INSTALLATION OF THE BLANKET.
3. STRIPS SHALL BE ROLLED OUT FLAT, PARALLEL TO DIRECTION OF FLOW WITHOUT BEING STRETCHED.
4. WHEN MULTIPLE STRIPS ARE REQUIRED TO COVER THE WIDTH OF THE SLOPE, THE SIDES SHALL OVERLAP A MINIMUM OF 3".
5. WHEN MULTIPLE STRIPS ARE REQUIRED TO COVER THE LENGTH OF THE SLOPE, THE ENDS SHALL OVERLAP A MINIMUM OF 6".
6. THE UPSLOPE END SHALL BE ANCHORED IN A 6" VERTICAL TRENCH AND BACKFILLED (NOTE: WHEN, IN THE OPINION OF THE QCP, CONDITIONS WARRANT, OTHER EDGES EXPOSED TO EXCESSIVE FLOW SHALL BE INSTALLED AS PREVIOUSLY SPECIFIED).
7. STAPLES SHALL BE U-SHAPED WIRE WITH A MINIMUM 11 GAUGE THICKNESS, AND THE LEGS SHALL BE AT LEAST 6" LONG WITH A 1" CROWN.
8. EACH STRIP SHALL BE STAPLED IN 3 ROWS, AT EDGES AND CENTER, WITH STAPLES SPACED NOT MORE THAN A 3 FOOT GRID.

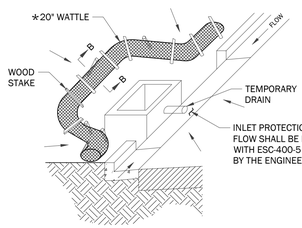
TEMPORARY EROSION CONTROL BLANKET
NOT TO SCALE



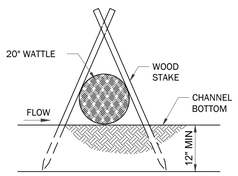
PLAN VIEW



SECTION A-A



CURB INLET PROTECTION

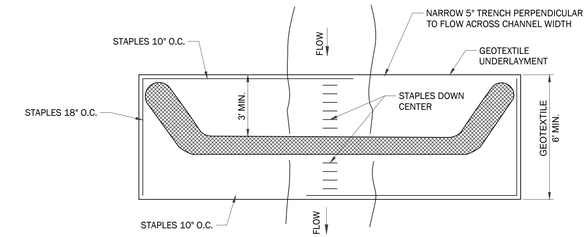


SECTION B-B

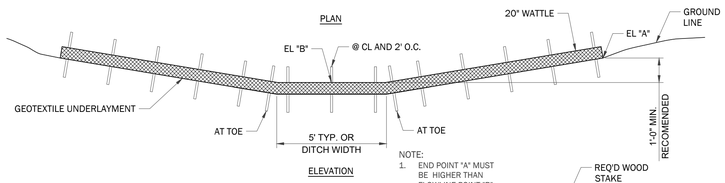
NOTES:

1. ANCHORING STAKES SHALL BE SIZED, SPACED, AND BE OF A MATERIAL THAT EFFECTIVELY SECURES THE WATTLE. STAKE SPACING SHALL BE A MAXIMUM OF TWO FEET.
2. OVERLAP ENDS OF WATTLES PER MANUFACTURERS RECOMMENDATIONS (18"' MIN, 3' MAX).
* 3. SILT FENCE OR SAND BAGS MAY ALSO BE USED FOR THIS APPLICATION. HAY BALES NOT ACCEPTABLE DURING THIS STAGE.

WATTLE INLET PROTECTION
NOT TO SCALE



PLAN



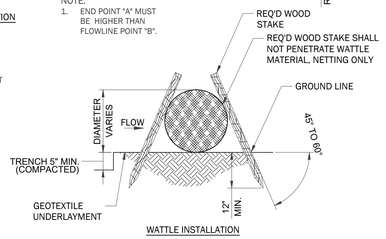
ELEVATION

NOTE:
1. END POINT "A" MUST BE HIGHER THAN FLOWLINE POINT "B".

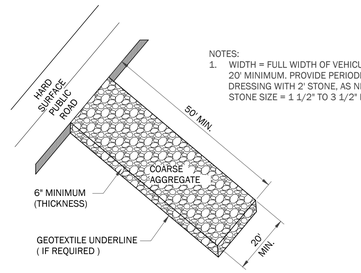
NOTES:

1. SPACING SHALL BE AS SHOWN ON THE PLANS FOR SPECIAL AREA'S, OR AS DETERMINED BY THE ACTUAL PROJECT NEEDS TO MINIMIZE EROSION.
2. ADDITIONAL EROSION CONTROL METHODS WILL BE REQUIRED AT THE DITCH OUTLET TO CONTAIN SEDIMENT.
3. WATTLE SHOULD NOT BE PLACED IN HARD BOTTOM CHANNELS.
4. STAPLES PLACED 18" APART ALONG THE CHANNEL EDGES AND DOWN THE CENTER OF THE CHANNEL. STAPLES PLACED 10" APART ACROSS THE UPSTREAM AND DOWNSTREAM EDGES.

WATTLE CHECK DAM / SEDIMENT CONTROL LOG
NOT TO SCALE



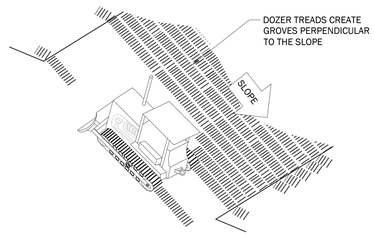
WATTLE INSTALLATION



NOTES:

1. WIDTH = FULL WIDTH OF VEHICULAR ACCESS, 20' MINIMUM. PROVIDE PERIODIC TOP DRESSING WITH 2' STONE, AS NECESSARY. STONE SIZE = 1 1/2" TO 3 1/2" DIA.

CONSTRUCTION ENTRANCE/EXIT PAD
NOT TO SCALE



SURFACE ROUGHENING
NOT TO SCALE

ALABAMA LICENSED PROFESSIONAL ENGINEER
No. 24443
8/08/2024

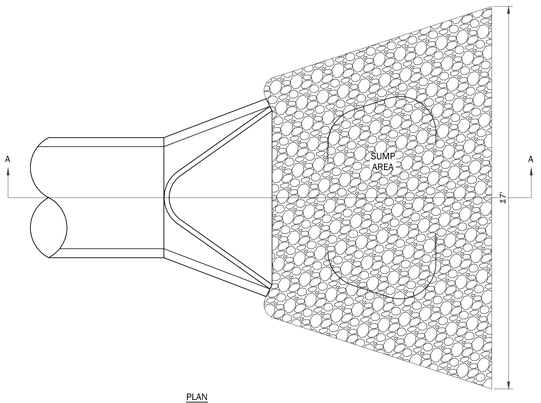
SHEET TITLE:
EROSION CONTROL DETAILS

PROJ. MGR.:	CAH
DRAWN:	CAH
DATE: AUGUST 8, 2024	
REVISIONS	

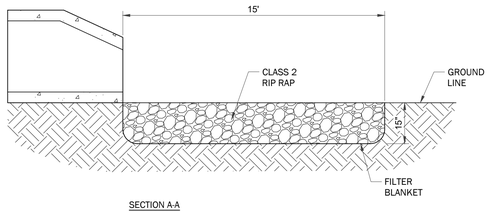
JOB NO. 24-40A

SHEET NO:
C4.1
6 OF 8

0 1" 2"

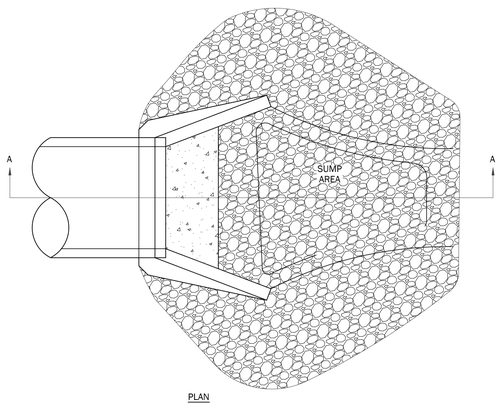


PLAN

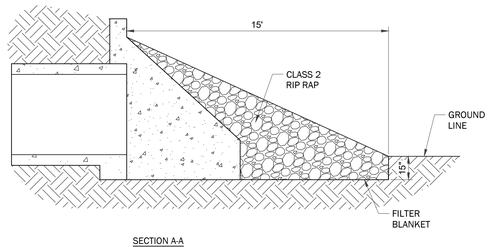


SECTION A-A

PIPE OUTLET TO FLAT AREA - NOT WELL-DEFINED CHANNEL



PLAN



SECTION A-A

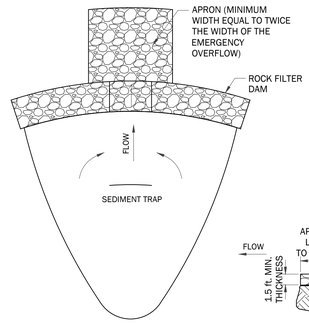
PIPE OUTLET TO WELL-DEFINED CHANNEL

NOTES:

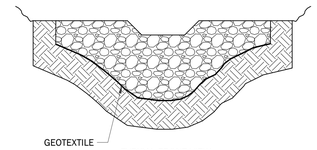
1. IN A WELL DEFINED CHANNEL EXTEND THE APRON UP THE CHANNEL BANKS TO THE TOP OF THE BANK.
2. A FILTER BLANKET OR FILTER FABRIC SHOULD BE INSTALLED BETWEEN THE RIPRAP AND SOIL FOUNDATION. A NON-WOVEN GEOTEXTILE MEETING THE REQUIREMENTS OF AASHTO M288 FOR A CLASS 2 SEPARATION GEOTEXTILE.

OUTLET PROTECTION DURING CONSTRUCTION

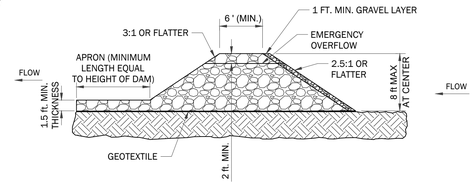
NOT TO SCALE



TYPICAL TOP SECTION



TYPICAL FRONT VIEW



TYPICAL SECTION

NOTES:

1. SEDIMENT TRAP SHALL BE ACCESSIBLE FOR PERIODIC SEDIMENT REMOVAL.
2. DAM SHALL BE FACED WITH 1 FOOT OF SMALLER STONE ON UPSTREAM SIDE, (TYPICALLY No. 57 STONE).
3. REMOVE SEDIMENT FROM THE TRAP AREA WHEN IT ACCUMULATES ½ THE DESIGN VOLUME.
4. IF THE BASIN DOES NOT DRAIN BETWEEN STORM EVENTS DUE TO THE SMALLER STONE ON THE UPSTREAM FACE BEING CLOGGED, THE CLOGGED STONE SHOULD BE REPLACED WITH CLEAN STONE.
5. RIPRAP CLASS AS SHOWN IN PLANS.
6. NON-WOVEN GEOTEXTILE, MEETING AASHTO M288 CLASS 2, SHOULD BE USED AS A SEPARATOR BETWEEN THE GRADED STONE, THE SOIL BASE AND THE ABUTMENTS. GEOTEXTILE MAY ALSO BE ADDED BETWEEN THE SMALLER STONE AND ROCK.

ROCK FILTER DAM AND EROSION CONTROL SEDIMENT TRAP

NOT TO SCALE

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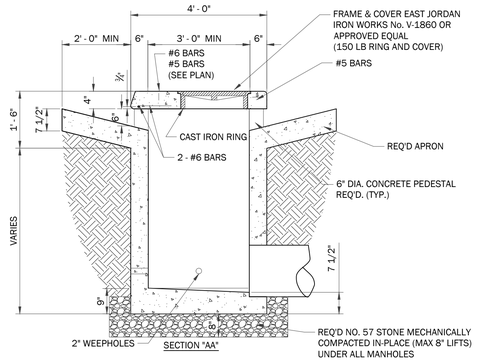
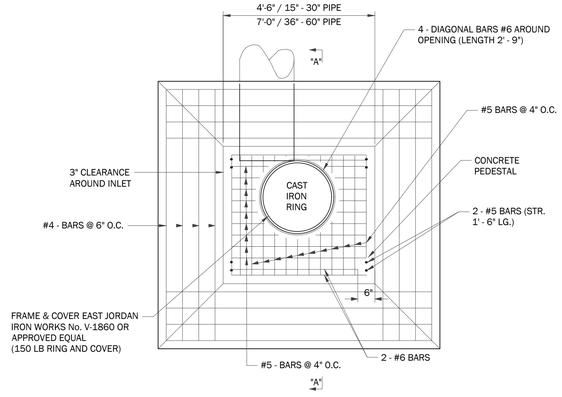


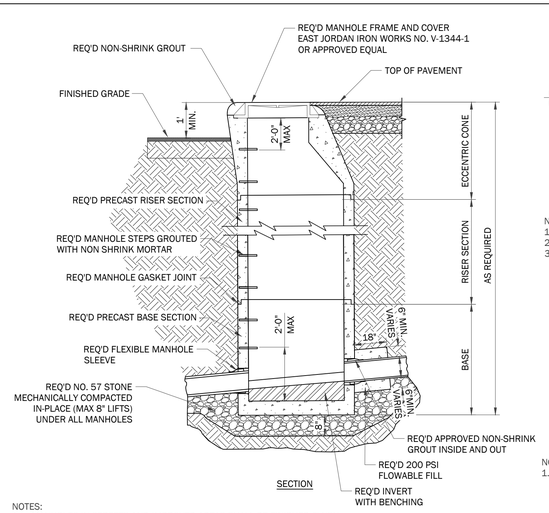


PLAN

NOTES:
1. USE 3000 P.S.I. CONCRETE AND DEFORMED REINFORCING STEEL.
2. IN LIEU OF BOX, CONTRACTOR MAY USE CIRCULAR PRECAST CONCRETE MANHOLE WITH INSIDE DIAMETER EQUAL TO AT LEAST PIPE I.D. + 2'-0"
3. A MINIMUM OF (3) THREE. LADDER BARS ARE REQ'D. IN ALL INLETS. WHERE DEPTH IS GREATER THAN 4' - 0", NUMBER AND LOCATION OF LADDER BARS IN INLET TO BE AS DIRECTED BY ENGINEER.
4. RING AND COVER SHALL BE ALIGNED WILL WALL FOR ACCESS TO LADDER BARS.
5. IF THE CONTRACTOR CHOOSES A CIRCULAR STRUCTURE FOR THE YARD INLET, THEN THE APRON SHALL ALSO BE CIRCULAR.

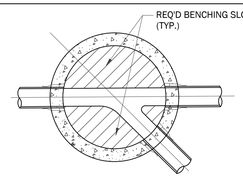
YARD INLET

NOT TO SCALE



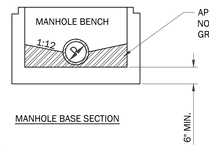
SECTION

NOTES:
1. MANHOLES IN STREETS AND IMPROVED AREAS SHALL BE FINISHED FLUSH WITH FINISHED SURFACE. IN UNIMPROVED AREAS, 12" ABOVE FINISHED SURFACE OR AS DIRECTED BY THE ENGINEER.
2. AFTER SEWER PIPE HAS BEEN LAID TO CORRECT GRADE ALIGNMENT, THE OPENING BETWEEN THE SEWER PIPE AND MANHOLE WALLS WILL BE SEALED WITH NON-SHRINKING GROUT SUCH AS "WATER PLUG OR ANTI-HYDRO".
3. DAMAGE TO THE PRECAST MANHOLE STRUCTURE SPECIFICALLY TO THE TONGUE AND GROOVE ENDS AND/OR GASKET SECTION SHALL REQUIRE THE STRUCTURE TO BE REPLACED BEFORE INSTALLATION. EXTREME CARE SHOULD BE USED DURING DELIVERY, INSTALLATION, ETC.



MANHOLE BASE PLAN

NOTES:
1. PLACE STEPS ON LEAST OBSTRUCTED WALL.
2. MAX. DEFLECTION ANGLE OF SEWER ALIGNMENT = 90°
3. OPENINGS FOR PIPES TO BE FACTORY CAST OR CORED.

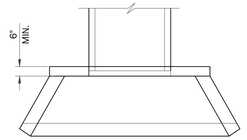


MANHOLE BASE SECTION

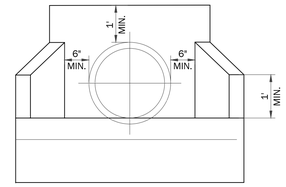
NOTES:
1. BENCH REQUIRED FOR PIPE DIAMETER LESS THAN 48"

STANDARD PRECAST MANHOLE

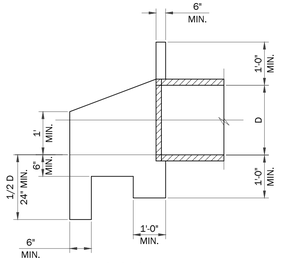
NOT TO SCALE



TOP VIEW



END VIEW

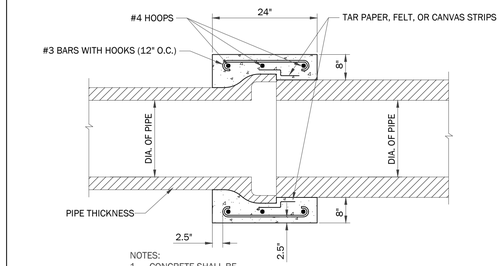


SECTION VIEW

NOTES:
1. ALL SUBMITTAL/SHOP DRAWINGS FOR PRECAST OR CAST-IN-PLACE HEADWALLS SHALL BE STAMPED BY A LICENSED, PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF ALABAMA. ALL DESIGN CALCULATIONS SHALL BE SUBMITTED.
2. THE DETAILS AND DIMENSIONS CHART PROVIDED GIVE STANDARD DIMENSIONS FOR HEADWALLS FOR VARIOUS STORM DRAIN DIAMETERS. THE CONTRACTOR SHOULD NOTE ALL MINIMUM DIMENSIONS.

VERTICAL HEADWALL

NOT TO SCALE



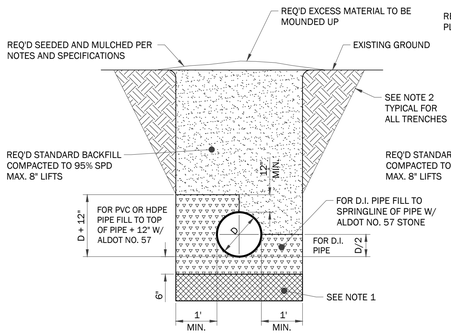
NOTES:
1. CONCRETE SHALL BE 3,000 PSI MIN.

CONCRETE COLLAR DETAIL

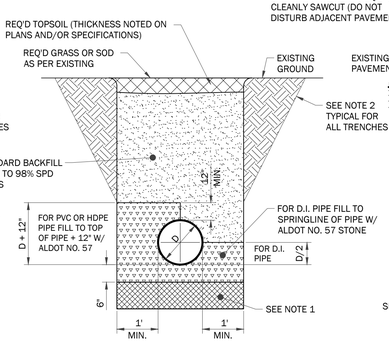
NOT TO SCALE

TRENCH DETAIL - PAVED SURFACES (NEW CONSTRUCTION)

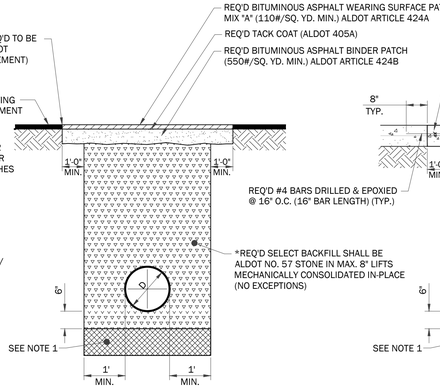
NOT TO SCALE



UNIMPROVED AREAS

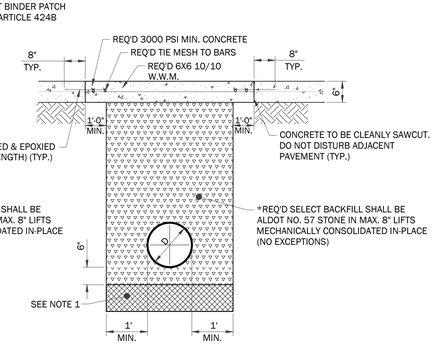


IMPROVED AREAS OR LAWNS



NOTE: IF UTILITY IMPROVEMENT FALLS WITHIN AN AREA THAT IS TO BE REMOVED DURING ROADWAY CONSTRUCTION THEN ELIMINATE THE WEARING SURFACE PATCHING AND MATCH THE BINDER PATCHING WITH EXISTING PAVEMENT.

EXISTING ASPHALT PAVING



EXISTING CONCRETE PAVING

TRENCH DETAILS

NOT TO SCALE

TRENCH DETAIL NOTES

1. TRENCH FOUNDATION REQUIRED IF DIRECTED BY THE ENGINEER, DEPTH VARIES.
2. THERE IS NO ADDITIONAL PAY FOR TRENCH LAY BACK, BENCHING, SHORING, TRENCH BOXES, ETC. THIS IS CONSIDERED A SUBSIDIARY OBLIGATION TO THE UTILITY INSTALLATION.
* 3. THE CONTRACTOR SHALL MECHANICALLY CONSOLIDATE ALL STONE BACKFILL IN MAXIMUM 8" LIFTS AS NOTED. FAILURE TO DO SO SHALL RESULT IN THE STONE BEING REMOVED/REINSTALLED AT THE CONTRACTOR'S EXPENSE OR ONLY PARTIAL PAYMENT BEING MADE FOR THE BID ITEM. STONE SHALL BE PLACED IN APPROPRIATE THICKNESS AND COMPACTED IN THE FOLLOWING SEQUENCE:
   (1) BEDDING (6" MIN.)
   (2) SPRINGLINE OF PIPE
   (3) ONE FOOT ABOVE TOP OF PIPE
   (4) 8" LIFTS FOR REMAINING TRENCH DEPTH
4. UNIMPROVED AREAS SHALL BE CONSIDERED AREAS WHERE NO PREVIOUS DEVELOPMENT HAS OCCURRED AND THE AREA IS NOT MAINTAINED REGULARLY SUCH AS A WOODED/FORESTED AREA OR OPEN FIELD.
5. IMPROVED AREAS OR LAWNS SHALL BE CONSIDERED AREAS WHERE REGULAR MAINTENANCE OCCURS SUCH AS IN PUBLIC RIGHT-OF-WAYS AND ON PRIVATE PROPERTIES. SETTLEMENT OF ANY KIND IN THESE AREAS IS UNACCEPTABLE AND MAXIMUM EFFORT SHALL BE GIVEN TO ENSURE THE IMPROVED/LANDSCAPE AREAS ARE RETURNED TO THEIR PREVIOUS STATE, UNLESS FURTHER IMPROVED BY THE PROJECT.
6. PAVEMENT AREAS (ASPHALT OR CONCRETE) SHALL BE CONSIDERED ANY ROADWAY, DRIVE, SIDEWALK, PAVERS, PARKING LOT, ETC. WHERE THE EXISTING OR FINAL FINISH GRADE IS AN ASPHALT OR CONCRETE SURFACE.
7. IN AREAS OF EXISTING PAVEMENT, THE TRENCH SHALL BE BACKFILLED COMPLETELY WITH STONE AS SHOWN ON THE EXISTING PAVEMENT TRENCH DETAILS. THESE AREAS INCLUDE CROSSING OF EXISTING ROADWAYS, PARKING LOTS, SIDEWALKS, ETC., AND ARE AREAS WHERE EXCAVATED WIDTHS DO NOT ALLOW FOR COMPACTION AND TESTING OF STANDARD BACKFILL. THESE CONFINED AREAS TYPICALLY CANNOT BE BENCHED BACK AND REQUIRE TALLER TRENCH BOXES TO MAINTAIN OSHA REQUIREMENTS.

   IN THE EVENT THAT THE EXISTING PAVEMENT AREA IS BEING COMPLETELY REMOVED TO A LIMIT THAT PROVIDES SUFFICIENT SPACE TO ALLOW PROPER VIBRATORY EQUIPMENT AND, IF NECESSARY, BENCHING OF SIDE SLOPES AS REQUIRED IN THE PROJECT EARTHWORK SPECIFICATIONS (BENCHING IS REQUIRED ON SLOPES GREATER THAN 4:1 SLOPE), THEN THE CONTRACTOR MAY FOLLOW THE TRENCH DETAIL FOR PAVED AREAS (NEW CONSTRUCTION). THIS DETAIL ALLOWS FOR THE USE OF EARTHEN BACKFILL AND WOULD ALSO REQUIRE THE CONTRACTOR TO ALLOW SUFFICIENT WIDTH THAT THE OWNER'S GEOTECHNICAL REPRESENTATIVE COULD TAKE PERIODIC COMPACTION TESTS. IF THE CONTRACTOR IS UNSURE OF WHERE STONE OR EARTHEN BACKFILL IS TO BE INSTALLED ON A PROSPECTIVE PROJECT, THEN THEY SHALL REQUEST CLARIFICATION DURING THE BIDDING OF THE PROJECT. THERE SHALL BE NO CLAIMS CONSIDERED AFTER THE PROJECT HAS BEEN BID.

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REVISIONS	

JOB NO. 24-40A

SHEET NO:

C5.0

8 OF 8

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